Deutsches Institut für Bautechnik

Zulassungsstelle für Bauprodukte und Bauarten

Bautechnisches Prüfamt

Eine vom Bund und den Ländern gemeinsam getragene Anstalt des öffentlichen Rechts

Kolonnenstraße 30 B D-10829 Berlin Tel.: +49 30 78730-0 Fax: +49 30 78730-320 E-Mail: dibt@dibt.de www.dibt.de





Mitglied der EOTA

Member of EOTA

European Technical Approval ETA-11/0514

English translation prepared by DIBt - Original version in German language

Handelsbezeichnung Trade name

Zulassungsinhaber Holder of approval

Zulassungsgegenstand und Verwendungszweck

Generic type and use of construction product

Geltungsdauer: *Validity:*

vom from bis

to

Herstellwerk

Manufacturing plant

MKT Injektionssystem VMU plus für Bewehrungsanschlüsse MKT Injection system VMU plus for rebar connections

MKT

Metall-Kunststoff-Technik GmbH & Co. KG

Auf dem Immel 2 67685 Weilerbach DEUTSCHLAND

Nachträglich eingemörtelter Bewehrungsanschluss mit dem

MKT Injektionssystem VMU plus

Post- installed rebar connection with MKT Injection System VMU plus

16 May 2013

16 May 2018

Werk 2, D

Diese Zulassung umfasst This Approval contains 20 Seiten einschließlich 11 Anhänge 20 pages including 11 annexes

Diese Zulassung ersetzt This Approval replaces ETA-11/0514 mit Geltungsdauer vom 28.11.2011 bis 06.10.2014 ETA-11/0514 with validity from 28.11.2011 to 06.10.2014



Europäische Organisation für Technische Zulassungen European Organisation for Technical Approvals



Page 2 of 20 | 16 May 2013

I LEGAL BASES AND GENERAL CONDITIONS

- 1 This European technical approval is issued by Deutsches Institut für Bautechnik in accordance with:
 - Council Directive 89/106/EEC of 21 December 1988 on the approximation of laws, regulations and administrative provisions of Member States relating to construction products¹, modified by Council Directive 93/68/EEC² and Regulation (EC) N° 1882/2003 of the European Parliament and of the Council³;
 - Gesetz über das In-Verkehr-Bringen von und den freien Warenverkehr mit Bauprodukten zur Umsetzung der Richtlinie 89/106/EWG des Rates vom 21. Dezember 1988 zur Angleichung der Rechts- und Verwaltungsvorschriften der Mitgliedstaaten über Bauprodukte und anderer Rechtsakte der Europäischen Gemeinschaften (Bauproduktengesetz - BauPG) vom 28. April 1998⁴, as amended by Article 2 of the law of 8 November 2011⁵;
 - Common Procedural Rules for Requesting, Preparing and the Granting of European technical approvals set out in the Annex to Commission Decision 94/23/EC⁶;
 - Guideline for European technical approval of "Metal anchors for use in concrete Part 5: Bonded anchors", ETAG 001-05.
- Deutsches Institut für Bautechnik is authorized to check whether the provisions of this European technical approval are met. Checking may take place in the manufacturing plant. Nevertheless, the responsibility for the conformity of the products to the European technical approval and for their fitness for the intended use remains with the holder of the European technical approval.
- This European technical approval is not to be transferred to manufacturers or agents of manufacturers other than those indicated on page 1, or manufacturing plants other than those indicated on page 1 of this European technical approval.
- This European technical approval may be withdrawn by Deutsches Institut für Bautechnik, in particular pursuant to information by the Commission according to Article 5(1) of Council Directive 89/106/EEC.
- Reproduction of this European technical approval including transmission by electronic means shall be in full. However, partial reproduction can be made with the written consent of Deutsches Institut für Bautechnik. In this case partial reproduction has to be designated as such. Texts and drawings of advertising brochures shall not contradict or misuse the European technical approval.
- The European technical approval is issued by the approval body in its official language. This version corresponds fully to the version circulated within EOTA. Translations into other languages have to be designated as such.

Official Journal of the European Communities L 40, 11 February 1989, p. 12

Official Journal of the European Communities L 220, 30 August 1993, p. 1

Official Journal of the European Union L 284, 31 October 2003, p. 25

⁴ Bundesgesetzblatt Teil I 1998, p. 812

⁵ Bundesgesetzblatt Teil I 2011, p. 2178

Official Journal of the European Communities L 17, 20 January 1994, p. 34



Page 3 of 20 | 16 May 2013

II SPECIFIC CONDITIONS OF THE EUROPEAN TECHNICAL APPROVAL

1 Definition of product and intended use

1.1 Definition of the construction product

The subject of this approval is the post-installed connection, by anchoring or overlap connection joint, of reinforcing bars (rebars) in existing structures made of normal weight concrete, using the "MKT Injection system VMU plus for rebar connection" in accordance with the regulations for reinforced concrete construction.

Reinforcing bars made of steel with a diameter d_s from 8 to 25 mm according to Annex 3 and MKT Injection Adhesive VMU plus are used for rebar connections. The rebar is placed into a drilled hole filled with injection mortar and is anchored via the bond between rebar, injection mortar and concrete.

1.2 Intended use

The rebar connection may be used in normal weight concrete of a minimum grade of C12/15 and maximum grade C50/60 according to EN 206-1:2000. It may be used in non-carbonated concrete with the allowable chloride content in concrete of 0.40% (CL 0.40) related to the cement content according to EN 206-1.

Rebar connections with reinforcing bars may be used for predominantly static loads.

