



Approval body for construction products and types of construction

#### **Bautechnisches Prüfamt**

An institution established by the Federal and Laender Governments



# European Technical Assessment

#### **General Part**

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

This version replaces

Deutsches Institut für Bautechnik

ETA-09/0148

of 4 July 2014

TILCA® TIM V+ Injection system for concrete

Bonded Anchor with Anchor rod for use in concrete

Egli, Fischer & Co. AG Gotthardstraße 6 8022 ZÜRICH SCHWEIZ

Egli, Fischer & Co. AG, Plant 1 Germany

27 pages including 3 annexes which form an integral part of this assessment

Guideline for European technical approval of "Metal anchors for use in concrete", ETAG 001 Part 5: "Bonded anchors", April 2013, used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011.

ETA-09/0148 issued on 21 June 2013

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# European Technical Assessment ETA-09/0148

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### Specific Part

### 1 Technical description of the product

The "TILCA® TIM V+ Injection system for concrete" is a bonded anchor consisting of a cartridge with injection mortar TILCA® TIM V+ and a steel element. The steel element consist of a commercial threaded rod with washer and hexagon nut in the range of M8 to M30 or a reinforcing bar in the range of diameter 8 to 32 mm.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between metal part, injection mortar and concrete.

The product description is given in Annex A.

# 2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

### 3 Performance of the product and references to the methods used for its assessment

### 3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance			
Characteristic resistance for tension loads in non-cracked concrete	See Annex C 1 / C 4 / C 7 / C 10			
Characteristic resistance for tension loads in cracked concrete	See Annex C 2 / C 5 / C 8 / C 11			
Characteristic resistance for shear loads in cracked and non-cracked concrete	See Annex C 3 / C 6 / C 9 / C 12			
Displacements under tension and shear loads	See Annex C 13 / C 14			

### 3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	No performance determined (NPD)

### 3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances there may be requirements (e.g. transposed European legislation and national laws, regulations and administrative provisions) applicable to the products falling within the scope of this European Technical Assessment. In order to meet the provisions of Regulation (EU) No 305/2011, these requirements need also to be complied with, when and where they apply.



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### 3.4 Safety in use (BWR 4)

The essential characteristics regarding Safety in use are included under the Basic Works Requirement Mechanical resistance and stability.

- 3.5 Protection against noise (BWR 5) Not applicable.
- 3.6 Energy economy and heat retention (BWR 6) Not applicable.

### 3.7 Sustainable use of natural resources (BWR 7)

The sustainable use of natural resources was not investigated.

### 3.8 General aspects

The verification of durability is part of testing the essential characteristics. Durability is only ensured if the specifications of intended use according to Annex B are taken into account.

# 4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

According to Decision of the Commission of 24 June 1996 (96/582/EC) (OJ L 254 of 08.10.96 p. 62-65), the system of assessment and verification of constancy of performance (see Annex V and Article 65 Paragraph 2 to Regulation (EU) No 305/2011) given in the following table applies.

Product	Intended use	Level or class	System
Metal anchors for use in concrete (heavy-duty type)	For fixing and/or supporting concrete structural elements or heavy units such as cladding and suspended ceilings	_	1

# Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at Deutsches Institut für Bautechnik.

Issued in Berlin on 4 July 2014 by Deutsches Institut für Bautechnik

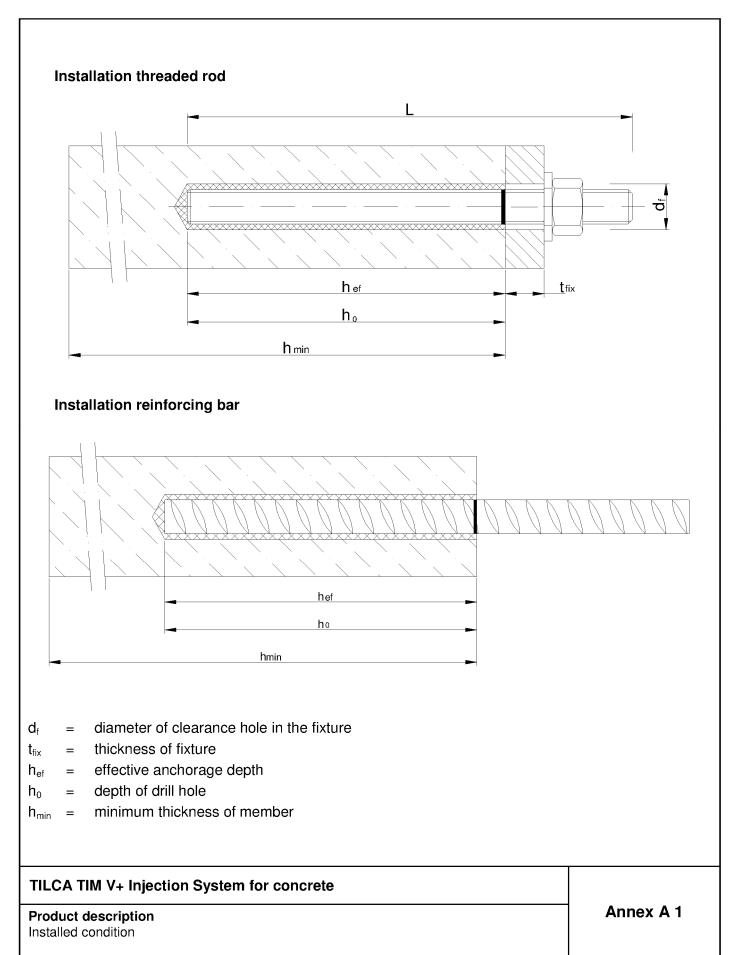
Uwe Bender Head of Department *Beglaubigt:* Baderschneider

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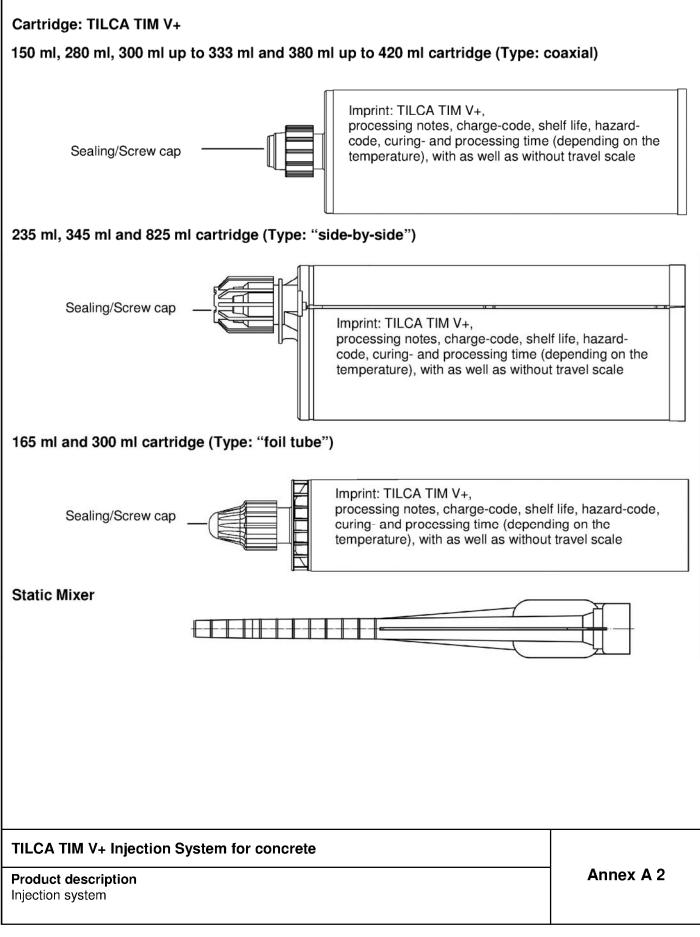
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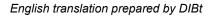
English translation prepared by DIBt



