



European Technical Approval ETA-11/0523

English translation prepared by DIBt - Original version in German language

Handelsbezeichnung <i>Trade name</i>	BTI Universalrahmendübel ProCon SXR <i>BTI frame fixing ProCon SXR</i>
Zulassungsinhaber <i>Holder of approval</i>	BTI Befestigungstechnik GmbH & Co. KG Salzstraße 51 74653 Ingelfingen DEUTSCHLAND
Zulassungsgegenstand und Verwendungszweck <i>Generic type and use of construction product</i>	Kunststoffdübel als Mehrfachbefestigung von nichttragenden Systemen zur Verankerung im Beton und Mauerwerk <i>Plastic anchor for multiple use in concrete and masonry for non- structural applications</i>
Geltungsdauer: <i>Validity:</i>	vom <i>from</i> bis <i>to</i> 16 December 2011 19 December 2012
Herstellwerke <i>Manufacturing plants</i>	BTI Herstellwerk 1 BTI manufacturing plant 1

Diese Zulassung umfasst
This Approval contains

29 Seiten einschließlich 17 Anhänge
29 pages including 17 annexes

I LEGAL BASES AND GENERAL CONDITIONS

- 1 This European technical approval is issued by Deutsches Institut für Bautechnik in accordance with:
 - Council Directive 89/106/EEC of 21 December 1988 on the approximation of laws, regulations and administrative provisions of Member States relating to construction products¹, modified by Council Directive 93/68/EEC² and Regulation (EC) N° 1882/2003 of the European Parliament and of the Council³;
 - *Gesetz über das In-Verkehr-Bringen von und den freien Warenverkehr mit Bauprodukten zur Umsetzung der Richtlinie 89/106/EWG des Rates vom 21. Dezember 1988 zur Angleichung der Rechts- und Verwaltungsvorschriften der Mitgliedstaaten über Bauprodukte und anderer Rechtsakte der Europäischen Gemeinschaften (Bauproduktengesetz - BauPG) vom 28. April 1998⁴, as amended by law of 31 October 2006⁵;*
 - Common Procedural Rules for Requesting, Preparing and the Granting of European technical approvals set out in the Annex to Commission Decision 94/23/EC⁶;
 - Guideline for European technical approval of "Plastic Anchors for Multiple Use in Concrete and Masonry for Non-structural Applications - Part 1: General", ETAG 020-01.
- 2 Deutsches Institut für Bautechnik is authorized to check whether the provisions of this European technical approval are met. Checking may take place in the manufacturing plant. Nevertheless, the responsibility for the conformity of the products to the European technical approval and for their fitness for the intended use remains with the holder of the European technical approval.
- 3 This European technical approval is not to be transferred to manufacturers or agents of manufacturers other than those indicated on page 1, or manufacturing plants other than those indicated on page 1 of this European technical approval.
- 4 This European technical approval may be withdrawn by Deutsches Institut für Bautechnik, in particular pursuant to information by the Commission according to Article 5(1) of Council Directive 89/106/EEC.
- 5 Reproduction of this European technical approval including transmission by electronic means shall be in full. However, partial reproduction can be made with the written consent of Deutsches Institut für Bautechnik. In this case partial reproduction has to be designated as such. Texts and drawings of advertising brochures shall not contradict or misuse the European technical approval.
- 6 The European technical approval is issued by the approval body in its official language. This version corresponds fully to the version circulated within EOTA. Translations into other languages have to be designated as such.

¹ Official Journal of the European Communities L 40, 11 February 1989, p. 12

² Official Journal of the European Communities L 220, 30 August 1993, p. 1

³ Official Journal of the European Union L 284, 31 October 2003, p. 25

⁴ *Bundesgesetzblatt Teil I 1998*, p. 812

⁵ *Bundesgesetzblatt Teil I 2006*, p. 2407, 2416

⁶ Official Journal of the European Communities L 17, 20 January 1994, p. 34

II SPECIFIC CONDITIONS OF THE EUROPEAN TECHNICAL APPROVAL

1 Definition of the product and intended use

1.1 Definition of the construction product

The BTI frame fixing ProCon SXR in the range ProCon SXR 8 and ProCon SXR 10 is a plastic anchor consisting of a plastic sleeve made of polyamide and an accompanying specific screw of galvanised steel, of galvanised steel with an additional Duplex-coating or of stainless steel.

The plastic sleeve is expanded by screwing in the specific screw which presses the sleeve against the wall of the drilled hole.

The installed anchor is shown in Annex 1.

1.2 Intended use

The anchor is intended to be used for anchorages for which requirements for safety in use in the sense of the Essential Requirement 4 of Council Directive 89/106/EEC shall be fulfilled and failure of the fixture represents an immediate risk to human life.

The anchor is to be used only for multiple fixing for non-structural applications in concrete and masonry. The base material shall consist of reinforced or unreinforced normal weight concrete of strength class C12/15 at minimum according to EN 206-1:2000-12 and of masonry walls according to Annex 6, 7, 8, 9 and 10. The anchor may be used in cracked and non-cracked concrete. The mortar strength class of the masonry has to be M 2,5 according to EN 998-2:2003 at minimum.

The anchor type ProCon SXR 10 may also be used in masonry walls made of (non-cracked) autoclaved aerated concrete blocks (AAC) according to Annex 16 and 17.

By multiple anchor use it is assumed that in the case of excessive slip or failure of one anchor the load can be transmitted to neighbouring anchors without significantly violating the requirements on the fixture in the serviceability and ultimate limit state.

Therefore the design of the fixture may specify the number n_1 of fixing points to fasten the fixture and the number n_2 of anchors per fixing point. Furthermore by specifying the design value of actions N_{Sd} on a fixing point to a value $\leq n_3$ (kN) up to which the strength and stiffness of the fixture are fulfilled and the load transfer in the case of excessive slip or failure of one anchor need not to be taken into account in the design of the fixture.

The following default values for n_1 , n_2 and n_3 may be taken:

$$\begin{array}{l} n_1 \geq 4; \quad n_2 \geq 1 \quad \text{and} \quad n_3 \leq 4,5 \text{ kN} \quad \text{or} \\ n_1 \geq 3; \quad n_2 \geq 1 \quad \text{and} \quad n_3 \leq 3,0 \text{ kN}. \end{array}$$

The anchor may also be used in concrete with requirements related to resistance to fire according 4.2.1.2.

The specific screw made of galvanised steel or galvanised steel with an additional Duplex-coating may only be used in structures subject to dry internal conditions.

The specific screw made of stainless steel may be used in structures subject to dry internal conditions and also in structures subject to external atmospheric exposure (including industrial and marine environment), or exposure in permanently damp internal conditions, if no particular aggressive conditions exist. Such particular aggressive conditions are e. g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e. g. in desulphurization plants or road tunnels where de-icing materials are used).

The specific screw made of galvanised steel or galvanised steel with an additional Duplex-coating may also be used in structures subject to external atmospheric exposure, if the area of the head of the screw is protected against moisture and driving rain after mounting of the fixing unit in this way, that intrusion of moisture into the anchor shaft is prevented. Therefore there shall be an external cladding or a ventilated rainscreen mounted in front of the head of the screw and the head of the screw itself shall be coated with a soft plastic, permanently elastic bitumen-oil-combination coating (e. g. undercoating or body cavity protection for cars).

The anchor may be used in the following temperature range:

Temperature range b):	-40 °C to +80 °C	(max long term temperature +50 °C and max short term temperature +80 °C)
Temperature range c):	-40 °C to +50 °C	(max long term temperature +30 °C and max short term temperature +50 °C)

The provisions made in this European technical approval are based on an assumed working life of the anchor of 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

2 Characteristics of the product and methods of verification

2.1 Characteristics of the product

The anchor corresponds to the drawings and information given in Annex 2 and 3. The characteristic material values, dimensions and tolerances of the anchor not given in these Annexes shall correspond to the respective values laid down in the technical documentation⁷ of this European technical approval.

The characteristic values for the design of the anchorages are given in Annex 3 and 4, 6 to 10 and 16.

Each anchor is to be marked with the identifying mark, the type, the diameter and the length of the anchor according to Annex 2.

The minimum embedment depth shall be marked.

The anchor shall only be packaged and supplied as a complete unit.

⁷ The technical documentation of this European technical approval is deposited at the Deutsches Institut für Bautechnik and, as far as relevant for the tasks of the approved bodies involved in the attestation of conformity procedure, is handed over to the approved bodies.

2.2 Methods of verification

The assessment of the fitness of the anchor for the intended use in relation to the requirements for safety in use in the sense of the Essential Requirement 4 has been made in compliance with the Guideline for European technical approval of "Plastic Anchors for Multiple Use in Concrete and Masonry for Non-structural Applications", ETAG 020,

- Part 1: "General",
- Part 2: "Plastic Anchors for Use in Normal Weight Concrete",
- Part 3: "Plastic Anchors for Use in Solid Masonry Materials" and
- Part 4: "Plastic Anchors for Use in Hollow or Perforated Masonry",
- Part 5: "Plastic Anchors for Use in Autoclaved Aerated Concrete (AAC)"

based on the use categories a, b, c and d.

In addition to the specific clauses relating to dangerous substances contained in this European technical approval, there may be other requirements applicable to the products falling within its scope (e. g. transposed European legislation and national laws, regulations and administrative provisions). In order to meet the provisions of the Construction Products Directive, these requirements need also to be complied with, when and where they apply.