The fire resistance of post-installed rebar connections is not covered by this European technical approval. Fatigue, dynamic or seismic loading of post-installed rebar connections are not covered by this European technical approval.

Rebar connections may only be carried out in a manner, which is also possible with cast-in straight reinforcing bars, e.g. those in the following applications (see Annexes 2):

- an overlap joint with existing reinforcement in a building component (Figures 1 and 2),
- anchoring of the reinforcement at a slab or beam support, (e.g. according to Figure 3: end support of a slab, designed simply supported, as well as an appropriate general reinforcement for restraint forces),
- anchoring of reinforcement of building components stressed primarily in compression (Figure 4),
- anchoring of reinforcement to cover the envelope line of tensile force in the bending member (Figure 5).

The post-installed rebar connections may be used in the temperature range of -40 $^{\circ}$ C to +80 $^{\circ}$ C (max short term temperature +80 $^{\circ}$ C and max long term temperature +50 $^{\circ}$ C).

This European technical approval covers anchoring in bore holes made with hammer drilling or compressed air drilling. The post-installed rebar connection may be installed in dry or wet concrete. It must not be installed in flooded holes.

The provisions made in this European technical approval are based on an assumed working life of the post-installed rebar connection of 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.



Page 4 of 20 | 16 May 2013

2 Characteristics of the product and methods of verification

2.1 Characteristics of the product

The post-installed rebar connection corresponds to the drawings and provisions given in Annexes 1 to 3. The characteristic material values, dimensions and tolerances not indicated in Annexes 1 to 3 shall correspond to the respective values laid down in the technical documentation⁷ of this European technical approval.

The two components of the injection adhesive are delivered in unmixed condition in coaxial cartridges or in side-by-side cartridges of sizes according to Annex 1. Each cartridge is marked with the identifying mark "MKT Injection Adhesive VMU plus" with the processing notes, charge code, shelf life, hazard code, curing time and processing time (depending on temperature).

The rebar shall comply with the specifications given in Annex 3.

2.2 Methods of verification

The assessment of fitness of the post-installed rebar connection for the intended use in relation to the requirements for mechanical resistance and stability and safety in use in the sense of the Essential Requirements 1 and 4 has been made in accordance with the "Guideline for European technical approval of Metal Anchors for Use in Concrete", Part 1 "Anchors in general" and Part 5 "Bonded anchors" and EOTA Technical Report TR 023 "Assessment of post-installed rebar connections".

In addition to the specific clauses relating to dangerous substances contained in this European technical approval, there may be other requirements applicable to the products falling within its scope (e.g. transposed European legislation and national laws, regulations and administrative provisions). In order to meet the provisions of the Construction Products Directive, these requirements need also to be complied with, when and where they apply.

3 Evaluation and attestation of conformity and CE marking

3.1 System of attestation of conformity

According to the Decision 96/582/EC of the European Commission⁹ system 2(i) (referred to as System 1) of the attestation of conformity applies.

This system of attestation of conformity is defined as follows:

System 1: Certification of the conformity of the product by an approved certification body on the basis of:

- (a) Tasks for the manufacturer:
 - (1) factory production control;
 - (2) further testing of samples taken at the factory by the manufacturer in accordance with a prescribed control plan;

The technical documentation of this European technical approval is deposited at the Deutsches Institut für Bautechnik and, as far as relevant for the tasks of the approved bodies involved in the attestation of conformity procedure, is handed over to the approved bodies.

The Technical Report TR 023 "Assessment of post-installed rebar connections" is published on EOTA website www.EOTA.eu.

Official Journal of the European Communities L 254 of 08.10.1996



Page 5 of 20 | 16 May 2013

- (b) Tasks for the approved body:
 - (3) initial type-testing of the product;
 - (4) initial inspection of factory and of factory production control;
 - (5) continuous surveillance, assessment and approval of factory production control.

Note: Approved bodies are also referred to as "notified bodies".

3.2 Responsibilities

3.2.1 Tasks for the manufacturer

3.2.1.1 Factory production control

The manufacturer shall exercise permanent internal control of production. All the elements, requirements and provisions adopted by the manufacturer shall be documented in a systematic manner in the form of written policies and procedures, including records of results performed. This production control system shall insure that the product is in conformity with this European technical approval.

The manufacturer may only use initial/raw/constituent materials stated in the technical documentation of this European technical approval.

The factory production control shall be in accordance with the control plan which is part of the technical documentation of this European technical approval. The control plan is laid down in the context of the factory production control system operated by the manufacturer and deposited with Deutsches Institut für Bautechnik.¹⁰

The results of factory production control shall be recorded and evaluated in accordance with the provisions of the control plan.

3.2.1.2 Other tasks for the manufacturer

The manufacturer shall, on the basis of a contract, involve a body which is approved for the tasks referred to in section 3.1 in the field of anchors in order to undertake the actions laid down in section 3.2.2 For this purpose, the control plan referred to in sections 3.2.1.1 and 3.2.2 shall be handed over by the manufacturer to the approved body involved.

The manufacturer shall make a declaration of conformity, stating that the construction product is in conformity with the provisions of this European technical approval.

3.2.2 Tasks for the approved bodies

The approved body shall perform the

- initial type-testing of the product,
- initial inspection of factory and of factory production control,
- continuous surveillance, assessment and approval of factory production control,

in accordance with the provisions laid down in the control plan.

The approved body shall retain the essential points of its actions referred to above and state the results obtained and conclusions drawn in a written report.

The approved certification body involved by the manufacturer shall issue an EC certificate of conformity of the product stating the conformity with the provisions of this European technical approval.