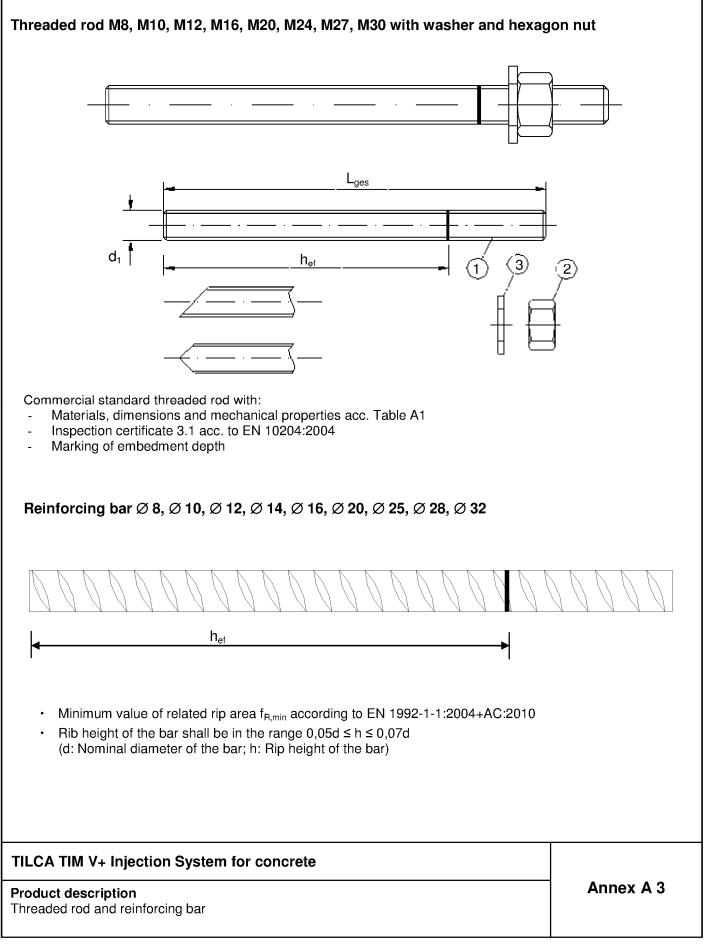














# Table A1: Materials

Part	Designation	Material					
	, zinc plated $\geq$ 5 µm acc. to EN ISO 4042:19						
	, bot-dip galvanised $\geq$ 40 µm acc. to EN ISO 4042.18		C:2009				
1	Anchor rod	Steel, EN 10087:1998 or EN 10263:200 Property class 4.6, 5.8, 8.8, EN 1993-1-8					
2	Hexagon nut, EN ISO 4032:2012	Steel acc. to EN 10087:1998 or EN 10263:2001 Property class 4 (for class 4.6 rod) EN ISO 898-2:2012, Property class 5 (for class 5.8 rod) EN ISO 898-2:2012, Property class 8 (for class 8.8 rod) EN ISO 898-2:2012					
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Steel, zinc plated or hot-dip galvanised					
Stain	less steel						
1	Anchor rod	Material 1.4401 / 1.4404 / 1.4571, EN 10 > M24: Property class 50 EN ISO 3506- ≤ M24: Property class 70 EN ISO 3506-	1:2009 1:2009				
2	Hexagon nut, EN ISO 4032:2012	Material 1.4401 / 1.4404 / 1.4571 EN 10 > M24: Property class 50 (for class 50 rc ≤ M24: Property class 70 (for class 70 rc	od) EN ISO 3506-2:2009				
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4401, 1.4404 or 1.4571, EN					
High	corrosion resistance steel						
1	Anchor rod	Material 1.4529 / 1.4565, EN 10088-1:2005, > M24: Property class 50 EN ISO 3506-1:2009 ≤ M24: Property class 70 EN ISO 3506-1:2009					
2	Hexagon nut, EN ISO 4032:2012	Material 1.4529 / 1.4565 EN 10088-1:20 > M24: Property class 50 (for class 50 rc ≤ M24: Property class 70 (for class 70 rc	005, od) EN ISO 3506-2:2009				
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4529 / 1.4565, EN 10088-1:20					
Reinf	forcing bars						
1	Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C $f_{yk}$ and k according to NDP or NCL of EN $f_{uk} = f_{tk} = k \cdot f_{yk}$	I 1992-1-1/NA:2013				
	CA TIM V+ Injection System for concre	te	Annex A 4				
Prod Mate	luct description prials						
41125 1	14		8 06 01-178/14				



## Specifications of intended use

### Anchorages subject to:

- Static and quasi-static loads: M8 to M30, Rebar Ø8 to Ø32.
- Seismic action for Performance Category C1: M12 to M30, Rebar Ø12 to Ø32.

#### **Base materials:**

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000.
- Strength classes C20/25 to C50/60 according to EN 206-1:2000.
- Non-cracked concrete: M8 to M30, Rebar Ø8 to Ø32.
- Cracked concrete: M12 to M30, Rebar Ø12 to Ø32.

#### **Temperature Range:**

- I: 40 °C to +40 °C (max long term temperature +24 °C and max short term temperature +40 °C) II: 40 °C to +80 °C (max long term temperature +50 °C and max short term temperature +80 °C)
- III: 40 °C to +120 °C (max long term temperature +72 °C and max short term temperature +120 °C)

### Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc coated steel, stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist
  - (high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

### Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to supports, etc.).
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Anchorages under static or quasi-static actions are designed in accordance with:
  - EOTA Technical Report TR 029 "Design of bonded anchors", Edition September 2010 or
  - CEN/TS 1992-4:2009
- Anchorages under seismic actions (cracked concrete) are designed in accordance with:
  - EOTA Technical Report TR 045 "Design of Metal Anchors under Seismic Action", Edition February 2013
  - Anchorages shall be positioned outside of critical regions (e.g. plastic hinges) of the concrete structure.
  - Fastenings in stand-off installation or with a grout layer are not allowed.

### Installation:

- Dry or wet concrete: M8 to M30, Rebar Ø8 to Ø32.
- Flooded holes (not sea water): M8 to M16, Rebar Ø8 to Ø16.
- Hole drilling by hammer or compressed air drill mode.
- Overhead installation allowed.
- Anchor installation carried out by appropriately gualified personnel and under the supervision of the person responsible for technical matters of the site.

### TILCA TIM V+ Injection System for concrete

# Intended Use

Specifications

Annex B 1



Anchor size		M 8	M 10	M 12	M 16	M 20	M 24	M 27	M 30
Nominal drill hole diameter	d <sub>0</sub> [mm] =	10	12	14	18	24	28	32	35
	h <sub>ef,min</sub> [mm] =	60	60	70	80	90	96	108	120
Effective anchorage depth	h <sub>ef,max</sub> [mm] =	160	200	240	320	400	480	540	600
Diameter of clearance hole in the fixture	d <sub>f</sub> [mm] ≤	9	12	14	18	22	26	30	33
Diameter of steel brush	d <sub>b</sub> [mm] ≥	12	14	16	20	26	30	34	37
Torque moment	T <sub>inst</sub> [Nm] ≤	10	20	40	80	120	160	180	200
Thickness of fixture	t <sub>fix,min</sub> [mm] >	0							
Thickness of fixture	t <sub>fix,max</sub> [mm] <	1500							
Minimum thickness of member	h <sub>min</sub> [mm]	h <sub>ef</sub> + 30 mm ≥ 100 mm				$h_{ef} + 2d_0$			
Minimum spacing	s <sub>min</sub> [mm]	40	50	60	80	100	120	135	150
Minimum edge distance	c <sub>min</sub> [mm]	40	50	60	80	100	120	135	150