3 Evaluation and attestation of conformity and CE marking

3.1 System of attestation of conformity

According to the decision 97/463/EG of the European Commission⁸ the system 2(ii) (referred to as system 2+) of attestation of conformity applies.

This system of attestation of conformity is defined as follows.

System 2+: Declaration of conformity of the product by the manufacturer on the basis of:

- (a) Tasks for the manufacturer:
 - (1) initial type-testing of the product;
 - (2) factory production control;
 - (3) testing of samples taken at the factory in accordance with a prescribed test plan.
- (b) Tasks for the approved body:
 - (4) certification of factory production control on the basis of:
 - initial inspection of factory and of factory production control;
 - continuous surveillance, assessment and approval of factory production control.

3.2 Responsibilities

3.2.1 Tasks of the manufacturer

3.2.1.1 Factory production control

The manufacturer shall exercise permanent internal control of production. All the elements, requirements and provisions adopted by the manufacturer shall be documented in a systematic manner in the form of written policies and procedures, including records of results performed. This production control system shall insure that the product is in conformity with this European technical approval.

The manufacturer may only use raw materials stated in the technical documentation of this European technical approval.

⁸ Official Journal of the European Communities L 198 of 25.07.1997.

The factory production control shall be in accordance with the control plan which is part of the technical documentation of this European technical approval. The control plan is laid down in the context of the factory production control system operated by the manufacturer and deposited at Deutsches Institut für Bautechnik.⁹

The results of factory production control shall be recorded and evaluated in accordance with the provisions of the control plan.

3.2.1.2 Other tasks of manufacturer

The manufacturer shall, on the basis of a contract, involve a body which is approved for the tasks referred to in section 3.1 in the field of anchors in order to undertake the actions laid down in section 3.2.2. For this purpose, the control plan referred to in sections 3.2.1.1 and 3.2.2 shall be handed over by the manufacturer to the approved body involved.

The manufacturer shall make a declaration of conformity, stating that the construction product is in conformity with the provisions of this European technical approval.

3.2.2 Tasks of approved bodies

The approved body shall perform the

- initial inspection of factory and of factory production control,
- continuous surveillance, assessment and approval of factory production control,

in accordance with the provisions laid down in the control plan.

The approved body shall retain the essential points of its actions referred to above and state the results obtained and conclusions drawn in a written report.

The approved certification body involved by the manufacturer shall issue an EC certificate of conformity of the factory production control stating the conformity with the factory production control of this European technical approval.

In cases where the provisions of the European technical approval and its control plan are no longer fulfilled the certification body shall withdraw the certificate of conformity and inform Deutsches Institut für Bautechnik without delay.

3.3 CE marking

The CE marking shall be affixed on each packaging of the anchor. The letters "CE" shall be followed by the identification number of the approved certification body, where relevant, and be accompanied by the following additional information:

- the name and address of the holder of the approval (legal entity responsible for the manufacturer),
- the last two digits of the year in which the CE marking was affixed,
- the number of the EC certificate for the factory production control,
- the number of the European technical approval,
- the number of the guideline for European technical approval,
- use categories a, b, c and d ("d" only for anchor type ProCon SXR 10).

⁹

The control plan is a confidential part of the documentation of the European technical approval, but not published together with the European technical approval and only handed over to the approved body involved in the procedure of attestation of conformity.
See section 3.2.2.

4 Assumptions under which the fitness of the product for the intended use was favourably assessed

4.1 Manufacturing

The European technical approval is issued for the product on the basis of agreed data/information, deposited with Deutsches Institut für Bautechnik, which identifies the product that has been assessed and judged. Changes to the product or production process, which could result in this deposited data/information being incorrect, should be notified to Deutsches Institut für Bautechnik before the changes are introduced. Deutsches Institut für Bautechnik will decide whether or not such changes affect the European technical approval and consequently the validity of the CE marking on the basis of the European technical approval and if so whether further assessment or alterations to the European technical approval shall be necessary.

4.2 Installation

4.2.1 Design of anchorages

4.2.1.1 General

The anchor is to be used only for multiple fixing for non-structural applications.

By multiple anchor use it is assumed that in the case of excessive slip or failure of one anchor the load can be transmitted to neighbouring anchors without significantly violating the requirements on the fixture in the serviceability and ultimate limit state.

Therefore the design of the fixture may specify the number n_1 of fixing points to fasten the fixture and the number n_2 of anchors per fixing point. Furthermore by specifying the design value of actions N_{Sd} on a fixing point to a value $\leq n_3$ (kN) up to which the strength and stiffness of the fixture are fulfilled and the load transfer in the case of excessive slip or failure of one anchor need not to be taken into account in the design of the fixture.

The following default values for n_1 , n_2 and n_3 may be taken:

$$\begin{array}{l} n_1 \geq 4; \quad n_2 \geq 1 \quad \text{and} \quad n_3 \leq 4,5 \text{ kN} \quad \text{or} \\ n_1 \geq 3; \quad n_2 \geq 1 \quad \text{and} \quad n_3 \leq 3,0 \text{ kN}. \end{array}$$

Fitness for the intended use of the anchor is given under the following conditions:

- The design of anchorages is carried out in compliance with ETAG 020, Guideline for European technical approval of "Plastic Anchors for Multiple Use in Concrete and Masonry for Non-structural Applications", Annex C under the responsibility of an engineer experienced in anchorages. This design method applies to plastic anchors subject to static or quasi-static actions in tension, shear or combined tension and shear or bending; it is not applicable to plastic anchors loaded in compression or subject to fatigue, impact, or seismic actions.
- Verifiable calculation notes and drawings shall be prepared taking account of the loads to be anchored, the nature and strength of the base materials and the dimensions of the anchorage members as well as of the relevant tolerances.

Shear loads acting on an anchor may be assumed to act without lever arm if both of the following conditions are fulfilled:

- The fixture shall be made of metal and in the area of the anchorage be fixed directly to the base material either without an intermediate layer or with a levelling layer of mortar with a thickness ≤ 3 mm.

- The fixture shall be in contact with the anchor over its entire thickness. (Therefore the diameter of clearance hole in the fixture d_f has to be equal or smaller than the value given in Annex 3, Table 3.)

If these two conditions are not fulfilled the lever arm is calculated according to ETAG 020, Annex C. The characteristic bending moment is given in Annex 3, Table 4.

4.2.1.2 Resistance in concrete (use category "a")

The characteristic values of resistance of the anchor for use in concrete are given in Annex 3, Table 4 and Annex 4, Table 5 and 6. The design method is valid for cracked and non-cracked concrete.

According to the Technical Report TR 020 "Evaluation of anchorages in concrete concerning resistance to fire" it can be assumed that for fastening of facade systems the load bearing behaviour of the BTI frame fixing ProCon SXR 10 has a sufficient resistance to fire at least 90 minutes (R90) if the admissible load $[F_{Rk} / (\gamma_M \cdot \gamma_F)]$ is $\leq 0,8$ kN (no permanent centric tension load).

4.2.1.3 Resistance in solid masonry (use category "b")

The characteristic values of resistance of the anchor for use in solid masonry are given in Annex 3, Table 4 and Annex 6, 8 and 10. These values are independent of the load direction (tension, shear or combined tension and shear) and the mode of failure.

The characteristic resistances given in Annex 6, 8 and 10 for use in solid masonry are only valid for the base material and the bricks according this table or larger brick sizes and larger compressive strength of the masonry unit.

If smaller brick sizes are present on the construction site or if the mortar strength is smaller than the required value, the characteristic resistance of the anchor may be determined by job site tests according to 4.2.3.

4.2.1.4 Resistance in hollow or perforated masonry (use category "c")

The characteristic resistances for use in hollow or perforated masonry given in Annex 7, 9 and 10 are only valid for the bricks and blocks according this table regarding base material, size of the units, compressive strength and configuration of the voids.

These values are independent of the load direction (tension, shear or combined tension and shear) and the mode of failure and are valid for $h_{nom} = 50$ mm only.

The influence of larger embedment depths ($h_{nom} \geq 50$ mm) and/or different bricks and blocks (according Annex 7, 9 and 10 regarding base material, size of the units, compressive strength and configuration of the voids) has to be detected by job site tests according to 4.2.3.

4.2.1.5 Resistance in (non-cracked) autoclaved aerated concrete blocks (AAC, use category "d")

The characteristic values of resistance of the anchor type ProCon SXR 10 for use in masonry made of (non-cracked) autoclaved aerated concrete blocks (AAC) are given in Annex 16, Table 14. These values are independent of the load direction (tension, shear or combined tension and shear) and the mode of failure.

The anchor shall not be installed and used in water saturated aerated concrete.

4.2.1.6 Specific conditions for the design method in solid masonry, hollow or perforated masonry and AAC blocks

The mortar strength class of the masonry has to be M 2,5 according to EN 998-2:2003 at minimum.

The characteristic resistance F_{Rk} for a single plastic anchor may also be taken for a group of two or four plastic anchors with a spacing equal or larger than the minimum spacing s_{min} .

The distance between single plastic anchors or a group of anchors should be $s \geq 250$ mm.

If the vertical joints of the wall are designed not to be filled with mortar then the design resistance N_{Rd} has to be limited to 2,0 kN to ensure that a pull-out of one brick out of the wall will be prevented. This limitation can be omitted if interlocking units are used for the wall or when the joints are designed to be filled with mortar.