In cases where the provisions of the European technical approval and its control plan are no longer fulfilled the certification body shall withdraw the certificate of conformity and inform Deutsches Institut für Bautechnik without delay.

The control plan is a confidential part of the European technical approval and only handed over to the approved body involved in the procedure of attestation of conformity. See section 3.2.2.



Page 6 of 20 | 16 May 2013

3.3 CE marking

The CE marking shall be affixed on each packaging of the injection mortar. The letters "CE" shall be followed by the identification number of the approved certification body, where relevant, and be accompanied by the following additional information:

- the name and address of the holder of the approval (legal entity responsible for the manufacture),
- the last two digits of the year in which the CE marking was affixed,
- the number of the EC certificate of conformity for the product,
- the number of the European technical approval,
- the number of the guideline for European technical approval.

4 Assumptions under which the fitness of the product for the intended use was favourably assessed

4.1 Manufacturing

The European technical approval is issued for the product on the basis of agreed data/information, deposited with Deutsches Institut für Bautechnik, which identifies the product that has been assessed and judged. Changes to the product or production process, which could result in this deposited data/information being incorrect, should be notified to Deutsches Institut für Bautechnik before the changes are introduced. Deutsches Institut für Bautechnik will decide whether or not such changes affect the approval and consequently the validity of the CE marking on the basis of the approval and if so whether further assessment or alterations to the approval shall be necessary.

4.2 Drafting

Rebar connections must be designed in keeping with good engineering practice. Considering the loads to be anchored, design calculations and design drawings must be produced which can be checked. At least the following items must be stated in the design drawings:

- grade of concrete strength,
- diameter, drilling technique, concrete cover, spacing and embedment depth of the rebar,
- length ℓ_v and length for markings ℓ_m on the injection extension according to Annex 9,
- use of a guide device (drilling aid) for drilling holes close to edges (if necessary),
- kind of preparation of the joint between building component being connected including the diameter and thickness of concrete layer that has to be removed.

4.3 Design

4.3.1 General

The actual position of the reinforcement in the existing structure shall be determined on the basis of the construction documentation and taken into account when designing.

The design of post-installed rebar connections according to Annex 2 and determination of the internal section forces to be transferred in the construction joint shall be verified in accordance with EN 1992-1-1:2004. When ascertaining the tensile force in the rebar, allowance shall be made for the statically effective height of the bonded-in reinforcement.

The verification of the immediate local force transfer to the concrete has been provided.

The verification of the transfer of the loads to be anchored to the building component shall be provided.

The spacing between post-installed rebars shall be greater than the minimum of 5 d_s and 50 mm (see Annex 4).



Page 7 of 20 | 16 May 2013

4.3.2 Determination of the basic anchorage length

The required basic anchorage length $\ell_{b,rqd}$ shall be determined in accordance with EN 1992-1-1, Section 8.4.3:

 $\ell_{b.rqd}$ = (d_s / 4) (σ_{sd} / f_{bd})

with: d_s = diameter of the rebar

 σ_{sd} = calculated design stress of the rebar

f_{bd} = design value of bond strength according to Annex 5, Table 4
 in consideration of the coefficient related to the quality of bond conditions and of the coefficient related to the bar diameter and of the drilling technique

4.3.3 Determination of the design anchorage length

The required design anchorage length ℓ_{bd} shall be determined in accordance with EN 1992-1-1, Section 8.4.4:

 $\ell_{bd} = \alpha_1 \cdot \alpha_2 \cdot \alpha_3 \cdot \alpha_4 \cdot \alpha_5 \ \ell_{b,rqd} \ge \ell_{b,min}$

with: $\ell_{b,rqd}$ = according to section 4.3.2

 α_1 = 1.0 for straight bars

 $\alpha_2 = 0.7...1.0$ calculated acc. to EN 1992-1-1, Table 8.2

 α_3 = 1.0 because of no transverse reinforcement

 α_4 = 1.0 because of no welded transverse reinforcement

α₅ = 0.7...1.0 for influence of transverse pressure acc. to EN 1992-1-1, Table 8.2

 $\ell_{\rm b.min}$ = minimum anchorage length acc. to EN 1992-1-1

= max $\{0.3 \ell_{b,rad}; 10d_s; 100 \text{ mm}\}$ under tension

= max {0.6 $\ell_{b,rad}$; 10d_s; 100 mm} under compression

The maximum permissible embedment depth is given in Annex 5 depending on diameter of the rebar.

4.3.4 Overlap joints

The required design lap length ℓ_0 shall be determined in accordance with EN 1992-1-1, Section 8.7.3:

 $\ell_0 = \alpha_1 \cdot \alpha_2 \cdot \alpha_3 \cdot \alpha_5 \cdot \alpha_6 \ \ell_{b,rqd} \ge \ell_{0,min}$

with: $\ell_{b,rad}$ = according to Section 4.3.2

 α_1 = 1.0 for straight bars

 α_2 = 0.7...1.0 calculated acc. to EN 1992-1-1, Table 8.2

 α_3 = 1.0 because of no transverse reinforcement

 α_5 = 0.7...1.0 for influence of transverse pressure acc. to EN 1992-1-1, Table 8.2

 α_6 = 1.0...1.5 for influence of percentage of lapped bars relative to the total cross-section area acc. to EN 1992-1-1, Table 8.3

 $\ell_{0,min}$ = minimum lap length acc. to EN 1992-1-1

= max $\{0.3 \cdot \alpha_6 \ell_{b,rad}; 15d_s; 200 mm\}$

The maximum permissible embedment depth is given in Annex 5 depending on diameter of the rebar.