# Table B2: Installation parameters for rebar

Rebar size			Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Nominal drill hole diameter	d <sub>0</sub> [mm] =	12	14	16	18	20	24	32	35	40
Effective encharage depth	h <sub>ef,min</sub> [mm] =	60	60	70	75	80	90	100	112	128
Effective anchorage depth	h <sub>ef,max</sub> [mm] =	160	200	240	280	320	400	480	540	640
Diameter of steel brush	d <sub>b</sub> [mm] ≥	14	16	18	20	22	26	34	37	41,5
Minimum thickness of member	h <sub>min</sub> [mm]	h <sub>ef</sub> + 30 mm ≥ 100 mm		h <sub>ef</sub> + 2d <sub>0</sub>						
Minimum spacing	s <sub>min</sub> [mm]	40	50	60	70	80	100	125	140	160
Minimum edge distance	c <sub>min</sub> [mm]	40	50	60	70	80	100	125	140	160

## TILCA TIM V+ Injection System for concrete

Intended Use Installation parameters Annex B 2



### Steel brush Table B3: Parameter cleaning and setting tools d<sub>b,min</sub> Threaded Piston d<sub>0</sub> $\mathbf{d}_{\mathsf{b}}$ Rebar min. Rod Drill bit - Ø Brush - Ø

			Diusii - Ø	Brush - Ø	picg
(mm)	(mm)	(mm)	(mm)	(mm)	(No.)
M8		10	12	10,5	
M10	8	12	14	12,5	
M12	10	14	16	14,5	No No
	12	16	18	16,5	<ul> <li>piston plug</li> <li>required</li> </ul>
M16	14	18	20	18,5	
	16	20	22	20,5	
M20	20	24	26	24,5	# 24
M24		28	30	28,5	# 28
M27	25	32	34	32,5	# 32
M30	28	35	37	35,5	# 35
	32	40	41,5	40,5	# 38





Recommended compressed air tool (min 6 bar) Drill bit diameter (d<sub>0</sub>): 10 mm to 40 mm

Hand pump (volume 750 ml) Drill bit diameter (d<sub>0</sub>): 10 mm to 20 mm





# TILCA TIM V+ Injection System for concrete

## Intended Use

Cleaning and setting tools

Annex B 3

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Installation inst	ructions
	1. Drill with hammer drill a hole into the base material to the size and embedment depth required by the selected anchor (Table B1 or Table B2). In case of aborted drill hole: the drill hole shall be filled with mortar
	Attention! Standing water in the bore hole must be removed before cleaning.
4x	2a. Starting from the bottom or back of the bore hole, blow the hole clean with compressed air (min. 6 bar) or a hand pump (Annex B 3) a minimum of four times. If the bore hole ground is not reached an extension shall be used.
or	The hand-pump can be used for anchor sizes up to bore hole diameter 20 mm.
4x)	For bore holes larger than 20 mm or deeper 240 mm, compressed air (min. 6 bar) <b>must</b> be used.
<u>********</u> **	<ul> <li>Check brush diameter (Table B3) and attach the brush to a drilling machine or a battery screwdriver. Brush the hole with an appropriate sized wire brush &gt; d<sub>b,min</sub> (Table B3) a minimum of four times. If the bore hole ground is not reached with the brush, a brush extension shall be used (Table B3).</li> </ul>
or	<ul> <li>Finally blow the hole clean again with compressed air (min. 6 bar) or a hand pump (Annex B 3) a minimum of four times. If the bore hole ground is not reached an extension shall be used.</li> <li>The hand-pump can be used for anchor sizes up to bore hole diameter 20 mm.</li> <li>For bore holes larger than 20 mm or deeper 240 mm, compressed air (min. 6 bar) must be used.</li> </ul>
	After cleaning, the bore hole has to be protected against re-contamination in an appropriate way, until dispensing the mortar in the bore hole. If necessary, the cleaning repeated has to be directly before dispensing the mortar. In-flowing water must not contaminate the bore hole again.
	3. Attach a supplied static-mixing nozzle to the cartridge and load the cartridge into the correct dispensing tool. Cut off the foil tube clip before use. For every working interruption longer than the recommended working time (Table B4) as well as for new cartridges, a new static-mixer shall be used.
heri	4. Prior to inserting the anchor rod into the filled bore hole, the position of the embedment depth shall be marked on the anchor rods.
min. 3 full stroke	5 Prior to dispensing into the anchor hole, squeeze out separately a minimum of three full strokes and discard non-uniformly mixed adhesive components until the mortar shows a consistent grey colour. For foil tube cartridges is must be discarded a minimum of six full strokes.
TILCA TIM V+ Inje	ction System for concrete

### Intended Use Installation instructions

Annex B 4



Installation inst	Installation instructions (continuation)							
	6 Starting from the bottom or back of the cleaned anchor hole fill the hole up to approximately two-thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid creating air pockets. For embedment larger than 190 mm an extension nozzle shall be used. For overhead and horizontal installation a piston plug (Annex B 3) and extension nozzle shall be used. Observe the gel-/ working times given in Table B4.							
	Push the threaded rod or reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached. The anchor should be free of dirt, grease, oil or other foreign material.							
	8. Be sure that the anchor is fully seated at the bottom of the hole and that excess mortar is visible at the top of the hole. If these requirements are not maintained, the application has to be renewed. For overhead application the anchor rod should be fixed (e.g. wedges).							
+20°C	9. Allow the adhesive to cure to the specified time prior to applying any load or torque. Do not move or load the anchor until it is fully cured (attend Table B4).							
Tinst.	<ol> <li>After full curing, the add-on part can be installed with the max. torque (Table B2) by using a calibrated torque wrench.</li> </ol>							

#### Table B4: Minimum curing time

Concrete temperature	Gelling- / working time	Minimum curing time in dry concrete <sup>2)</sup>
≥ -10 °C <sup>1)</sup>	90 min	24 h
≥ -5 °C	90 min	14 h
2° 0 ≤	45 min	7 h
≥ + 5 °C	25 min	2 h
≥ + 10 °C	15 min	80 min
≥ + 20 °C	6 min	45 min
≥ + 30 °C	4 min	25 min
≥ + 35 °C	2 min	20 min
≥ + 40 °C	1,5 min	15 min

<sup>1)</sup> Cartridge temperature <u>must</u> be at min. +15°C <sup>2)</sup> In wet concrete the curing time <u>must</u> be doubled

## TILCA TIM V+ Injection System for concrete

Intended Use Installation instructions (continuation) Curing time

Annex B 5



Anchor size threaded rod			M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30	
Steel failure											
Characteristic tension res Steel, property class 4.6	istance,	N <sub>Rk,s</sub>	[kN]	15	23	34	63	98	141	184	224
Characteristic tension res Steel, property class 5.8	istance,	N <sub>Fik,s</sub>	[kN]	18	29	42	78	122	176	230	280
Characteristic tension res Steel, property class 8.8	istance,	N <sub>Rk,s</sub>	[kN]	29	46	67	125	196	282	368	449
Characteristic tension res Stainless steel A4 and H0 property class 50 (>M24)	CR,	N <sub>Rk,s</sub>	[kN]	26	41	59	110	171	247	230	281
Combined pull-out and					•						
Characteristic bond resist	ance in non-cracked con	crete C20/2	25								
Temperature range I:	dry and wet concrete	τ <sub>Rk,ucr</sub>	[N/mm <sup>2</sup> ]	10	12	12	12	12	11	10	9
40°C/24°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm <sup>2</sup> ]	7,5	8,5	8,5	8,5		not adr	nissible	•
Temperature range II:	dry and wet concrete	τ <sub>Rk,ucr</sub>	[N/mm <sup>2</sup> ]	7,5	9	9	9	9	8,5	7,5	6,5
80°C/50°C	flooded bore hole	τ <sub>Rk,ucr</sub>	[N/mm <sup>2</sup> ]	5,5	6,5	6,5	6,5		not adr	nissible	
Temperature range III:	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm <sup>2</sup> ]	5,5	6,5	6,5	6,5	6,5	6,5	5,5	5,0
120°C/72°C	flooded bore hole	τ <sub>Rk,ucr</sub>	[N/mm <sup>2</sup> ]	4,0 5,0 5,0 5,0 not admissible							
l I		C30/37	•	1,04							
Increasing factors for con $\Psi_c$	crete	C40/50		1,08			08	3			
		C50/60		1,10							
Splitting failure		1									
Edge distance		C <sub>cr,sp</sub>	[mm]	$1,0 \cdot h_{ef} \le 2 \cdot h_{ef} \left(2,5 - \frac{h}{h_{ef}}\right) \le 2,4 \cdot h_{ef}$							
Axial distance		S <sub>cr,sp</sub>	[mm]				2 0	cr,sp			
Install safety factor (dry a	nd wet concrete)	γ2		1,0 1,2							
Install safety factor (flood	ed bore hole)	γ2			. 1	,4			not adr	nissible	
Install safety factor (flood	ed bore noie)	γ2		<u> </u>	1	,4			not aor	nissidie	