If the joints of the masonry are not visible the characteristic resistance F_{Rk} has to be reduced with the factor $\alpha_j = 0,5$.

If the joints of the masonry are visible (e.g. unplastered wall) following has to be taken into account:

- The characteristic resistance F_{Rk} may be used only, if the wall is designed such that the joints are to be filled with mortar.
- If the wall is designed such that the joints are not to be filled with mortar then the characteristic resistance F_{Rk} may be used only, if the minimum edge distance c_{min} to the vertical joints is observed. If this minimum edge distance c_{min} can not be observed then the characteristic resistance F_{Rk} has to be reduced with the factor $\alpha_j = 0,5$.

4.2.1.7 Characteristic values, spacing and dimensions of anchorage member

The minimum spacing and dimensions of anchorage member according to Annex 5, 11 and 17 shall be observed depending on the base material.

4.2.1.8 Displacement behaviour

The displacements under tension and shear loading in concrete and masonry are given in Annex 5, Table 7 and Annex 17, Table 16.

4.2.2 Installation of anchor

The fitness for use of the anchor can only be assumed if the following conditions of installation are met:

- Anchor installation carried out by appropriately qualified personnel under the supervision of the person responsible for technical matters on site.
- Use of the anchor only as supplied by the manufacturer without exchanging any component of the anchor.
- Anchor installation in accordance with the manufacturer's specifications and drawings using the tools indicated in this European technical approval.
- Checks before placing the anchor, to ensure that the characteristic values of the base material in which the anchor is to be placed, is identical with the values, which the characteristic loads apply for.
- Observation of the drill method according Annex 7, 9 and 10 (Drill holes in some hollow or perforated masonry may only be drilled using the rotary drill. Other drilling methods may also be used if job-site tests according to 4.2.3 evaluate the influence of hammer or impact drilling.).
- For the fixing of the anchor type ProCon SXR 10 in autoclaved aerated concrete blocks with a nominal compressive strength $f_{ck} < 4 \text{ N/mm}^2$ the hole is made by using the accompanying AAC Hole Punch according Annex 16, Table 15. The Hole Punch is driven into the autoclaved aerated concrete by using the hammer drilling of the power drill. To ensure the correct application of the Hole Punch a marking flute at the surface of the fixture around the hole is visible.

Drill holes in autoclaved aerated concrete blocks with a compressive strength $f_{ck} \geq 4 \text{ N/mm}^2$ have to be drilled with a drill bit using the rotary drill.

- Placing drill holes without damaging the reinforcement.
- The anchor shall not be installed and used in water saturated aerated concrete (AAC).
- Holes to be cleaned of drilling dust.

- In case of aborted hole: New drilling at a minimum distance away of twice the depth of the aborted hole or smaller distance if the aborted drill hole is filled with high strength mortar.
- The plastic sleeve is inserted through the fixture by slight hammer blows and the special screw is screwed in until the head of the screw touches the sleeve. The anchor is correct mounted, if there is no turn-through of the plastic sleeve in the drill hole and if slightly move on turning of the screw is impossible after the complete turn-in of the screw.
- Temperature during installation of the anchor ≥ -5 °C (plastic sleeve and base material).
- Exposure to UV due to solar radiation of the anchor not protected ≤ 6 weeks.

4.2.3 Job site tests according to ETAG 020, Annex B

4.2.3.1 General

In the absence of national requirements the characteristic resistance of the plastic anchor may be determined by job site tests according to 4.2.3, if the plastic anchor has already characteristic values given in Annex 6 to 10 for the same base material as it is present on the construction works.

Furthermore job site tests for use in (different) solid masonry are possible only if the plastic anchor has already characteristic values given in Annex 6, 8 and 10 for use in solid masonry.

Job site tests for use in (different) hollow or perforated masonry are possible only if the plastic anchor has already characteristic values given in Annex 7, 9 and 10 for use in hollow or perforated masonry.

Job site tests are also possible, if another drill method is been used as it is given in Annex 7, 9 and 10.

The characteristic resistance to be applied to a plastic anchor should be determined by means of at least 15 pull-out tests carried out on the construction work with a centric tension load acting on the plastic anchor. These tests may also performed in a laboratory under equivalent conditions as used on construction work

Execution and evaluation of the tests as well as issue of the test report and determination of the characteristic resistance should be supervised by the person responsible for execution of works on site and be carried out by a competent person.

Number and position of the plastic anchors to be tested should be adapted to the relevant special conditions of the construction work in question and, for example, in the case of blind and larger areas be increased such that a reliable information about the characteristic resistance of the plastic anchor embedded in the base material in question can be derived. The tests should take account of the unfavourable conditions of practical execution.

4.2.3.2 Assembly

The plastic anchor to be tested shall be installed (e. g. preparation of drill hole, drilling tool to be used, drill bit, type of drilling hammer or rotation, thickness of fixture) and as far as spacing and edge distances are concerned be distributed in the same way as foreseen for the intended use.

Depending on the drilling tool hard metal hammer drill bits or hard metal percussion drill bits, respectively, according to ISO 5468 should be used. New drill bits should be used for one test series or drill bits with $d_{\text{cut,m}} = 8,25 \text{ mm} < d_{\text{cut}} \leq 8,45 \text{ mm} = d_{\text{cut,max}}$ (ProCon SXR 8) or $d_{\text{cut,m}} = 10,25 \text{ mm} < d_{\text{cut}} \leq 10,45 \text{ mm} = d_{\text{cut,max}}$ (ProCon SXR 10) respectively.

4.2.3.3 Execution of test

The test rig used for the pull-out tests shall provide a continuous slow increase of the load, controlled by a calibrated load cell. The load shall apply perpendicular to the surface of the base material and shall be transmitted to the anchor via a hinge. The reaction forces shall be transmitted into the base material such that possible breakout of the masonry is not restricted. This condition is considered as fulfilled, if the support reaction forces are transmitted either in adjacent masonry units or at a distance of at least 150 mm from the plastic anchors. The load shall be increased continuously in a way that the ultimate load is reached after about 1 minute. The load is measured when the ultimate load (N_1) is achieved.

If no pull-out failure occurs, other test methods are needed, e.g. proof-loading.

4.2.3.4 Test report

The test report shall include all information necessary to assess the resistance of the tested anchor. It shall be given to the person responsible for the design of the fastening and shall be included in the construction dossier.

The minimum data required are:

- Name of product
- Construction site, owner of building; date and location of the tests, air temperature
- Date and place of tests
- Test rig
- Type of structure to be fixed
- Masonry (type of brick, strength class, all dimensions of bricks, mortar group if possible); visual assessment of masonry (flush joints, joint clearance, regularity)
- Plastic anchor and special screw
- value of the cutting diameter of hard metal hammer-drill bits, measured before and after drilling if no new drill bits are used
- Results of tests including the indication of value N_1 ; mode of failure
- Tests carried out or supervised by ...; signature

4.2.3.5 Evaluation of test results

The characteristic resistance F_{Rk1} is derived from the measured values N_1 as follows

$$F_{Rk1} = 0,5 \cdot N_1$$

The characteristic resistance F_{Rk1} has to be equal or smaller than the characteristic resistance F_{Rk} which is given in the European technical approval for similar masonry (bricks or blocks)

N_1 = the mean value of the five smallest measured values at ultimate load

In absence of national regulations the partial safety factors for the resistance of the plastic anchor may be taken as $\gamma_M = 2,5$ for use in masonry.

5 Indications to the manufacturer

5.1 Responsibility of the manufacturer

It is in the responsibility of the manufacturer to ensure that the information on the specific conditions according to 1 and 2 including Annexes referred to as well as sections 4.2.1, 4.2.2 and 5 is given to those who are concerned. This information may be made by reproduction of the respective parts of the European technical approval. In addition, all installation data shall be shown clearly on the packaging and/or on an enclosed instruction sheet, preferably using illustrations.

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The minimum data required are:

- base material for the intended use,
- ambient temperature of the base material during installation of the anchor,
- drill bit diameter,
- minimum effective anchorage depth,
- minimum hole depth,
- information on the installation procedure,
- identification of the manufacturing batch.

All data shall be presented in a clear and explicit form.

5.2 Packaging, transport and storage

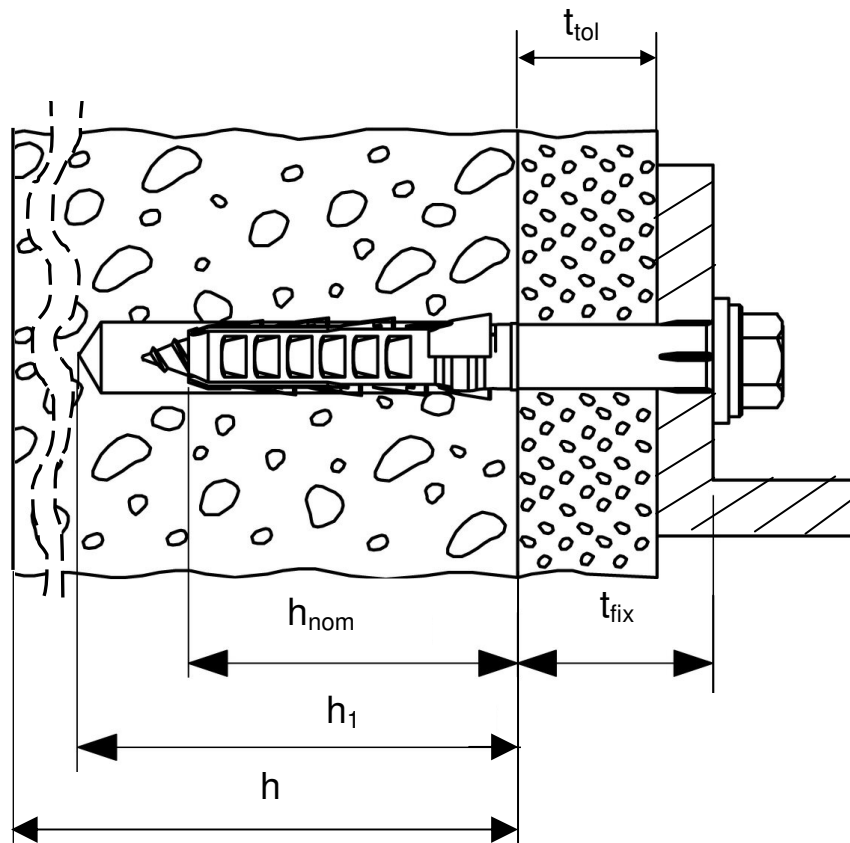
The anchor shall only be packaged and supplied as a complete unit.