Page 8 of 20 | 16 May 2013

4.3.5 Embedment depth for overlap joints

For calculation of the effective embedment depth of overlap joints the concrete cover at end-face of bonded-in rebar c₁ shall be considered (see Annex 4, Figure 7):

 $\ell_{\rm v} \ge \ell_0 + c_1$

with: ℓ_0 = required lap length acc. to Section 4.3.4 and to EN 1992-1-1

c₁ = concrete cover at end-face of bonded-in rebar (see Annex 4, Figure 7)

If the clear distance between the overlapping rebars is greater than 4 d_s the lap length shall be enlarged by the difference between the clear distance and 4 d_s .

4.3.6 Concrete cover

The concrete cover required for bonded-in rebars is shown in Annex 5, Table 2, in relation to the drilling method and the hole tolerance.

Furthermore the minimum concrete cover given in EN 1992-1-1, Section 4.4.1.2 shall be observed.

4.3.7 Transverse reinforcement

The requirements of transverse reinforcement in the area of the post-installed rebar connection connection shall comply with EN 1992-1-1, Section 8.7.4.

4.3.8 Connection joint

The transfer of shear forces between new concrete and existing structure shall be designed according to EN 1992-1-1. The joints for concreting must be roughened to at least such an extent that aggregate protrude.

In case of a carbonated surface of the existing concrete structure the carbonated layer shall be removed in the area of the post-installed rebar connection with a diameter of d_s + 60 mm prior to the installation of the new rebar.

The depth of concrete to be removed shall correspond to at least the minimum concrete cover for the respective environmental conditions in accordance with EN 1992-1-1.

The foregoing may be neglected if building components are new and not carbonated and if building components are in dry conditions.

4.4 Installation

The fitness for use of the post-installed rebar connection can only be assumed if the rebar is installed as follows:

- the installation of post-installed rebar shall be done only by suitable trained installer and under supervision on site; the conditions under which an installer may be considered as suitable trained and the conditions for supervision on site are up to the Member States in which the installation is done,
- use of the injection system only as supplied by the manufacturer without exchanging the components of the Injection system,
- installation in accordance with the manufacturer's specifications and drawings using the tools indicated in the technical documentation of this European technical approval,
- checks before rebar installation to ensure that the strength class of the concrete in which the
 post-installed rebar connection is to be placed is in the range given and is not lower than
 that of the concrete to which the characteristic loads apply,
- check of concrete being well compacted, e.g. without significant voids,
- check the position of the existing rebars (if the position of existing rebars is not known, it shall be determined using a rebar detector suitable for this purpose as well as on the basis of the construction documentation and then marked on the building component for the overlap joint),



Page 9 of 20 | 16 May 2013

- keeping the anchorage depth as specified in the design drawings,
- keeping of concrete cover and spacing as specified in the design drawings,
- positioning of the drill holes without damaging the reinforcement,
- in case of aborted drill hole the drill hole shall be filled with mortar,
- the post-installed rebar connection must not be installed in flooded holes,
- the drilling and cleaning of the hole and the installation shall be performed only with the
 equipment specified by the manufacturer according to the manufacturer's installation
 instructions (see Annexes 6 to 10), it shall be ensured that this equipment is available on
 site and it is used,
- during curing of the injection mortar the temperature of the building component must not be less than -10 °C and no more than +40 °C; observing the curing time given in Annex 8.

5 Recommendations concerning packaging, transport and storage

5.1 Responsibility of the manufacturer

It is in the responsibility of the manufacturer to ensure that the information on the specific conditions according to sections 1 and 2 including Annexes referred to as wall as sections 4 and 5.2 is given to those who are concerned. This information may be made by reproduction of the respective parts of the European technical approval. In addition all installation data shall be shown clearly on the package and/or on an enclosed instruction sheet, preferably using illustration(s).

The minimum data required are:

- drill bit diameter,
- diameter of rebar,
- admissible service temperature range,
- curing time of the injection mortar,
- Installation instructions including cleaning of the drill hole,
- reference to any special installation equipment needed,
- identification of the manufacturing batch,

All data shall be presented in a clear and explicit form.

5.2 Packaging, transport and storage

The mortar cartridges shall be protected against sun radiation and shall be stored according to the manufacture's installation instructions in dry condition at temperatures of at least +5 °C to not more than +25 °C.

Mortar cartridges with expired shelf life must no longer be used.