### Performances

Characteristic values of resistance for threaded rods under tension loads in non-cracked concrete (Design according to TR 029)  $\,$ 



Anchor size threaded	rod			M 12	M 16	M 20	M24	M 27	M 30
Steel failure								•	
Characteristic tension re Steel, property class 4.6	,	N <sub>Rk,s</sub> =N <sup>0</sup> <sub>Rk,s,seis</sub>	[kN]	34	63	98	141	184	224
Characteristic tension re Steel, property class 5.8	esistance,	N <sub>Rk,s</sub> =N <sup>0</sup> <sub>Rk,s,seis</sub>	[kN]	42	78	122	176	230	280
Characteristic tension re Steel, property class 8.8	sistance,	N <sub>Rk,s</sub> =N <sup>0</sup> <sub>Rk,s,seis</sub>	[kN]	67	125	196	282	368	449
Characteristic tension re Stainless steel A4 and H property class 50 (>M24	esistance, ICR,	N <sub>Rk,s</sub> =N <sup>0</sup> <sub>Rk,s,seis</sub>	[kN]	59	110	171	247	230	281
	l concrete cone failure								
Characteristic bond resi	stance in cracked concret	e C20/25							
		$ au_{ m Rk,cr}$	[N/mm <sup>2</sup> ]	5,5	5,5	5,5	5,5	6,5	6,5
Temperature range I:	dry and wet concrete	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	3,7	3,7	3,7	3,8	4,5	4,5
40°C/24°C		$ au_{ m Rk,cr}$	[N/mm <sup>2</sup> ]	5,5	5,5	not admissible			
	flooded bore hole	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	3,7	3,7	not admissible			
	dry and wet concrete	$ au_{Rk,cr}$	[N/mm <sup>2</sup> ]	4,0	4,0	4,0	4,0	4,5	4,5
Temperature range II:		$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	2,7	2,7	2,7	2,8	3,1	3,1
80°C/50°C	flooded bore hole	$ au_{Rk,cr}$	[N/mm <sup>2</sup> ]	4,0	4,0	not admissible			
		$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	2,7	2,7	not admissible			
	dry and wet concrete	$ au_{\mathrm{Rk,cr}}$	[N/mm <sup>2</sup> ]	3,0	3,0	3,0	3,0	3,5	3,5
Temperature range III:		$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	2,0	2,0	2,0	2,1	2,4	2,4
120°C/72°C		$ au_{Rk,cr}$	[N/mm <sup>2</sup> ]	3,0	3,0	not admissible			
	flooded bore hole	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	2,0	2,0	not admissible			
Increasing factors for co	ncrete	C30/37		1,04					
(only static or quasi-stat		C40/50		1,08					
ψ <sub>c</sub>		C50/60	C50/60			1,	10		
Splitting failure		1							
Edge distance		C <sub>cr,sp</sub>	[mm]		1,0 · h <sub>ef</sub> ≤	≤2·h <sub>ef</sub> (2	$5 - \frac{h}{h_{ef}}$	≤ 2,4 · h <sub>ef</sub>	
Axial distance		S <sub>cr,sp</sub>	[mm]			2 c	cr,sp		
Installation safety factor	(dry and wet concrete)	γ2				1	,2		
Installation safety factor	(flooded bore hole)	γ2		1	,4		not adr	missible	

### Performances

Characteristic values of resistance for threaded rods under tension loads in cracked concrete (Design according to TR 029 or TR 045)



Anchor size threaded rod			M 8	M 10	M 12	M 16	M 20	M24	M 27	М 30
Steel failure without lever arm										
Characteristic shear resistance.	$V_{Rk,s}$	[kN]	7	12	17	31	49	71	92	112
Steel, property class 4.6	$V^0_{Rk,s,seis}$	[kN]	-	-	12	22	34	50	65	78
Characteristic shear resistance.	V <sub>Rk,s</sub>	[kN]	9	15	21	39	61	88	115	140
Steel, property class 5.8	$V^0_{\ Rk,s,seis}$	[kN]	-	-	15	27	43	62	81	98
Characteristic shear resistance.	V <sub>Rk,s</sub>	[kN]	15	23	34	63	98	141	184	224
Steel, property class 8.8	$V^0_{Rk,s,seis}$	[kN]	-	-	24	44	69	99	129	157
Characteristic shear resistance,	$V_{Rk,s}$	[kN]	13	20	30	55	86	124	115	140
Stainless steel A4 and HCR, property class 50 (>M24) and 70 ( $\leq$ M24)	$V^0_{\ Rk,s,seis}$	[kN]	-	-	21	39	60	87	81	98
Steel failure with lever arm		1		1	1	I	1	I		
Characteristic bending moment.	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	15	30	52	133	260	449	666	900
Steel, property class 4.6	M <sup>0</sup> <sub>Rk,s,seis</sub>	[Nm]		1	No Perfe	ormance I	Determine	ed (NPD)	1	
Characteristic bending moment,	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	19	37	65	166	324	560	833	1123
Steel, property class 5.8	M <sup>0</sup> <sub>Rk,s,seis</sub>	[Nm]		1	No Perfe	ormance I	Determine	ed (NPD)	1	
Characteristic bending moment,	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	30	60	105	266	519	896	1333	1797
Steel, property class 8.8	M <sup>0</sup> <sub>Rk,s,seis</sub>	[Nm]		1	No Perfe	ormance I	Determine	ed (NPD)		
Characteristic bending moment,	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	26	52	92	232	454	784	832	1125
Stainless steel A4 and HCR, property class 50 (>M24) and 70 ( $\leq$ M24)	M <sup>0</sup> <sub>Rk,s,seis</sub>	[Nm]			No Perfe	ormance I	Determine	d (NPD)		
Concrete pry-out failure	1		I							
Factor k in equation (5.7) of Technical Report TR 029 for the design of Bonded Anchors	k	[-]				2	,0			
Installation safety factor	γ2	1				1	,0			
Concrete edge failure										
Installation safety factor	γ2					1	,0			

### Performances

Characteristic values of resistance for threaded rods under shear loads in cracked and non-cracked concrete, (Design according to TR 029 or TR 045)