The anchor shall be stored under normal climatic conditions in its original light-proof packaging. Before installation, it shall not be extremely dried nor frozen.

Georg Feistel
Head of Department

beglaubigt:
Baderschneider

BTI frame fixing ProCon SXR



Intended Use

Fixing in concrete and different kinds of masonry and autoclaved aerated concrete (AAC)

Legend

- h_{nom} = overall plastic anchor embedment depth in the base material
- h_1 = depth of drill hole to deepest point
- h = thickness of member (wall)
- t_{fix} = t_{tol} + thickness of fixture
- t_{tol} = thickness of layer or non-load bearing coating

BTI frame fixing ProCon SXR

Annex 1

Intended use

of European
technical approval

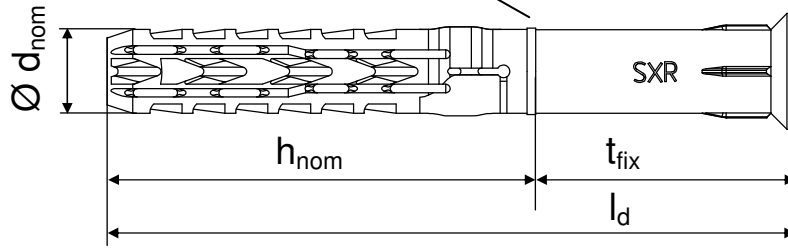
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BTI frame fixing ProCon SXR

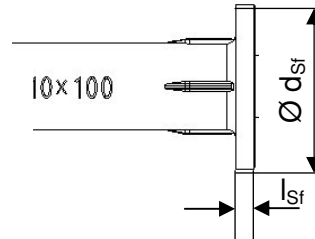
Marking:
Brand
Anchor type
Size
e.g. SXR 10x100

Anchor sleeve:

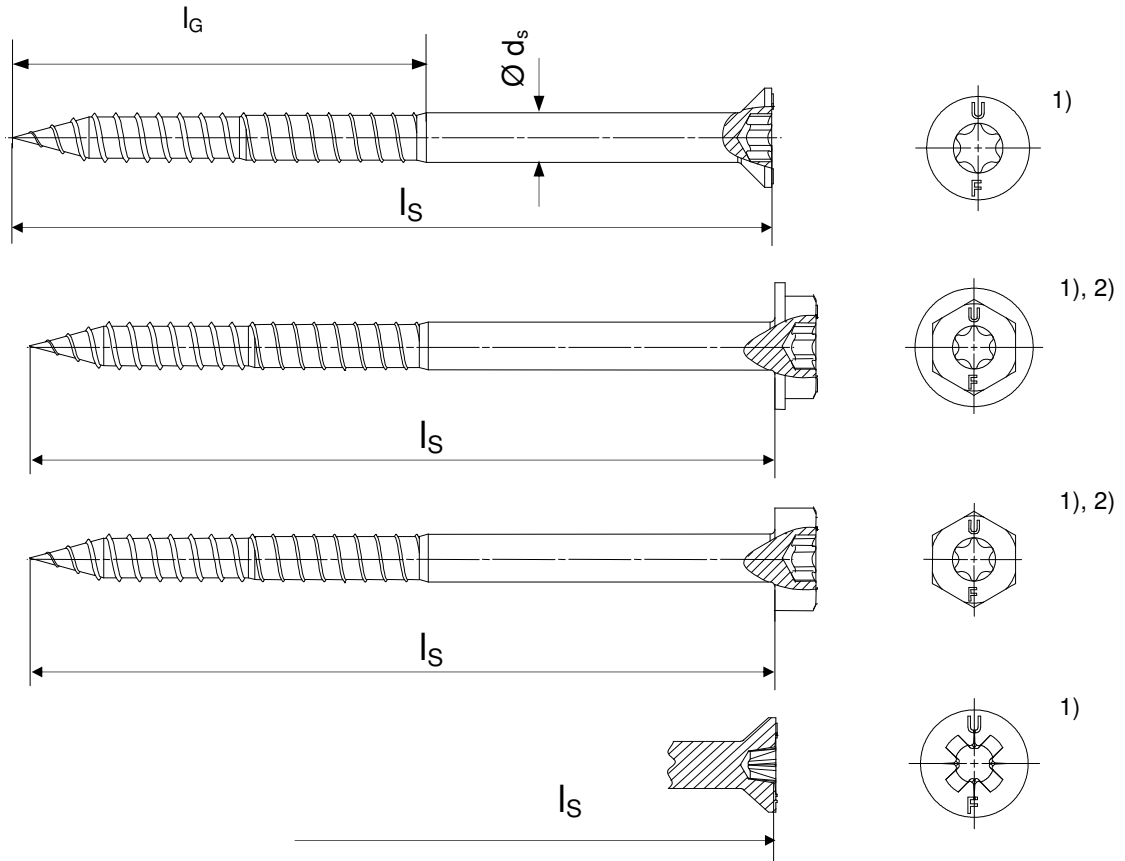
Marking of embedment depth



flat collar version:



Special screw:



- 1) Additional marking of the special stainless steel screw: „A4“.
- 2) Internal driving feature for Torx bit is optional for hexagonal head.

BTI frame fixing ProCon SXR

Anchor type, special screws, dimensions

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Table 1: Dimensions [mm]

Anchor type	Anchor sleeve						Special screw		
	h_{nom} [mm]	$\varnothing d_{nom}$ [mm]	t_{fix} [mm]	l_d [mm]	l_{sf} [mm]	$\varnothing d_{sf}$ [mm]	$\varnothing d_s$ [mm]	l_G [mm]	l_s [mm]
ProCon SXR 8	50	8	≥ 1	51-360	1,8	15,0	6,0	≥ 55	$\geq 57^{2)}$
ProCon SXR 10	50	10	≥ 1	51-360	2,2	18,5	7,0	≥ 56	$\geq 58^{1)}$

- 1) To insure that the screw penetrates the anchor sleeve, l_s must be $l_d + l_{sf}^{3)}$ + 7 mm
- 2) To insure that the screw penetrates the anchor sleeve, l_s must be $l_d + l_{sf}^{3)}$ + 6 mm
- 3) Only valid for flat collar version

Table 2: Materials

Name	Material
Anchor sleeve	Polyamide, PA6, colour grey
Special screw	Steel ($f_{yk} \geq 480 \text{ N/mm}^2$; $f_{uk} \geq 600 \text{ N/mm}^2$) gvz A2G or A2F acc. to EN ISO 4042 or gvz A2G or A2F acc. to EN ISO 4042 + Duplex-coating type Delta-Seal in three layers (total layer thickness $\geq 6 \mu\text{m}$) or Stainless steel to EN 10 088 ($f_{yk} \geq 450 \text{ N/mm}^2$; $f_{uk} \geq 580 \text{ N/mm}^2$)

Table 3: Installation parameters

Anchor type		ProCon SXR 8	ProCon SXR 10
Drill hole diameter	$d_0 =$ [mm]	8	10
Cutting diameter of drill bit	$d_{cut} \leq$ [mm]	8,45	10,45
Depth of drill hole to deepest point ¹⁾	$h_1 \geq$ [mm]	60	60
Overall plastic anchor embedment depth in the base material ^{1) 2)}	$h_{nom} \geq$ [mm]	50	50
Diameter of clearance hole in the fixture	$d_f \leq$ [mm]	8,5	10,5

- 1) See Annex 1
- 2) For hollow and perforated masonry the influence of $h_{nom} \geq 50 \text{ mm}$ has to be detected by job site tests according 4.2.1.4 and 4.2.3.