Uwe Bender
Head of Department

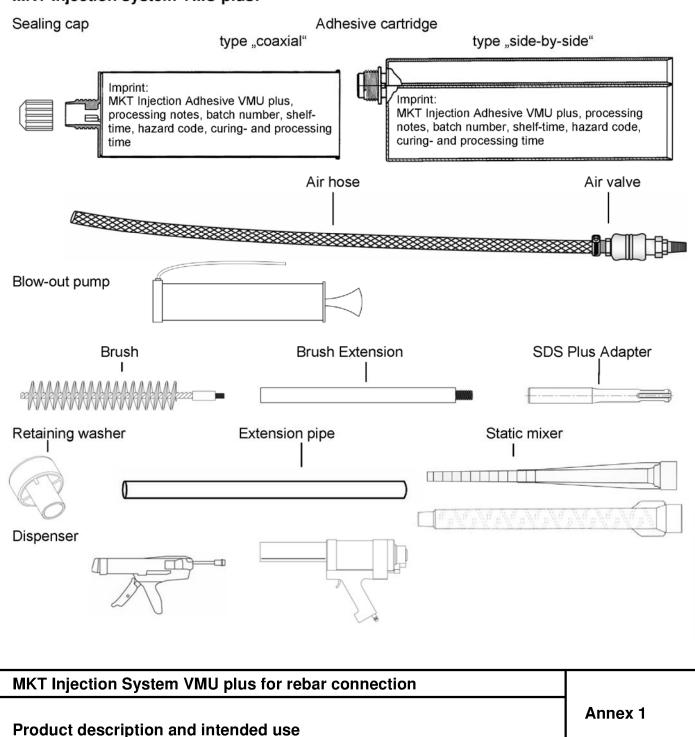
beglaubigt: Baderschneider



Product description and intended use

- Only for design of rebar connections based on EN 1992-1-1:2004
- Non carbonated concrete C12/15 C50/60 according to EN 206-1:2000
- Reinforcement bars, diameter Ø = 8 mm 25 mm according to Annex 3, Table 1
- Maximum long term temperature +50 °C, maximum short term temperature +80 °C
- Maximum anchorage length see Annex 5, Table 3
- Installation in dry or wet concrete, no installation in flooded holes







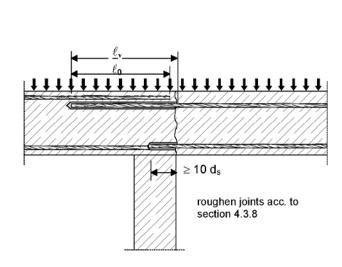


Figure 1: Overlap joint in slabs and beams

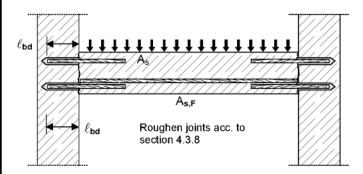


Figure 3: End anchoring of slabs or beams, designed as simply supported

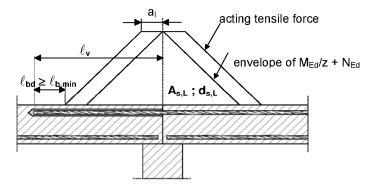


Figure 5: Anchoring of reinforcement to cover the line of acting tensile force

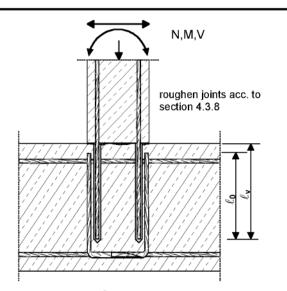


Figure 2: Overlap joint in a foundation of a column or wall where the rebars are stressed in tension

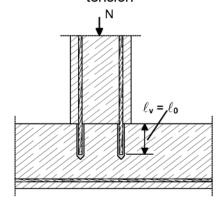


Figure 4: Rebar connection of components stressed primarily in compression. The rebars are stressed in compression

Notes to Figure 1 - 5:

No transverse reinforcement is plotted, the transverse reinforcement as required by EN 1992-1-1 shall be present. The shear transfer between old and new concrete shall be designed according to EN 1992-1-1.

Notations and definitions of anchorage

Notations and definitions of anchorage and overlap joints see Annex 4.

MKT Injection System VMU plus for rebar connection

Application examples of post-installed rebars

Annex 2

Deutsches Institut für **Bautechnik**

English translation prepared by DIBt



Figure 6: Properties of reinforcement bars "rebars"

Table 1: Materials

Abstract of EN 1992-1-1 Annex C, Table C.1, Properties of reinforcement

Product form		Bars and de-coiled rods		
Class		В	С	
Characteristic yield str	rength f _{yk} or f _{0,2k} (N/mm²)	400 to 600		
Minimum value of $k = (f_t / f_y)_k$		≥ 1,08	≥ 1,15 < 1,35	
Characteristic strain at maximum force ε _{uk} (%)		≥ 5,0	≥ 7,5	
Bendability		Bend / Re	ebend test	
Maximum deviation from nominal mass (individual bar) (%)	Nominal bar size (mm) ≤ 8 > 8	± 6,0 ± 4,5		

b) Abstract of EN 1992-1-1 Annex C, Table C.2N, Properties of reinforcement

Product form		Bars and de-coiled rods		
Class		В	С	
Minimum value of	Nominal diameter of the rebar (mm)			
related rib area f _{R,min}	8 to 12 > 12		040 056	

Rib height of the bar shall be in the range $0.05d \le h \le 0.07d$. (d: Nominal diameter of the bar; h: Rib height of the bar)

MKT Injection System VMU plus for rebar connection	
Properties of reinforcement	Annex 3



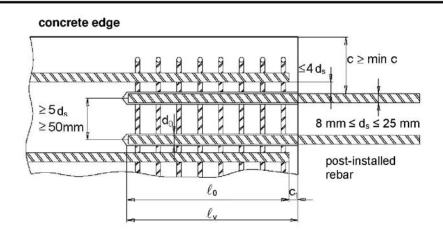


Figure 7: General rules for post-installed rebar connections

c concrete cover of post-installed reinforcement

concrete cover at front end of cast in place rebar min c minimum concrete cover according to Table 2

d_s diameter of post-installed rebar

 ℓ_0 lap length

 ℓ_{v} anchorage length

d₀ nominal drill bit diameter according to Annex 7, Table 5

The following notes must be observed:

- Bond strength f_{bd} according to EN 1992-1-1 may be applied
- ℓ_0 and ℓ_v according to sections 4.3.4 and 4.3.5
- Transverse reinforcement according to EN 1992-1-1.
- If the clear distance of overlapping bars is greater than 4 d_s, the lap length must be increased by a length equal to the clear space where it exceeds 4 d_s.
- The minimum concrete cover according to EN 1992-1-1 shall be kept.