Annex C 3

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	haracteristic val on-cracked conc							nsion	load	s in		
Anchor size reinforcir	ng bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Steel failure												
Characteristic tension r	esistance	N <sub>Rk,s</sub>	[kN]					$A_{s}\boldsymbol{\cdot}f_{uk}$				
Combined pull-out an	d concrete cone failure											
Characteristic bond res	istance in uncracked conc	rete C20/25	5									
Temperature range I:	dry and wet concrete	τ <sub>Rk,ucr</sub>	[N/mm²]	10	12	12	12	12	12	11	10	8,5
40°C/24°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm <sup>2</sup> ]	7,5	8,5	8,5	8,5	8,5		not adr	nissible	
Temperature range II:	dry and wet concrete	$\tau_{\rm Rk,ucr}$	[N/mm²]	7,5	9	9	9	9	9	8,0	7,0	6,0
80°C/50°C	flooded bore hole	τ <sub>Rk,ucr</sub>	[N/mm²]	5,5	6,5	6,5	6,5	6,5		not adr	nissible	
Temperature range III:	dry and wet concrete	$\tau_{\rm Rk,ucr}$	[N/mm²]	5,5	6,5	6,5	6,5	6,5	6,5	6,0	5,0	4,5
120°C/72°C	flooded bore hole	$\tau_{\rm Rk,ucr}$	[N/mm <sup>2</sup> ]	4,0	5,0	5,0	5,0	5,0		not adr	nissible	
		C30/37			•	•	•	1,04	•			
Increasing factors for $c_{0}$	oncrete	C40/50						1,08				
τu		C50/60						1,10				
Splitting failure												
Edge distance		C <sub>cr,sp</sub>	[mm]		1,	,0 ∙ h <sub>ef</sub>	≤2 · h <sub>et</sub>	2,5 -	$\frac{h}{h_{ef}} \le$	2,4 · h <sub>e</sub>	f	
Axial distance		S <sub>cr,sp</sub>	[mm]					2 c <sub>cr,sp</sub>				
Installation safety factor	r (dry and wet concrete)	γ2		1,0				1	,2			
Installation safety factor	r (flooded bore hole)	γ2				1,4				not adr	nissible	

## TILCA TIM V+ Injection System for concrete

Performances

Characteristic values of resistance for rebar under tension loads in non-cracked concrete (Design according to TR 029)



bar sistance concrete cone failure tance in cracked concrete dry and wet concrete	$N_{RK,s} = N_{RK,s,seis}^{0}$ $= C20/25$ $\tau_{RK,cr}$	[kN]	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
concrete cone failure	e C20/25	[kN]							<u> </u>
concrete cone failure	e C20/25	[kN]							
tance in cracked concrete						$A_{s} \boldsymbol{\cdot} f_{uk}$			
		1							
dry and wet concrete	$ au_{Rk,cr}$								
dry and wet concrete		[N/mm <sup>2</sup> ]	5,5	5,5	5,5	5,5	5,5	6,5	6,5
1	$\tau^0_{Rk,seis}$	[N/mm²]	3,7	3,7	3,7	3,7	3,7       3,8       4,5         not admissible         4,0       4,0       4,5         4,0       4,0       4,5         2,7       2,8       3,1         not admissible         not admissible         3,0       3,0       3,5         2,0       2,1       2,4         not admissible         not admissible         1,04       1,08	4,5	
flooded bore hole	$\tau_{\rm Rk,cr}$	[N/mm²]	5,5	5,5	5,5		5       5,5       6,5         7       3,8       4,5         not admissible       not admissible         0       4,0       4,5         7       2,8       3,1         not admissible       not admissible         0       3,0       3,5         0       2,1       2,4         not admissible       not admissible         0       2,1       2,4         1       not admissible         0       3,0       3,5         0       2,1       2,4         1       not admissible       1         1       1       1         1       1       1         1       1       2         1       1       2         1       1       2         1       1       2         1       1       2         1       1       2         1       1       2         1       1       2         1       1       3         1       1       3         1       1       3         1       1       3		
liooded bore hole	$\tau^0_{\ Rk,seis}$	[N/mm²]	3,7	3,7	3,7		5       5,5       6,5         7       3,8       4,5         not admissible       not admissible         0       4,0       4,5         7       2,8       3,1         not admissible       not admissible         0       3,0       3,5         0       2,1       2,4         not admissible       not admissible         0       2,1       2,4         1       not admissible         0       3,0       3,5         0       2,1       2,4         1       not admissible       1         1       1       1         1       1       1         1       1       2         1       1       2         1       1       2         1       2       1         2       1       2         1       1       1         1       1       2         1       1       2         1       1       2         1       1       2         1       1       2         1       1       2		
	τ <sub>Rk,cr</sub>	[N/mm <sup>2</sup> ]	4,0	4,0	4,0	4,0	5       5,5       6,5         7       3,8       4,5         not admissible       not admissible         0       4,0       4,5         7       2,8       3,1         not admissible       not admissible         0       3,0       3,5         0       2,1       2,4         not admissible       not admissible         0       2,1       2,4         10	4,5	
dry and wet concrete	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	2,7	2,7	2,7	2,7	3,7     3,8     4,5       not admissible       not admissible       4,0     4,0       4,0     4,5       2,7     2,8       3,0     3,0       3,0     3,0       3,0     2,1       2,0     2,1       2,1     2,4	3,1	
	$\tau_{Rk,cr}$	[N/mm²]	4,0	4,0	4,0		3,7       3,8       4,5         not admissible       not admissible         1,0       4,0       4,5         2,7       2,8       3,1         2,7       2,8       3,1         not admissible       not admissible         3,0       3,0       3,5         2,0       2,1       2,4         not admissible       not admissible         1,0       1,2,4         1,0       1,2,4         1,0       1,04         1,08       1,04		
flooded bore hole	$\tau^0_{Rk,seis}$	[N/mm²]	2,7	2,7	2,7				
	τ <sub>Rk,cr</sub>	[N/mm²]	3,0	3,0	3,0	3,0	3,0	3,5	3,5
dry and wet concrete	$\tau^0_{Rk,seis}$	[N/mm²]	2,0	2,0	2,0	2,0	2       3,8       4,5         not admissible       not admissible         not admissible	2,4	
	$\tau_{\rm Rk,cr}$	[N/mm²]	3,0	3,0	3,0		not admissible04,04,572,83,172,83,10admissible03,03,502,12,40admissible0not admissible02,12,41not admissible		
flooded bore hole	$\tau^0_{Rk,seis}$	[N/mm²]	2,0	2,0	2,0		not adr	nissible	
	C30/37	1		1		1,04	not admissible not admissible 4,0 4,5 2,8 3,1 not admissible not admissible 3,0 3,5 2,1 2,4 not admissible not admissible		
crete cactions)	C40/50					1,08	not admissible not admissible 4,0 4,5 2,8 3,1 not admissible not admissible 3,0 3,5 2,1 2,4 not admissible not admissible		
	C50/60					1,10			
	C <sub>cr,sp</sub>	[mm]		1,0 · h <sub>e</sub>	<sub>f</sub> ≤2 ⋅ h	ef (2,5 -	$\left(\frac{h}{h_{ef}}\right) \le 2$	,4 ⋅ h <sub>ef</sub>	
	S <sub>cr,sp</sub>	[mm]				2 c <sub>cr,sp</sub>	,		
dry and wet concrete)	γ2					1,2			
flooded bore hole)	γ2			1,4			not adn	nissible	
	actions) dry and wet concrete) flooded bore hole)	$\frac{dry and wet concrete}{dry and wet concrete} = \frac{\tau_{Pik,cr}}{\tau_{Pik,seis}^{0}}$ flooded bore hole $\frac{\tau_{Pik,seis}^{0}}{\tau_{Pik,seis}^{0}}$ $\frac{dry and wet concrete}{flooded bore hole} = \frac{\tau_{Pik,cr}}{\tau_{Pik,seis}^{0}}$ flooded bore hole $\frac{\tau_{Pik,seis}^{0}}{\tau_{Pik,seis}^{0}}$ $\frac{\tau_{Pik,seis}}{\tau_{Pik,seis}^{0}}$	$\frac{\tau_{\text{Rk,cr}}}{\tau_{\text{Rk,cr}}} \qquad [\text{N/mm}^2]$ $\frac{\tau_{\text{Rk,cr}}}{\tau_{\text{Rk,seis}}} \qquad [\text{N/mm}^2]$ $\frac{\tau_{\text{Rk,cr}}}{\tau_{\text{Rk,cr}}} \qquad [\text{N/mm}^2]$ $\frac{\tau_{\text{Rk,cr}}}{\tau_{\text{Rk,cr}}} \qquad [\text{N/mm}^2]$ $\frac{\tau_{\text{Rk,cr}}}{\tau_{\text{Rk,seis}}} \qquad [\text{N/mm}^2]$ $\frac{\tau_{\text{Rk,cr}}}{\tau_{\text{Rk,cr}}} \qquad [\text{Rmm}^2]$	${\text{trik,cr}} \qquad [N/mm^2] \qquad 4,0$ ${\text{trik,cr}} \qquad [N/mm^2] \qquad 2,7$ ${\text{flooded bore hole}} \qquad {\text{trik,cr}} \qquad [N/mm^2] \qquad 2,7$ ${\text{flooded bore hole}} \qquad {\text{trik,cr}} \qquad [N/mm^2] \qquad 2,7$ ${\text{trik,cr}} \qquad [N/mm^2] \qquad 2,7$ ${\text{trik,cr}} \qquad [N/mm^2] \qquad 3,0$ ${\text{trik,cr}} \qquad [N/mm^2] \qquad 2,0$	$\begin{array}{c c c c c c c } \mbox{track} & \begin{tabular}{ c c c c } \hline $T_{Bk,cr}$ & [N/mm^2] & 4,0 & 4,0 \\ \hline $t^0_{Bk,sels}$ & [N/mm^2] & 2,7 & 2,7 \\ \hline $t_{Bk,cr}$ & [N/mm^2] & 4,0 & 4,0 \\ \hline $t^0_{Bk,sels}$ & [N/mm^2] & 2,7 & 2,7 \\ \hline $t^0_{Bk,sels}$ & [N/mm^2] & 3,0 & 3,0 \\ \hline $t^0_{Bk,sels}$ & [N/mm^2] & 2,0 & 2,0 \\ \hline $t^0_{Bk,sels}$ & [N/mm^2] & 3,0 & 3,0 \\ \hline $t^0_{Bk,sels}$ & [N/mm^2] & 2,0 & 2,0 \\ \hline $t^0_{Bk,sels}$ & [N/mm^2] & [N/mm^2] & [N/mm^2] & [N/mm^2] & [N/mm^2] \\ \hline $t^0_{Bk,sels}$ & [N/mm^2] & [N$	$\begin{array}{c c c c c c c } \mbox{Tell,cr} & [N/mm^2] & 4,0 & 4,0 & 4,0 \\ \hline $\tau^0_{\mbox{PB,sels}} & [N/mm^2] & 2,7 & 2,7 & 2,7 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & 4,0 & 4,0 & 4,0 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & 4,0 & 4,0 & 4,0 \\ \hline $\tau^0_{\mbox{PB,sels}} & [N/mm^2] & 2,7 & 2,7 & 2,7 & 2,7 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & 3,0 & 3,0 & 3,0 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & 2,0 & 2,0 & 2,0 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & 2,0 & 2,0 & 2,0 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & 3,0 & 3,0 & 3,0 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & 3,0 & 3,0 & 3,0 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & 2,0 & 2,0 & 2,0 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & [N/mm^2] & 2,0 & 2,0 & 2,0 \\ \hline $t^0_{\mbox{PB,sels}} & [N/mm^2] & [$	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	$\begin{array}{c c c c c c c c c c } \mbox{T} Bk,cr & [N/mm^2] & 4,0 & 4,0 & 4,0 & 4,0 & 4,0 & 4,0 & 4,0 & 4,0 & 4,0 & 1,0 & $	$\begin{array}{c c c c c c c } \mbox{transmit $Trik, cr$} & [N/mm^2] & 4,0 & 5,0 & $