Table 4: Characteristic bending resistance of the screw in concrete, masonry and AAC

Anchor type		ProCon SXR 8		ProCon SXR 10	
		galvanised steel	stainless steel	galvanised steel	stainless steel
Characteristic bending resistance	$M_{Rk,s}$ [Nm]	12,4	10,4	17,7	17,1
Partial safety factor	$\gamma_{Ms}^{1)}$	1,25	1,29	1,25	1,29

- 1) In absence of other national regulations

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Characteristic resistance in concrete
(Use category "a")

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Table 5: Characteristic resistance of the screw for use in concrete

Failure of expansion element (special screw)		ProCon SXR 8		ProCon SXR 10	
		galvanized steel	stainless steel	galvanized steel	stainless steel
Characteristic tension resistance	$N_{Rk,s}$ [kN]	14,8	12,3	18,7	18,1
Partial safety factor	$\gamma_{Ms}^{1)}$	1,50	1,55	1,50	1,55
Characteristic shear resistance	$V_{Rk,s}$ [kN]	7,4	6,2	9,4	9,0
Partial safety factor	$\gamma_{Ms}^{1)}$	1,25	1,29	1,25	1,29

1) In absence of other national regulations

Table 6: Characteristic resistance for use in concrete

Pull-out failure (plastic sleeve)	ProCon SXR 8		ProCon SXR 10		
Temperature range	30/50 °C	50/80 °C	30/50 °C	50/80 °C	
Concrete \geq C12/15					
Characteristic resistance	$N_{Rk,p}$ [kN]	2,5	2,5	5,0	4,5
Partial safety factor	$\gamma_{Mc}^{1)}$	1,8			
Concrete cone failure and concrete edge failure for single anchor and anchor group					
Tension load ²⁾					
$N_{Rk,c} = 7,2 \cdot \sqrt{f_{ck,cube}} \cdot h_{ef}^{1,5} \cdot \frac{c}{c_{cr,N}} = N_{Rk,p} \cdot \frac{c}{c_{cr,N}}$					
with: $h_{ef}^{1,5} = \frac{N_{Rk,p}}{7,2 \cdot \sqrt{f_{ck,cube}}}$					
$\frac{c}{c_{cr,N}} \leq 1$					
Shear load ²⁾					
$V_{Rk,c} = 0,45 \cdot \sqrt{d_{nom}} \cdot (h_{nom}/d_{nom})^{0,2} \cdot \sqrt{f_{ck,cube}} \cdot c_1^{1,5} \cdot \left(\frac{c_2}{1,5c_1}\right)^{0,5} \cdot \left(\frac{h}{1,5c_1}\right)^{0,5}$					
with: $\left(\frac{c_2}{1,5c_1}\right)^{0,5} \leq 1$					
$\left(\frac{h}{1,5c_1}\right)^{0,5} \leq 1$					
c_1	Edge distance closest to the edge in loading direction				
c_2	Edge distance perpendicular to direction 1				
$f_{ck,cube}$	Nominal characteristic concrete compression strength (based on cubes), value for C50/60 at maximum				
Partial safety factor	$\gamma_{Mc}^{1)}$	1,8			

1) In absence of other national regulations

2) The design method according to ETAG 020, Annex C is to be used

BTI frame fixing ProCon SXR

Characteristic resistance in concrete
(Use category "a")

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Table 7: Displacements under tension und shear loading in concrete¹⁾, masonry¹⁾

Anchor type	Tension load			Shear load		
	F ²⁾ [kN]	δ_{NO} [mm]	$\delta_{N\infty}$ [mm]	F ²⁾ [kN]	δ_{VO} [mm]	$\delta_{V\infty}$ [mm]
ProCon SXR 8	1,0	0,59	1,19	1,0	0,89	1,33
ProCon SXR 10	2,0	1,29	2,58	2,0	1,15	1,74

1) Valid for all ranges of temperatures

2) Intermediate values by linear interpolation

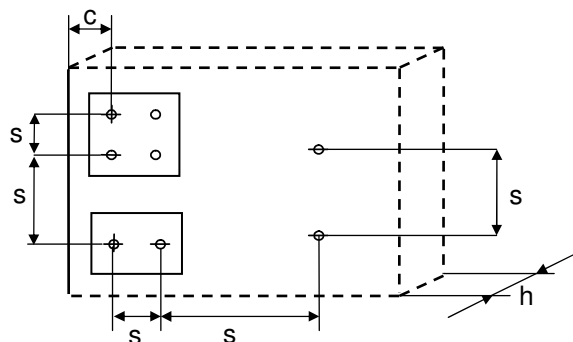
Table 8: Minimum thickness of member, edge distance and spacing in concrete

ProCon SXR 8: In case a fixing point consists of more than 1 anchor with spacing of $s \leq 60$ mm, this fixing point is considered as a group with a max. characteristic resistance $N_{Rk,p}$ acc. to Table 6. For $s > 60$ mm, the anchors are always considered as single anchors, each with a characteristic resistance $N_{Rk,p}$ acc. to Table 6.

ProCon SXR 10: In case a fixing point consists of more than 1 anchor with spacing of $s \leq 90$ mm, this fixing point is considered as a group with a max. characteristic resistance $N_{Rk,p}$ acc. to Table 6. For $s > 90$ mm, the anchors are always considered as single anchors, each with a characteristic resistance $N_{Rk,p}$ acc. to Table 6.

Anchor type		Minimum thickness of member h_{min} [mm]	Characteristic edge distance $c_{cr,N}$ [mm]	Minimum allowable spacing and edge distances ¹⁾ [mm]
ProCon SXR 8	Concrete \geq C16/20	100	50	$s_{min} = 50$ for $c_{min} \geq 50$
	Concrete C12/15		70	$s_{min} = 70$ for $c_{min} \geq 70$
ProCon SXR 10	Concrete \geq C16/20		100	$s_{min} = 70$ for $c_{min} \geq 60$ $s_{min} = 50$ for $c_{min} \geq 150$
	Concrete C12/15		140	$s_{min} = 100$ for $c_{min} \geq 85$ $s_{min} = 70$ for $c_{min} \geq 210$

1) Intermediate values by linear interpolation

Scheme of distance and spacing in concrete**BTI frame fixing ProCon SXR**

Displacements
Minimum thickness of member
Minimum spacing and edge distances in concrete

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Table 9.1: ProCon SXR 8 characteristic resistance F_{RK} in [kN] in solid masonry (use category "b")

Base material [Supplier Title]	Min. DF or min. size (L x W x H) [mm]	Bulk density class ρ [kg/dm ³]	Minimum compressive strength f_b [N/mm ²]	Drill method	Characteristic resistance F_{RK} ¹⁾ ProCon SXR 8 [kN]
					50/80 °C
Terracotta block Mz, e.g. Mz acc. to DIN 105, DIN EN 771-1 e.g. Schlagmann, Mz	3 DF (240x175x113)	$\geq 1,8$	20	H ²⁾	2,5
			10		2,0
Terracotta block Mz, e.g. Mz acc. to DIN 105, DIN EN 771-1	NF (240x115x71)	$\geq 1,8$	20	H ²⁾	2,5
			10		2,0
Terracotta block Mz, e.g. Mz acc. to DIN EN 771-1+ A1:2005, e.g. Wienerberger DK MS	DF (240x115x52)	$\geq 1,8$	28	H ²⁾	2,5
			20		2,0
			10		1,5
Calcium silicate solid brick e.g. KS acc. to DIN 106, DIN EN 771-2 e.g. KS Wemding, KS	NF (240x115x71)	$\geq 1,8$	20	H ²⁾	2,5
			10		2,0
	(175x500x235)	$\geq 2,0$	20		2,5
			10		2,5
Lightweight solid brick, e.g. acc. to DIN 18152, DIN EN 771-3 e.g. KLB V	(240x115x113)	$\geq 1,2$	2	H ²⁾	0,9
	(240x490x115)	$\geq 1,0$	2		1,2
	(250x240x245)	$\geq 1,8$	8		2,5
			4		1,2
	(240x490x115)	$\geq 1,4$	6		0,9
			4		0,6 (0,75)⁴⁾
Solid block normal concrete VBN acc. to DIN 18153, DIN EN 771-3 e.g. Adolf Blatt, VBN	(246x240x245)	$\geq 1,8$	12	H ²⁾	2,5
			8		1,5
			4		1,0 (0,75)⁴⁾
Partial safety factor³⁾				γ_{Mm}	2,5

1) Characteristic resistance F_{RK} for tension, shear or combined tension and shear loading.

The characteristic resistance is valid for single plastic anchor or for a group of two or four plastic anchors with a spacing equal or larger than the minimum spacing s_{min} according to Table 11. The specific conditions for the design method have to be considered according to chapter 4.2.1.5 of the ETA.

2) H = Hammer drilling, R = Rotary drilling

3) In absence of other national regulations

4) The value in brackets (F_{RK}) is valid for temperature range c) 30/50 °C only (compare chapter 1.2 of the ETA).