MKT Injection System VMU plus for rebar connection

General rules for post-installed rebar connections

Annex 4



Table 2: Minimum concrete cover min c of post-installed rebars depending on drilling method and drilling tolerances

Drilling method	Rebar diameter	Without drilling aid device	With aid device
Hammer drilling	< 25 mm	30 mm+ 0,06 • ℓ _v ≥ 2 d _s	30 mm+ 0,02 • ℓ _v ≥ 2 d _s
Traininer drining	= 25 mm	40 mm+ 0,06 • ℓ _v ≥ 2 d _s	40 mm+ 0,02 • ℓ _v ≥ 2 d _s
Compressed air	< 25 mm	50 mm+ 0,08 • ℓ _v	50 mm+ 0,02 • ℓ _v
drilling	= 25 mm	60 mm+ 0,08 • ℓ _v	60 mm+ 0,02 • ℓ _v

The minimum concrete cover according to EN 1992-1-1:2004 shall be kept.

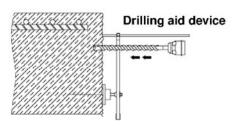


Table 3: Minimum anchorage length¹⁾ and lap length for C20/25 and maximum anchorage length I_{max}

Rod		l. [mm]	I _{0,min} [mm]	I _{max} [mm]	
Ø d _s [mm]	f _{y,k} [N/mm ²]	- I _{b,min} [mm]	10,min [11111]	'max L'''''J	
8	500	113	200	1000	
10	500	142	200	1000	
12	500	170	200	1200	
14	500	198	210	1400	
16	500	227	240	1600	
20	500	284	300	2000	
22	500	312	330	2000	
24	500	340	360	2000	
25	500	354	375	2000	

according to EN 1992-1-1: 2004: $I_{b,min}$ (8.6) and $I_{0,min}$ (8.11) for good bond conditions and α_6 = 1,0 for maximum yield strength of B500 B reinforcement bars and γ_M = 1,15

Table 4: Design values of ultimate bond stress f_{bd}²⁾

Concrete strength class	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
Design values of ultimate bond stress f _{bd} [N/mm²]	1,6	2,0	2,3	2,7	3,0	3,4	3,7	4,0	4,3

The values f_{bd} are valid for 'good' bond conditions according to EN 1992-1-1:2004. For all other bond conditions f_{bd} has to be multiplied by 0,7.

MKT Injection System VMU plus for rebar connection	
Minimum concrete cover, Minimum anchorage length, Design values of ultimate bond stress f _{bd}	Annex 5

Installation instructions

English translation prepared by DIBt

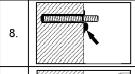


Ins	tallation instru	ıctions
1.	90"	Select drill bit diameter according to Table 5. Build drill hole by hammer drilling or compressed air drilling.
	57	Drill hole must be cleaned directly prior to installation of the anchor.
2a.	or	Starting from the bottom blow out the hole with compressed air (min. 6 bar) or blow-out pump at minimum four times. If the bore hole ground is not reaches an extension shall be used.
	4:4=	For drill holes deeper 240 mm compressed air (min. 6 bar) <u>must</u> be used.
2b.	₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩	Brush the drill hole at least 4x by using an appropriate sized wire brush according to Table 5 (the minimum brush diameter d _{b,min} has to be checked) with an drilling machine or a battery screwdriver. If the bore hole ground is not reached with the brush, a brush extension shall be used.
2c.	Or Or	Finally blow out the hole again with compressed air (min. 6 bar) or a blow-out pump a minimum of four times. If the bore hole ground is not reached an extension shall be used.
20.	4:4=4	For bore holes deeper 240 mm compressed air (min. 6 bar) <u>must</u> be used.
3.		Attach a supplied static-mixing nozzle to the cartridge and load the cartridge into the correct dispensing tool. For every working interruption longer than the recommended working time (Table 6) as well as for new cartridges, a new static-mixer shall be used.
4.	her S	Prior to inserting the anchor rod into the filled bore hole, the position of the anchorage length shall be marked on the rebar (e.g. with tape). Then insert the rebar in the empty hole in order to verify the correct hole depth.
5.	rain.3x	Prior to dispensing into the anchor hole, squeeze out separately a minimum of three full strokes and discard non-uniformly mixed adhesive components until the mortar shows a consistent grey colour.
6.	2/3	Starting from the bottom or back of the cleaned anchor hole fill the hole up to approximately two-thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid creating air pockets. For embedment larger than 190 mm an extension nozzle shall be used. For overhead and horizontal installation as well as in bore holes deeper 240 mm a piston plug and extension nozzle (Annex 9) shall be used. Observe the gel-/ working times given in Table 6.
7.		Push the reinforcing bar into the hole while turning slightly to ensure positive distribution of the adhesive until the anchorage length is reached.
		The reinforcing bar should be free of dirt, grease, oil or other foreign material.
MK	T Injection Sys	stem VMU plus for rebar connection

Z42601.13 8.06.01-522/12

Annex 6

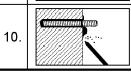
Installation instructions (continuation)



Be sure that the anchor is fully seated up to the full anchorage length and that excess mortar is visible at the top of the hole. If the hole is not completely filled, pull out anchor rod and start again from step no. 6. For overhead installation fix embedded part (e.g. wedges).



Allow the adhesive to cure to the specified time prior to applying any load or torque. Do not move or load the reinforcing bar until it is fully cured (attend Table 6).