# Performances

Characteristic values of resistance for rebar under tension loads in cracked concrete (Design according to TR 029 or TR 045)



Table C6: Characterist and non-crac										racke	d
Anchor size reinforcing bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Steel failure without lever arm			1					1			
Characteristic shear resistance	V <sub>Rk,s</sub>	[kN]				0,	50 • A <sub>s</sub> •	f <sub>uk</sub>			
Characteristic shear resistance	$V^0_{Rk,s,seis}$	[kN]				0,	35 • A₅ •	f <sub>uk</sub>			
Steel failure with lever arm											
Characteristic bending moment	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]				1.	2 ∙ W <sub>el</sub> ∙	f <sub>uk</sub>			
Characteristic bending moment	$M^0_{Rk,s,seis}$	[Nm]			No F	Performa	nce Dete	rmined (I	NPD)		
Concrete pry-out failure											
Factor k in equation (5.7) of Technical Report TR 029 for the design of bonded anchors	k	[-]					2,0				
Installation safety factor	γ2						1,0				
Concrete edge failure											
Installation safety factor	γ2						1,0				

## Performances

Characteristic values of resistance for rebar under shear loads in cracked and non-cracked concrete, (Design according to TR 029 or TR 045)



Anchor size threaded rod				M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Steel failure											
Characteristic tension resis Steel, property class 4.6	tance,	N <sub>Rk,s</sub>	[kN]	15	23	34	63	98	141	184	224
Characteristic tension resis	tance,	N <sub>Rk,s</sub>	[kN]	18	29	42	78	122	176	230	280
Steel, property class 5.8 Characteristic tension resis	tance,	N <sub>Bks</sub>	[kN]	29	46	67	125	196	282	368	449
Steel, property class 8.8 Characteristic tension resis	tance.	IN <sub>Rk,s</sub>	נגואן	29	40	07	120	190	202	300	449
Stainless steel A4 and HCI property class 50 (>M24) a	<b>λ</b> ,	$N_{Rk,s}$	[kN]	26	41	59	110	171	247	230	281
Combined pull-out and co											
	nce in non-cracked concret	e C20/25									
Temperature range I:	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm <sup>2</sup> ]	10	12	12	12	12	11	10	9
40°C/24°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm <sup>2</sup> ]	7,5	8,5	8,5	8,5		not adr	nissible	
Temperature range II:	dry and wet concrete	$\tau_{\rm Rk,ucr}$	[N/mm <sup>2</sup> ]	7,5	9	9	9	9	8,5	7,5	6,5
80°C/50°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm <sup>2</sup> ]	5,5	6,5	6,5	6,5		not adr	nissible	1
Temperature range III:	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm <sup>2</sup> ]	5,5	6,5	6,5	6,5	6,5	6,5	5,5	5,0
120°C/72°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm <sup>2</sup> ]	4,0	5,0	5,0	5,0		not adr	nissible	
		C30/37					1,	04			
Increasing factors for conci $\Psi_c$	rete	C40/50					1,	08			
		C50/60					1,	10			
Factor according to CEN/TS 1992-4-5 Section	6.2.2.3	k <sub>8</sub>	[-]				10	),1			
Concrete cone failure											
Factor according to CEN/TS 1992-4-5 Section	6.2.3.1	k <sub>ucr</sub>	[-]				10	),1			
Edge distance		C <sub>cr,N</sub>	[mm]				1,5	h <sub>ef</sub>			
Axial distance		S <sub>cr,N</sub>	[mm]				3,0	h <sub>ef</sub>			
Splitting failure			·								
Edge distance		C <sub>cr,sp</sub>	[mm]		1	,0 · h <sub>ef</sub> ≤	$2 \cdot h_{ef} (2,$	$5 - \frac{h}{h_{ef}}$	$\leq$ 2,4 $\cdot$ h <sub>c</sub>	ef	
Axial distance		S <sub>cr,sp</sub>	[mm]				2 c	cr,sp			
Installation safety factor (dr	y and wet concrete)	γ2	·	1,0				1,2			
	oded bore hole)	γ2			1	,4			not adr	missible	