BTI frame fixing ProCon SXR

Anchor type ProCon SXR 8:
Characteristic resistance in solid masonry
(Use category "b")

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Table 9.2: ProCon SXR 8 characteristic resistance F_{Rk} in [kN] in hollow or perforated masonry ("c")

Base material [Supplier Title]	Min. DF or min. size (L x W x H) [mm]	Bulk density class ρ [kg/dm ³]	Minimum compressive strength f_b [N/mm ²]	Drill method	Geometry Annex (Figure)	Characteristic resistance F_{Rk} ¹⁾ ProCon SXR 8 [kN]
						50/80 °C
Terracotta block Form B, HLz acc. to DIN 105, DIN EN 771-1 e.g. Wienerberger HLz	2 DF (240x115x113)	$\geq 1,2$	20	R ²⁾	13 (1)	1,2
			8			0,5
Terracotta block, HLz acc. DIN EN 771-1+ A1:2005, e.g. Wienerberger BS	DF (240x110x52)	$\geq 1,5$	28	H ²⁾	15 (17)	2,5
			20			1,2 (1,5) ⁴⁾
			10			0,6 (0,9) ⁴⁾
Terracotta block HLz e.g. acc. to DIN EN 771-1	2 DF (240x115x113)	$\geq 1,0$	12	H ²⁾	15 (22)	0,6
			8			0,4
Terracotta block Form B, HLz acc. to DIN 105, DIN EN 771-1 e.g. Schlagmann	12 DF (380x240x240)	$\geq 0,9$	8	R ²⁾	15 (20)	0,90
			6			0,6
			4			0,4
Terracotta block Form B, HLz acc. to DIN 105, DIN EN 771-1, Schlag- mann Planfüllziegel	12 DF (380x240x240)	$\geq 0,7$	6	R ²⁾	13 (2)	1,2
			4			0,75
			2			0,4
Hollow calcium silicate brick acc. to DIN 106, DIN EN 771-2 e.g. KS Wemding, KSL	5 DF (300x240x115)	$\geq 1,4$	16	H ²⁾	13 (4)	2,0
			6			0,75
	P10 (495x98x248)	$\geq 1,2$	6		13 (5)	1,2 (1,5) ⁴⁾
			2			0,4 (0,5) ⁴⁾
	3 DF (240x175x113)	$\geq 1,4$	20		15 (21)	1,2 (1,5) ⁴⁾
			8			0,5 (0,6) ⁴⁾
Hollow calcium silicate brick acc. to DIN 106, DIN EN 771-2 e.g. KS Wemding, KSL	2 DF (240x115x113)	$\geq 1,4$	12	H ²⁾	13 (8)	2,0
			6			1,0
Hollow block lightweight concrete, acc. to NF-P 14- 301, EN 771-3, e.g. Sepa Parpaing	(500x200x200)	$\geq 0,9$	4	R ²⁾	14 (10)	0,3 (0,4) ⁴⁾
Hollow brick lightweight concrete, e.g. acc. to DIN 18151, DIN EN 771-3, e.g. KLB, Hbl	(240x240x360)	$\geq 1,0$	6	H ²⁾	15 (19)	1,5
Hollow brick lightweight concrete, e.g. acc. to DIN EN 771-3, e.g. Roadstone masonry	(440x210x215)	$\geq 1,2$	10	R ²⁾	15 (18)	2,5
			6			1,5
Partial safety factor ³⁾	γ_{Mm}					2,5

For footnotes 1), 2), 3) and 4) see Annex 6, Table 9.1!

BTI frame fixing ProCon SXR

Anchor type ProCon SXR 10:
Characteristic resistance in hollow or perforated masonry
(Use category "c")

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Table 10.1: ProCon SXR10 characteristic resistance F_{RK} in [kN] in solid masonry (use category "b")

Base material [Supplier Title]	Min. DF or min. size (L x W x H) [mm]	Bulk density class ρ [kg/dm ³]	Minimum compressive strength f_b [N/mm ²]	Drill method	Characteristic resistance F_{RK} ¹⁾ ProCon SXR 10 [kN]	
					50/80 °C	30/50 °C
Terracotta block, Mz e.g. acc. to DIN 105, DIN EN 771-1, e.g. Vollmeter, Schlagmann, Mz	NF (240x115x71)	$\geq 1,8$	20 [10] ⁴⁾	H ²⁾	3,0	3,5
			36		5,0	5,0
	3 DF (240x175x113)	$\geq 1,8$	20 [10] ⁴⁾	H ²⁾	2,0 4,0³⁾	2,0 4,5³⁾
Terracotta block, Mz e.g. acc. to DIN EN 771-1+ A1:2005, e.g. Wienerberger MS	DF (240x115x52)	$\geq 1,8$	20 [10] ⁴⁾	H ²⁾	2,0	2,0
			28		3,0	3,0
Terracotta block, Mz e.g. acc. to DIN 105, DIN EN 771-1	NF (240x111x71)	$\geq 1,8$	20 [10] ⁴⁾	H ²⁾	3,0	3,0
Calcium silicate solid brick KS e.g. acc. to DIN 106, DIN EN 771-2 e.g. KS Wemding, KS	NF (240x115x71)	$\geq 1,8$	20 [10] ⁴⁾	H ²⁾	2,5 4,0³⁾	2,5 4,0³⁾
			36	H ²⁾	3,0 5,0	3,5 5,0
	(175x500x235)	$\geq 2,0$	20 [10] ⁴⁾	H ²⁾	4,5	4,5
			28		5,0	5,0
Lightweight solid brick, e.g. acc. to DIN 18152 DIN EN 771-3 e.g. KLB V	2 DF (240x115x113)	$\geq 1,2$	2	H ²⁾	0,75 0,9³⁾	0,75 0,9³⁾
	(240x490x115)	$\geq 1,2$	2	H ²⁾	1,2	1,2
	(250x240x245)	$\geq 1,6$	6	H ²⁾	2,5	2,5
	(240x490x115)	$\geq 1,6$	8	H ²⁾	3,0	3,0
Solid block normal concrete VBN acc. to DIN 18153, DIN EN 771-3 e.g. Adolf Blatt, VBN	(246x240x245)	$\geq 1,8$	20 [10] ⁴⁾	H ²⁾	4,5	4,5
Partial safety factor ³⁾				γ_{Mm}	2,5	

1) Characteristic resistance F_{RK} for tension, shear or combined tension and shear loading.

The characteristic resistance is valid for single plastic anchor or for a group of two or four plastic anchors with a spacing equal or larger than the minimum spacing s_{min} according to Table 11. The specific conditions for the design method have to be considered according to chapter 4.2.1.5 of the ETA.

2) H = Hammer drilling, R = Rotary drilling

3) In absence of other national regulations

4) For $10 \text{ N/mm}^2 \leq f_b < 20 \text{ N/mm}^2$: $F_{RK}' = 0,7 \cdot F_{RK}$

5) Only for edge distance $c \geq 200 \text{ mm}$; intermediate values by linear interpolation

BTI frame fixing ProCon SXR

Anchor type ProCon SXR 10:
Characteristic resistance in hollow or perforated masonry
(Use category "c")

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Table 10.2: ProCon SXR 10 characteristic resistance F_{Rk} in [kN] in hollow or perforated masonry (use category "c")

Base material [Supplier Title]	Min. DF or min. size (L x W x H) [mm]	Bulk density class ρ [kg/dm ³]	Minimum compressive strength f_b [N/mm ²]	Drill method	Geometry Annex (Figure)	Characteristic resistance F_{Rk} ¹⁾ ProCon SXR 10 [kN]	
						50/80 °C	30/50 °C
Terracotta block Form B, HLz acc. to DIN 105, DIN EN 771-1 e.g. Wienerberger Hlz	2 DF (240x115x113)	≥ 1,0	20 [10] ⁴⁾	R ²⁾	13 (1)	2,0	2,0
		≥ 1,2				2,5	3,0 ⁵⁾
Terracotta block Form B, HLz acc. to DIN 105, DIN EN 771-1, e.g. Schlagmann Planfüllziegel	12 DF (380x240x240)	≥ 0,7	6	R ²⁾	13 (2)	2,0	2,0
Terracotta block Form B, HLz acc. to DIN 105, DIN EN 771-1 e.g. Schlagmann Poroton T14	(300x240x240)	≥ 0,7	6	R ²⁾	13 (3)	0,3	0,4
Terracotta block HLz e.g. acc. to DIN EN 771-1	2 DF (240x115x113)	≥ 1,0	12	R ²⁾	15 (22)	0,9	0,9
			10			0,75	0,75
			8			0,6	0,6
Terracotta block, HLz acc. DIN EN 771-1+A1:2005, e.g. Wienerberger BS	DF (240x110x52)	≥ 1,6	28	H ²⁾	15 (17)	2,5	2,5
			20 [10] ⁴⁾			2,0	2,0
Hollow calcium silicate brick, acc. to DIN 106, DIN EN 771-2 e.g. KS Wemding, KSL	5 DF (300x240x115)	≥ 1,4	16 [10]	H ²⁾	13 (4)	3,0	3,5 ⁵⁾
	P10 (495x98x248)	≥ 1,2	6	H ²⁾	13 (5)	1,5	1,5
Hollow calcium silicate brick acc. to DIN 106, DIN EN 771-2 e.g. KS Wemding, KSL	2 DF (240x115x113)	≥ 1,4	12	H ²⁾	13 (8)	2,0	2,5
			10			2,0	2,0
			8			1,5	1,5
Hollow brick lightweight concrete, e.g. acc. to DIN 18151, DIN EN 771-3, e.g. KLB, Hbl		≥ 1,2	2	R ²⁾	13 (6)	1,5	1,5
					12 (Table)		
Hollow brick lightweight concrete, e.g. acc. to DIN EN 771-3, e.g. Roadstone masonry	10 DF (440x210x215)	≥ 1,2	8	H ²⁾	15 (18)	2,5	2,5
			6			2,0	2,0
Hollow brick Normal concrete, e.g. acc. to DIN 18153, DIN EN 771-3, e.g. Adolf Blatt, Hbn	10 DF (300x240x240)	≥ 1,6	6	H ²⁾	13 (7)	2,5	2,5
Partial safety factor ³⁾	γ_{Mm}					2,5	

For footnotes 1), 2), 3) and 4) see Annex 8, Table 10.1

BTI frame fixing ProCon SXR

Anchor type ProCon SXR 10:
Characteristic resistance in hollow or perforated masonry
(Use category "c")