Remove excess mortar.

Steel brush

9.



 Table 5:
 Parameters for cleaning and installation tools

d _s Rebar - Ø	d₀ Drill bit - Ø	d _b Brush - Ø	d _{b,min} min. Brush - Ø
[mm]	[mm]	[mm]	[mm]
8	12	14	12,5
10	14	16	14,5
12	16	18	16,5
14	18	20	18,5
16	20	22	20,5
20	25	27	25,5
22	28	30	28,5
24	32	34	32,5
25	32	34	32,5

MKT Injection System VMU plus for rebar connection

Installation instructions (continuation), Cleaning and setting tools

Annex 7

Z42601.13

Electronic copy of the ETA by DIBt: ETA-11/0514

Table 6: Maximum	processing time a	and minimum	curing time

Temperature of base material	Maximum processing time ¹⁾	Minimum curing time in dry concrete ⁵⁾
	t _{gel}	t _{cure,dry}
-10°C to -6°C	90 ²⁾ min	24 h
-5°C to -1°C	90 ³⁾ min	14 h
0°C to + 4°C	45 ³⁾ min	7 h
+ 5°C to + 9°C	25 ³⁾ min	2 h
+ 10°C to + 19°C	15 ³⁾ min	80 min
+ 20°C to + 24°C	6 ³⁾ min	45 min
+ 25°C to + 29°C	4 ³⁾ min	25 min
+ 30°C to + 40°C	2,5 ⁴⁾ min	15 min

t_{gel}: Maximum of time between the injection of the mortar and the end of the setting process.
 The cartridge temperature <u>must</u> be at minimum +15°C.
 The cartridge temperature <u>must</u> be between + 5°C and + 25°C.

Table 7: Dispensing tools

Electronic copy of the ETA by DIBt: ETA-11/0514

Cartridge type/size	Hand	l tool	Pneumatic tool		
Coaxial cartridges 150, 280, 300, 310, 330 ml	\/M_E	VM-P 330			
Coaxial cartridges 380, 410, 420 ml	VM-P 380	VM-P 380 Profi	VM-P 380 Pneumatic		
Side-by-side cartridges 235, 345 ml	VM-P 345	VM-P 345 Profi	VM-P 345 Pneumatic		
Side-by-side cartridge 865 ml	-	-	VM-P 865 Pneumatic		

MKT Injection System VMU plus for rebar connection

Processing time and curing time, Dispensing tools

Annex 8

⁴⁾ The cartridge temperature <u>must</u> be below + 20°C.

⁵⁾ The curing time t_{cure,dry} shall be doubled in wet concrete.



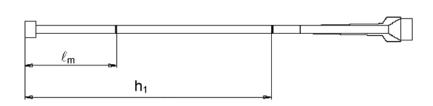


Figure 8: Marking lengths ℓ_{m} on extension pipe for injection of adhesive

Equation 1: $\ell_m = h_1 \cdot (1, 2 \cdot d_s^2 / d_0^2 - 0, 2)$ [mm]

As a rough estimate $\ell_{\rm m}$ = 1/3 h₁ can be assumed. Fill the hole with mortar until the marking of $\ell_{\rm m}$ is visible.

 ℓ_{m} distance from end of retaining washer to marking on extension pipe

 h_1 anchorage length = drill hole depth

d rebar diameter

d_o nominal drill bit diameter

Table 8: Retaining washer, maximum anchorage length and extension pipe

Rebar-Ø d₅	Drill bit - Ø		Retaining washer	coaxial (all)		ridge: , side-by-side 345 ml) Pneumatic dispenser		Cartridge: side-by-side (865 ml) Pneumatic dispenser	
	hammer drilled	compressed air drilled		I _{max}	Extension pipe	I _{max}	Extension pipe	I _{max}	Extension pipe
[mm]	[mm]		No.	[cm]		[cm]		[cm]	
8	12	-	-	VM-XLE 10		80	VM-XLE 10	80	VM-XLE 10
10	14	-	VM-IA 14		VM-XLE 10			100	
12		16	VM-IA 16			100		120	
14	18 20		VM-IA 18]		100		140	
16			VM-IA 20					160	
20	25	26	VM-IA 25		VM-XLE 10	70	VM-XLE 10		VM-XLE 16
22	28 32 32		VM-IA 28	50	VM-XLE 16	70	VM-XLE 16	200	
24			VM-IA 32			50			
25			VM-IA 32						