### Performances

Characteristic values of resistance for threaded rods under tension loads in non-cracked concrete (Design according to CEN/TS 1992-4)



#### Table C8: Characteristic values of resistance for threaded rods under tension loads in cracked concrete (Design according to CEN/TS 1992-4 or TR 045) Anchor size threaded rod M 12 M 16 M 20 M24 M27 M30 Steel failure Characteristic tension resistance, [kN] 34 63 98 141 184 224 N<sub>Rk,s</sub> = N<sup>0</sup><sub>Rk,s,seis</sub> Steel, property class 4.6 Characteristic tension resistance, [kN] 42 78 122 176 230 280 N<sub>Rk,s</sub> = N<sup>0</sup><sub>Rk,s,seis</sub> Steel, property class 5.8 Characteristic tension resistance, N<sub>Rk.s</sub>=N<sup>0</sup><sub>Rk.s.seis</sub> [kN] 67 125 196 282 368 449 Steel, property class 8.8 Characteristic tension resistance, Stainless steel A4 and HCR, [kN] 59 110 171 247 230 281 N<sub>Rk,s</sub> = N<sup>0</sup><sub>Rk,s,seis</sub> property class 50 (>M24) and 70 (≤ M24) Combined pull-out and concrete failure Characteristic bond resistance in cracked concrete C20/25 [N/mm<sup>2</sup>] 5,5 5,5 5,5 5,5 6,5 6,5 $\tau_{\rm Rk,cr}$ dry and wet concrete $\tau^0_{\text{Rk,seis}}$ 4,5 [N/mm<sup>2</sup>] 3.7 3,7 3,7 3,8 4.5 Temperature range I: 40°C/24°C [N/mm<sup>2</sup>] 5,5 not admissible 5,5 $\tau_{\text{Rk,cr}}$ flooded bore hole $\tau^0_{\ Rk,seis}$ not admissible [N/mm<sup>2</sup>] 3,7 3,7 4,0 4,0 4,5 $\tau_{Rk,cr}$ [N/mm<sup>2</sup>] 4,0 4,0 4,5 dry and wet concrete $\tau^0_{Rk,seis}$ 2,7 2,7 [N/mm<sup>2</sup>] 2,7 2,8 3,1 3,1 Temperature range II: 80°C/50°C 4,0 not admissible $\tau_{\mathsf{Rk},\mathsf{cr}}$ [N/mm<sup>2</sup>] 4,0 flooded bore hole $\tau^0_{\text{Rk,seis}}$ [N/mm<sup>2</sup>] 2,7 2,7 not admissible 3,0 [N/mm<sup>2</sup>] 3,0 3,0 3,0 3,5 3,5 $\tau_{\text{Rk,cr}}$ dry and wet concrete $\tau^0_{\ Rk,seis}$ 2,0 2,0 2,0 2,4 2,4 [N/mm<sup>2</sup>] 2,1 Temperature range III: 120°C/72°C [N/mm<sup>2</sup>] 3,0 3,0 not admissible $\tau_{\text{Rk,cr}}$ flooded bore hole $\tau^0_{Rk,seis}$ [N/mm<sup>2</sup>] 2,0 2,0 not admissible C30/37 1.04 Increasing factors for concrete (only static or quasi-static actions) C40/50 1,08

 $\Psi_{\rm C}$ 

Factor according to

CEN/TS 1992-4-5 Section 6.2.2.3

Concrete cone failure				
Factor according to CEN/TS 1992-4-5 Section 6.2.3.1	k <sub>cr</sub>	[-]		7,2
Edge distance	C <sub>cr,N</sub>	[mm]		1,5 h <sub>ef</sub>
Axial distance	S <sub>cr,N</sub>	[mm]		3,0 h <sub>ef</sub>
Splitting failure	·			
Edge distance	C <sub>cr,sp</sub>	[mm]	1,0 ∙ h <sub>ef</sub>	$\leq 2 \cdot h_{ef}\left(2,5-\frac{h}{h_{ef}}\right) \leq 2,4 \cdot h_{ef}$
Axial distance	S <sub>cr,sp</sub>	[mm]		2 c <sub>cr,sp</sub>
Installation safety factor (dry and wet concrete)	γ2			1,2
Installation safety factor (flooded bore hole)	γ2		1,4	not admissible

[-]

C50/60

 $k_8$ 

## TILCA TIM V+ Injection System for concrete

### Performances

Characteristic values of resistance for threaded rods under tension loads in cracked concrete (Design according to CEN/TS 1992-4 or TR 045)

Annex C 8

1,10

7,2



# Table C9: Characteristic values of resistance for threaded rods under shear loads in cracked and non-cracked concrete (Design according to CEN/TS 1992-4 or TR 045)

Anchor size threaded rod			M 8	M 10	M 12	M 16	M 20	M24	M 27	M 3
Steel failure without lever arm		l.			1	1	1	1		
Characteristic shear resistance,	V <sub>Rk,s</sub>	[kN]	7	12	17	31	49	71	92	112
Steel, property class 4.6	$V^0_{Rk,s,seis}$	[kN]	-	-	12	22	34	50	65	78
Characteristic shear resistance,	$V_{Rk,s}$	[kN]	9	15	21	39	61	88	115	14
Steel, property class 5.8	$V^0_{Rk,s,seis}$	[kN]	-	-	15	27	43	62	81	98
Characteristic shear resistance,	$V_{Rk,s}$	[kN]	15	23	34	63	98	141	184	22
Steel, property class 8.8	$V^0_{Rk,s,seis}$	[kN]	-	-	24	44	69	99	129	15
Characteristic shear resistance, Stainless steel A4 and HCR.	$V_{Rk,\mathbf{s}}$	[kN]	13	20	30	55	86	124	115	14
property class 50 (>M24) and 70 ( $\leq$ M24)	V <sup>0</sup> <sub>Rk,s,seis</sub>	[kN]	-	-	21	39	60	87	81	98
Ductility factor according to CEN/TS 1992-4-5 Section 6.3.2.1	k <sub>2</sub>				1	0,8	1		1	
Steel failure with lever arm										
Characteristic bending moment,	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	15	30	52	133	260	449	666	90
Steel, property class 4.6	M <sup>0</sup> <sub>Rk,s,seis</sub>	[Nm]		No	Performa	ance Det	ermined	(NPD)		
Characteristic bending moment,	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	19	37	65	166	324	560	833	112
Steel, property class 5.8	${\sf M}^{\sf O}_{\sf Rk,s,seis}$	[Nm]		No	Performa	ance Det	ermined	(NPD)		
Characteristic bending moment,	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	30	60	105	266	519	896	1333	17
Steel, property class 8.8	$M^0_{Rk,s,seis}$	[Nm]		No	Performa	ance Det	ermined	(NPD)	_	
Characteristic bending moment,	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	26	52	92	232	454	784	832	11:
Stainless steel A4 and HCR, property class 50 (>M24) and 70 ( $\leq$ M24)	M <sup>0</sup> <sub>Rk,s,seis</sub>	[Nm]		No	Performa	ance Det	ermined	(NPD)		
Concrete pry-out failure		1 1								
Factor in equation (27) of CEN/TS 1992-4-5 Section 6.3.3	k <sub>3</sub>					2,0				
Installation safety factor	γ2					1,0				
Concrete edge failure <sup>3)</sup>		l.								
Effective length of anchor	l <sub>t</sub>	[mm]			I <sub>t</sub> =	min(h <sub>et</sub> ; 8	3 d <sub>nom</sub> )			
Outside diameter of anchor	d <sub>nom</sub>	[mm]	8	10	12	16	20	24	27	30
Installation safety factor	$\gamma_2$					1,0				