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Table 10.3: ProCon SXR 10 characteristic resistance F_{RK} in [kN] in solid masonry and hollow or perforated masonry (use categories "b" + "c")

Base material [Supplier Title]	Min. DF or min. size (L x W x H) [mm]	Bulk density class ρ [kg/dm ³]	Minimum compressive strength f_b [N/mm ²]	Drill method	Geometry Annex (Figure)	Characteristic resistance F_{RK} ¹⁾ ProCon SXR 10 [kN]	
						50/80 °C	30/50 °C
Solid brick, normal weight concrete, e.g. Tarmac	(440x215x100)	$\geq 1,8$	20 [10] ⁴⁾	H ²⁾	-	4,0	4,5
Solid brick, lightweight concrete, e.g. Tarmac	(440x215x100)	$\geq 1,4$	6	H ²⁾	-	2,0 2,5 ⁵⁾	2,0 2,5 ⁵⁾
Heat insulation block e.g. Gisoton WDB	(390x240x250)	$\geq 0,7$	2	H ²⁾	14 (9)	1,5	1,5
Hollow block, lightweight concrete, acc. to NF-P 14-301 EN 771-3, e.g. Sepa Parpaing	(500x200x200)	$\geq 0,9$	4	R ²⁾	14 (10)	0,9 1,2 ⁵⁾	0,9 1,5 ⁵⁾
Terracotta blocks, HLz acc. to NF-P 13-301 EN 771-1, e.g. Imerys Gelimatic	(270x200x500)	$\geq 0,6$	6	R ²⁾	14 (11)	0,6	0,6 0,75 ⁵⁾
Terracotta blocks, HLz acc. to NF-P 13-301 EN 771-1, e.g. Terreal Calibric	(500x200x314)	$\geq 0,7$	8	R ²⁾	14 (12)	0,6	0,6 0,75 ⁵⁾
Terracotta blocks Form B, HLz acc. to NF-P 13-301, EN 771-1, e.g. Imerys Optibric	(560x200x274)	$\geq 0,6$	10	R ²⁾	14 (13)	1,2	1,2
Terracotta block, HLz acc. to NF-P 13-301, EN 771-1, e.g. Bouyer Leroux BGV	(570x200x314)	$\geq 0,6$	6	R ²⁾	14 (14)	0,75 0,9 ⁵⁾	0,75 1,2 ⁵⁾
Terracotta block, HLz acc. to NF-P 13-301, EN 771-1, e.g. Wienerberger Porotherm 30 R	(370x300x249)	$\geq 0,7$	10	R ²⁾	14 (15)	0,5 0,6 ⁵⁾	0,5 0,6 ⁵⁾
Terracotta block Form B, HLz acc. NF-P 13-301 EN 771-1, e.g. Wienerberger Porotherm GF R20	(500x200x299)	$\geq 0,7$	10	R ²⁾	14 (16)	0,6 0,75 ⁵⁾	0,6 0,75 ⁵⁾
Partial safety factor ³⁾						γ_{Mm}	2,5

1) Characteristic resistance F_{RK} for tension, shear or combined tension and shear loading.
The characteristic resistance is valid for single plastic anchor or for a group of two or four plastic anchors with a spacing equal or larger than the minimum spacing s_{min} according to Table 11. The specific conditions for the design method have to be considered according to chapter 4.2.1.5 of the ETA.

2) H = Hammer drilling, R = Rotary drilling

3) In absence of other national regulations

4) For $10 \text{ N/mm}^2 \leq f_b < 20 \text{ N/mm}^2$: $F_{RK}' = 0,7 \cdot F_{RK}$

5) Only for edge distance $c \geq 200 \text{ mm}$; intermediate values by linear interpolation.

BTI frame fixing ProCon SXR

Anchor type ProCon SXR 10:
Charact. resistance in solid and hollow or perforated masonry
(Use category "b" + "c")

Annex 10

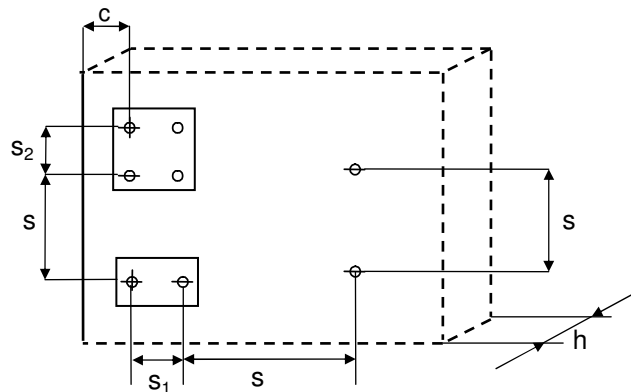
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Table 11: Minimum distances and dimensions in masonry

Anchor type			ProCon SXR 8	ProCon SXR 10
Minimum thickness of member	h_{min}	[mm]	100	100
Single anchor				
Minimum allowable spacing	a_{min}	[mm]	250	250
Minimum allowable edge distance	c_{min}	[mm]	100	100
Anchor Group				
Minimum allowable spacing perpendicular to free edge	$s_{1,min}$	[mm]	100	100
Minimum allowable spacing parallel to free edge	$s_{2,min}$	[mm]	100	100
Minimum allowable edge distance	c_{min}	[mm]	100	100

Scheme of distance and spacing in masonry



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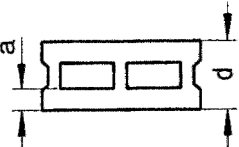
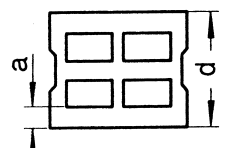
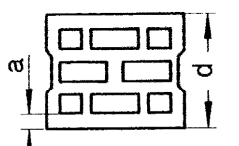
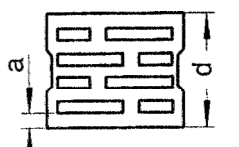
Minimum thickness of member,
minimum spacing and edge distances in masonry

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Table 12: Assignment type of anchor – geometry of bricks for lightweight concrete hollow blocks according to DIN 18151 DIN EN 771-3

Geometry	Thickness of brick d [mm]	Solid face shell in longitudinal direction a [mm]	Anchor type ProCon SXR 10 ProCon SXR 8
	175	50	●
	240 300	50	●
	240 300 365	35	●
	240 300 365	30	●

The anchor shall be placed in a way that the expanding part is anchored in the shell of the brick

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Anchorage in various kinds of masonry
Assignment type of anchor – lightweight concrete hollow bricks

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Table 13.1: Geometry of stones

<p>Figure 1</p>	<p>Figure 2</p>
<p>Figure 3</p>	<p>Figure 4</p>
<p>Figure 5</p>	<p>Figure 6</p>
<p>Figure 7</p>	<p>Figure 8</p>

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Geometry of stones

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Table 13.2: Geometry of stones

Figure 9

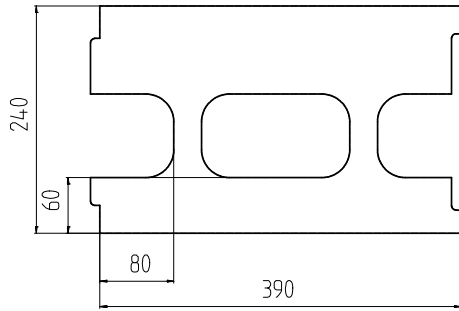


Figure 10

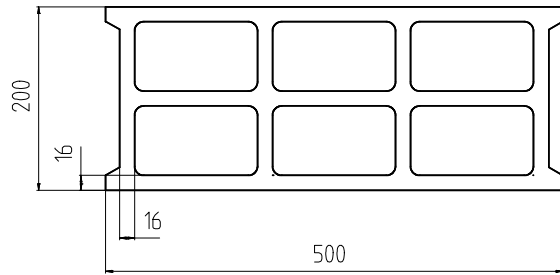


Figure 11

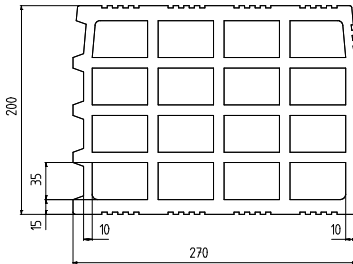


Figure 12

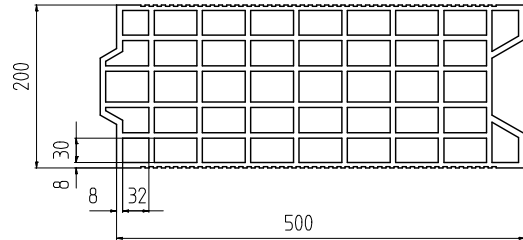


Figure 13

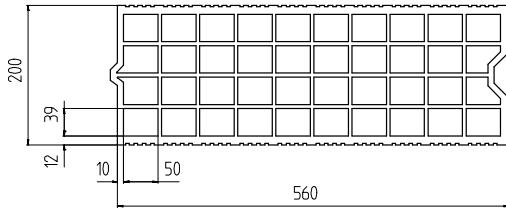


Figure 14

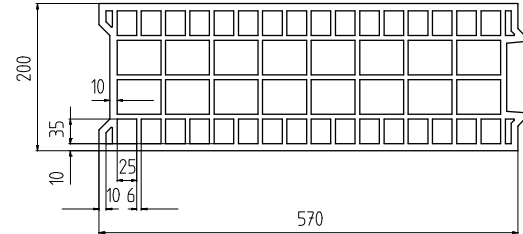


Figure 15

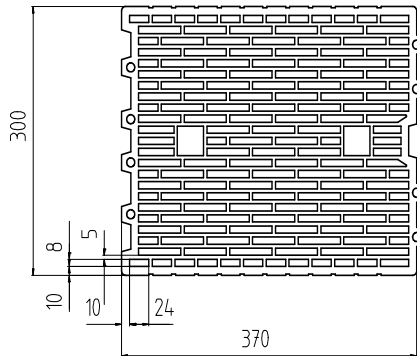
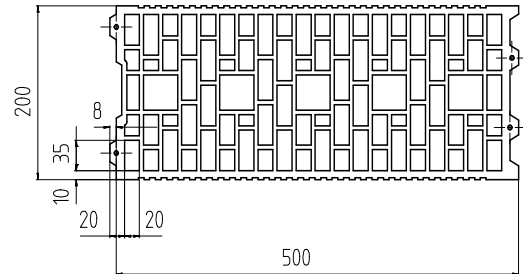