MKT Injection System VMU plus for rebar connection

Marking length,

Retaining washer, maximum anchorage length and extension pipe

Annex 9



Table 9: Values for pre-design of anchorings

Example for C20/25, good bond conditions, B500 B

	- Example for		orra corrara				
Rebar-∅	$\alpha_1 = \alpha_2$	$= \alpha_3 = \alpha_4 = \alpha_5 =$	1.0	α_2 or $\alpha_5 = 0.7$			
	₩ ₁ ₩ ₂ ₩ ₃ ₩ ₄ ₩ ₅ 1, 0			$\alpha_1 = \alpha_3 = \alpha_4 = 1,0$			
d_s	Anchorage	Design value	Mortar	Anchorage	Design value	Mortar	
	length I _{bd}	N _{Rd}	volume	length l _{bd}	N _{Rd}	volume	
[mm]	[mm]	[kN]	[ml]	[mm]	[kN]	[ml]	
[]	113*	6,53	9	113*	9,33	9	
8	180	10,40	14	150	12,39	11	
	250	14,45	19	190	15,69	14	
	378	21,85	29	265	21,88	20	
10	142*	10,26	13	142*	14,66	13	
	220	15,90	20	190	19,61	17	
	310	22,40	28	240	24,77	22	
	390	28,18	35	280	28,90	25	
	473	34,18	43	331	34,17	30	
	170*	14,74	18	170*	21,06	18	
	270	23,41	29	230	28,49	24	
12	370	32,08	39	280	34,68	30	
	470	40,75	50	340	42,12	36	
	567	49,16	60	397	49,18	42	
	198*	20,03	24	198*	28,61	24	
	310	31,36	37	260	37,57	31	
14	430	43,50	52	330	47,69	40	
	550	55,64	66	400	57,81	48	
	662	66,97	80	463	66,91	56	
	227*	26,24	31	227*	37,49	31	
	360	41,62	49	300	49,55	41	
16	490	56,65	67	380	62,76	52	
	620	71,68	84	450	74,32	61	
	756	87,40	103	529	87,37	72	
	284*	41,04	60	284*	58,63	60	
	450	65,03	95	380	78,45	81	
20	610	88,15	129	470	97,03	100	
	780	112,72	165	570	117,68	121	
	945	136,57	200	662	136,57	140	
22	312*	49,60	88	312*	70,85	88	
	490	77,89	139	420	95,38	119	
	680	108,10	192	520	118,09	147	
	860	136,71	243	620	140,80	175	
	1040	165,32	294	728	165,32	206	
24	340*	58,96	144	340*	84,23	144	
	540	93,64	228	450	111,48	190	
	740	128,33	312	570	141,21	241	
	940	163,01	397	680	168,46	287	
	1134	196,65	479	794	196,70	335	
25	354*	63,95	133	354*	91,35	133	
	560	101,16	211	470	121,29	177	
	770	139,09	290	590	152,26	222	
	970	175,22	365	710	183,22	267	
	1181	213,34	444	827	213,42	311	

^{*} minimum anchorage length see Annex 5, Table 3

The values apply to ,good' bonding conditions according to EN 1992-1-1. For all other bond conditions the values has to be multiplied by 0,7.

The mortar volume was calculated as follows: $V = 1.2 \cdot (d_0^2 - d_s^2) \cdot \pi \cdot l_b/4$

MKT Injection System VMU plus for rebar connection

Values for pre-design of anchorings Example for C20/25, good bond conditions, B500 B Annex 10



Table 10: Values for pre-design of overlap joints

Example for C20/25, ,good' bond conditions, B500 B

	· ·	· · · -		1	0.7		
Rebar-∅ d₅	$\alpha_1 = \alpha_2$	$= \alpha_3 = \alpha_4 = \alpha_5 =$	1,0	α_2 or $\alpha_5 = 0.7$ $\alpha_1 = \alpha_3 = \alpha_4 = 1.0$			
	Anchorage	Design value	Mortar	Anchorage	Design value	Mortar	
3	length l _{bd}	N _{Rd}	volume	length l _{bd}	N _{Rd}	volume	
[mm]	[mm]	[kN]	[ml]	[mm]	[kN]	[ml]	
8	200*	11,56	15	200*	16,62	15	
	240	13,87	18	220	18,17	17	
	290	16,76	22	230	18,99	17	
	378	21,85	29	265	21,88	20	
	200*	14,45	18	200*	20,64	18	
	270	19,51	24	230	23,74	21	
10	340	24,57	31	270	27,87	24	
	400	28,90	36	300	30,97	27	
	473*	34,18	43	331*	34,17	30	
	200*	17,34	21	200*	24,77	21	
	290	25,15	31	250	30,97	26	
12	380	32,95	40	300	37,16	32	
	480	41,62	51	350	43,35	37	
	567	49,16	60	397	49,18	42	
	210*	21,24	25	210*	30,35	25	
	320	32,37	39	270	39,02	33	
14	440	44,51	53	340	49,13	41	
	550	55,64	66	400	57,81	48	
	662	66,97	80	463	66,91	56	
	240*	27,75	33	240*	39,64	33	
	370	42,78	50	310	51,20	42	
16	500	57,81	68	380	62,76	52	
	630	72,83	86	460	75,97	62	
	756	87,40	103	529	87,37	72	
0.0	300*	43,35	64	300*	61,93	64	
	460	66,48	98	390	80,51	83	
20	620	89,60	131	480	99,09	102	
	780	112,72	165	570	117,68	121	
	945	136,57	200	662	136,67	140	
22	330*	52,46	93	330*	74,94	93	
	510	81,07	144	430	97,65	122	
	680	108,10	192	530	120,36	150	
	860	136,71	243	630	143,07	178	
	1040	165,32	294	728	165,32	206	
24	360*	62,43	152	360*	89,19	152	
	550	95,38	232	470	116,44	198	
	750	130,06	317	580	143,69	245	
	940	163,01	397	690	170,94	291	
	1134 275*	196,65	479	794	196,70	335	
	375*	67,74	141	375*	96,77	141	
25	580	104,77	218	490	126,45	184	
25	780	140,90	293	600	154,84	226	
	980	177,03	369	710	183,22	267	
	1181	213,34	444	827	213,42	311	

^{*} minimum anchorage length see Annex 5, Table 3

The values apply to 'good' bonding conditions according to EN 1992-1-1. For all other bond conditions the values has to be multiplied by 0,7.

The mortar volume was calculated as follows: $V = 1.2 \cdot (d_0^2 - d_s^2) \cdot \pi \cdot l_b/4$

MKT Injection System VMU plus for rebar connection

Values for pre-design of overlap joints Example for C20/25, good bond conditions, B500 B Annex 11