Figure 16



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Geometry of stones

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Table 13.3: Geometry of stones

Figure 17

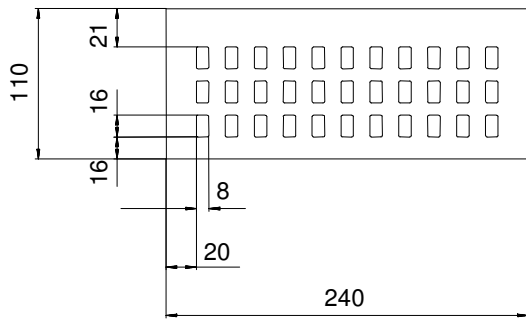


Figure 18

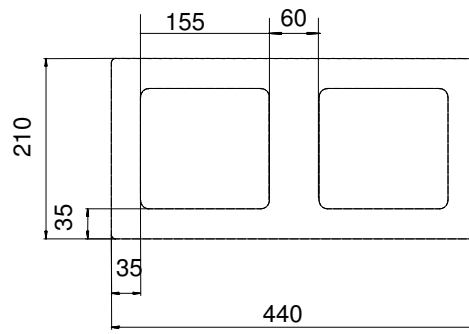


Figure 19

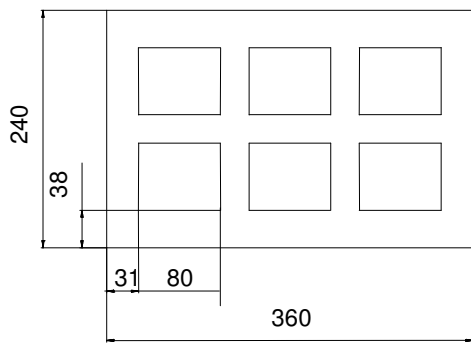


Figure 20

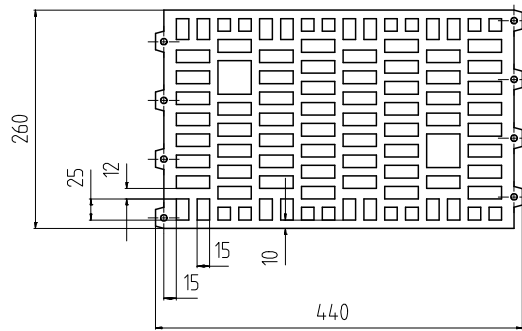


Figure 21

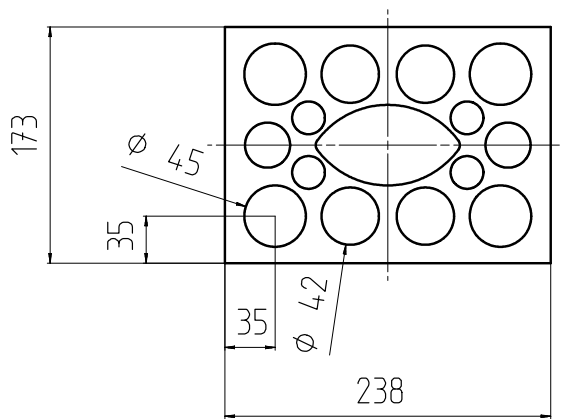
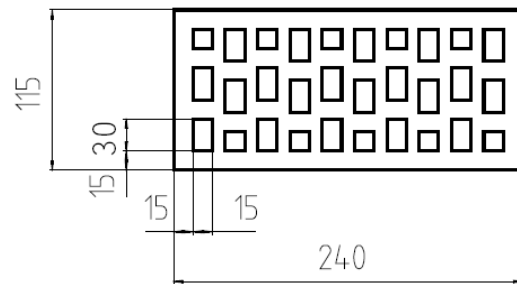


Figure 22



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Geometry of stones

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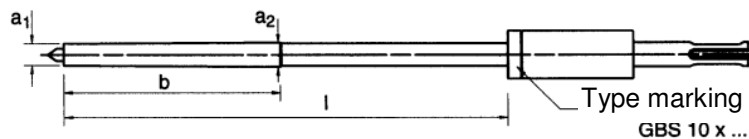
Table 14: ProCon SXR 10 characteristic resistance F_{RK} in [kN] in autoclaved aerated concrete (AAC) (use category "d")

Base material	Bulk density ρ [kg/m ³]	Minimum compressive strength f_{ck} [N/mm ²]	Drill method	Characteristic resistance F_{RK} ¹⁾ ProCon SXR 10 [kN]	
				50/80 °C	30/50 °C
Autoclaved aerated concrete blocks, e.g. PP acc. to DIN V 4165-100:2005-10, EN 771-4	≥ 350	2	with AAC Hole Punch ²⁾ , using the hammer drilling of the power drill	0,4	0,5
	≥ 500	4	Drill bit, rotary drilling	0,75	0,9
Partial safety factor³⁾	γ_{MAAC}			2,0	

- 1) Characteristic resistance F_{RK} for tension, shear or combined tension and shear loading.
The characteristic resistance is valid for single plastic anchor or for a group of two or four plastic anchors with a spacing equal or larger than the minimum spacing s_{min} according to Table 17. The specific conditions for the design method have to be considered according to chapter 4.2.1.5 of the ETA.
- 2) For the fixing in autoclaved aerated concrete with a nominal compressive strength $f_{ck} < 4$ N/mm² the hole is made by using the accompanying AAC Hole Punch according Table 15.
- 3) In absence of other national regulations

Table 15: Assignment AAC Hole Punch type – anchor type (length)

Type	AAC Hole Punch				Anchor type (length)
	a_1	a_2	b	l	
GBS 10 x 80	9	10	80	85	ProCon SXR 10 x 52 ProCon SXR 10 x 60 ProCon SXR 10 x 80
GBS 10 x 100				105	ProCon SXR 10 x 100
GBS 10 x 135			140	ProCon SXR 10 x 120	
GBS 10 x 160			90	165	ProCon SXR 10 x 140 ProCon SXR 10 x 160
GBS 10 x 185				190	ProCon SXR 10 x 180
GBS 10 x 230				235	ProCon SXR 10 x 200 ProCon SXR 10 x 230

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Anchor type ProCon SXR 10: Characteristic resistance in autoclaved aerated concrete (AAC - use category "d"), assignment AAC Hole Punch type – anchor type (length)

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Table 16: Displacements under tension und shear loading in autoclaved aerated concrete (AAC) ¹⁾

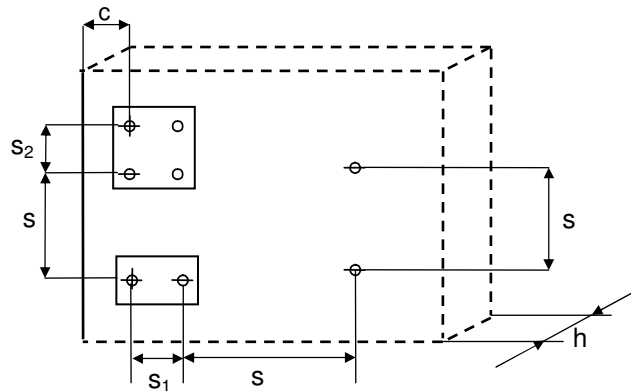
Anchor type	Tension load			Shear load		
	F ²⁾ [kN]	δ_{NO} [mm]	$\delta_{N\infty}$ [mm]	F ²⁾ [kN]	δ_{VO} [mm]	$\delta_{V\infty}$ [mm]
ProCon SXR 10	1,8	0,16	0,32	1,8	1,18	1,76

- 1) Valid for all ranges of temperatures
- 2) Intermediate values by linear interpolation

Table 17: Minimum distances and dimensions in AAC

Anchor type	ProCon SXR 10	
Minimum thickness of member	h_{min} [mm]	100
Single anchor		
Minimum allowable spacing	a_{min} [mm]	250
Minimum allowable edge distance	c_{min} [mm]	100
Anchor Group		
Minimum allowable spacing perpendicular to free edge	$s_{1,min}$ [mm]	200
Minimum allowable spacing parallel to free edge	$s_{2,min}$ [mm]	400
Minimum allowable edge distance	c_{min} [mm]	100

Scheme of distance and spacing in AAC



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Anchor type ProCon SXR 10:
Minimum thickness of member,
minimum spacing and edge distances in AAC

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