# TILCA TIM V+ Injection System for concrete

### Performances

Characteristic values of resistance for threaded rods under shear loads in cracked and non-cracked concrete, (Design according to CEN/TS 1992-4 or TR 045)



Table C10: Char non-	acteristic value									ls in		
Anchor size reinforcing ba	r			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Steel failure												
Characteristic tension resista	ance	N <sub>Rk,s</sub>	[kN]					$A_{s}\boldsymbol{\cdot}f_{uk}$				
Combined pull-out and cor	ncrete failure											
Characteristic bond resistance	ce in non-cracked concre	te C20/25	5									
Temperature range I:	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm²]	10	12	12	12	12	12	11	10	8,5
40°C/24°C	flooded bore hole	τ <sub>Rk,ucr</sub>	[N/mm <sup>2</sup> ]	7,5	8,5	8,5	8,5	8,5		not adr	nissible	
Temperature range II:	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm²]	7,5	9	9	9	9	9	8,0	7,0	6,0
80°C/50°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm <sup>2</sup> ]	5,5	6,5	6,5	6,5	6,5		not adr	nissible	
Temperature range III:	dry and wet concrete	$\tau_{\text{Rk,ucr}}$	[N/mm <sup>2</sup> ]	5,5	6,5	6,5	6,5	6,5	6,5	6,0	5,0	4,5
120°C/72°C	flooded bore hole	$ au_{Rk,ucr}$	[N/mm²]	4,0	5,0	5,0	5,0	5,0		not adr	nissible	
Increasing factors for concre	*0	C30/37						1,04				
Increasing factors for concre $\psi_c$	le	C40/50						1,08				
Factor according to		C50/60						1,10				
CEN/TS 1992-4-5 Section 6.	2.2.3	k <sub>8</sub>	[-]					10,1				
Concrete cone failure												
Factor according to CEN/TS 1992-4-5 Section 6.	2.3.1	k <sub>ucr</sub>	[-]					10,1				
Edge distance		C <sub>cr,N</sub>	[mm]					1,5 h <sub>ef</sub>				
Axial distance		S <sub>cr,N</sub>	[mm]					3,0 h <sub>ef</sub>				
Splitting failure		<u> </u>		1								
Edge distance		C <sub>cr,sp</sub>	[mm]			1,0 · h,	<sub>ef</sub> ≤2·h	<sub>ef</sub> (2,5	<u>h</u> h <sub>ef</sub> )≤2	,4 ⋅ h <sub>ef</sub>		
Axial distance		S <sub>cr,sp</sub>	[mm]					$2 c_{\text{cr,sp}}$				
Installation safety factor (dry	and wet concrete)	γ2		1.0				1	,2			
Installation safety factor (floo	oded bore hole)	γ2				1,4				not adr	nissible	
TILCA TIM V+ Injec	ction System for	concre	ete						1	٨٥٩٥	v C 1	<b>-</b>

### Performances

Characteristic values of resistance for rebar under tension loads in non-cracked concrete (Design according to CEN/TS 1992-4)



Anchor size reinforcing	bar			Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Steel failure				2.12		2.0		2.20	2.0	2.02
Characteristic tension res	istance	N <sub>Rk,s</sub> =N <sup>0</sup> <sub>Rk,s,seis</sub>	[kN]				$A_{s}\boldsymbol{\cdot}f_{uk}$			
Combined pull-out and	concrete failure									
Characteristic bond resist	ance in cracked concrete	C20/25								
		$\tau_{\text{Rk,cr}}$	[N/mm <sup>2</sup> ]	5,5	5,5	5,5	5,5	5,5	6,5	6,5
Temperature range I:	dry and wet concrete	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	3,7	3,7	3,7	3,7	3,8	4,5	4,5
40°C/24°C		$\tau_{\rm Rk,cr}$	[N/mm <sup>2</sup> ]	5,5	5,5	5,5		not adr	nissible	L
	flooded bore hole	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	3,7	3,7	3,7		not adr	nissible	
		τ <sub>Rk,cr</sub>	[N/mm <sup>2</sup> ]	4,0	4,0	4,0	4,0	4,0	4,5	4,5
Temperature range II:	dry and wet concrete	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	2,7	2,7	2,7	2,7	2,8	3,1	3,1
80°C/50°C		τ <sub>Rk,cr</sub>	[N/mm <sup>2</sup> ]	4,0	4,0	4,0		not adr	nissible	1
	flooded bore hole	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	2,7	2,7	2,7		not adr	nissible	
		τ <sub>Rk,cr</sub>	[N/mm <sup>2</sup> ]	3,0	3,0	3,0	3,0	3,0	3,5	3,5
Temperature range III:	dry and wet concrete	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	2,0	2,0	2,0	2,0	2,1	2,4	2,4
120°C/72°C		τ <sub>Rk,cr</sub>	[N/mm <sup>2</sup> ]	3,0	3,0	3,0		not adr	nissible	
	flooded bore hole	$\tau^0_{Rk,seis}$	[N/mm <sup>2</sup> ]	2,0	2,0	2,0		not adr	nissible	
Increasing factors for con		C30/37					1,04			
(only static or quasi-static		C40/50					1,08			
Ψc		C50/60	1				1,10			
Factor according to CEN/TS 1992-4-5 Section	n 6.2.2.3	k <sub>8</sub>	[-]				7,2			
Concrete cone failure										
Factor according to CEN/TS 1992-4-5 Section	16.2.3.1	k <sub>cr</sub>	[-]				7,2			
Edge distance	0.2.0.1	C <sub>cr,N</sub>	[mm]				1,5 h <sub>et</sub>			
Axial distance		S <sub>cr,N</sub>	[mm]				3,0 h <sub>et</sub>			
Splitting failure			L	1						
Edge distance		C <sub>cr,sp</sub>	[mm]		1,0 ·	h <sub>ef</sub> ≤2 ⋅ h	$e_{\rm ef}\left(2,5-\frac{1}{h}\right)$	<u>h</u> n <sub>ef</sub> )≤ 2,4	∙h <sub>ef</sub>	
Axial distance		S <sub>cr,sp</sub>	[mm]				2 c <sub>cr,sp</sub>			
Installation safety factor (	dry and wet concrete)	γ2					1,2			
Installation safety factor (f	flooded bore hole)	γ2			1,4			not adr	nissible	
				1						
TILCA TIM V+ Inj Performances	ection System fo	r concrete							ex C 1	



Table C12: Characteristic value and non-cracked co											
Anchor size reinforcing bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Steel failure without lever arm											
Characteristic shear resistance	V <sub>Rk,s</sub>	[kN]				0,	50 • A <sub>s</sub> •	f <sub>uk</sub>			
Unaracteristic shear resistance	$V^0_{Rk,s,seis}$	[kN]				0,	35 • A <sub>s</sub> •	<b>f</b> <sub>uk</sub>			
Ductility factor according to CEN/TS 1992-4-5 Section 6.3.2.1	k <sub>2</sub>						0,8				
Steel failure with lever arm											
Characteristic bending moment	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]				1.	2 ∙ W <sub>el</sub> ∙	f <sub>uk</sub>			
Characteristic bending moment	$M^0_{\ Rk,s,seis}$	[Nm]			No P€	erformar	nce Dete	rmined	(NPD)		
Concrete pry-out failure											
Factor in equation (27) of CEN/TS 1992-4-5 Section 6.3.3	k <sub>3</sub>						2,0				
Installation safety factor	γ2						1,0				
Concrete edge failure											
Effective length of anchor	l <sub>f</sub>	[mm]				$I_{f} = rr$	nin(h <sub>ef</sub> ; 8	d <sub>nom</sub> )			
Outside diameter of anchor	d <sub>nom</sub>	[mm]	8	10	12	14	16	20	24	27	30
Installation safety factor	γ2						1,0				

### Performances

Characteristic values of resistance for rebar under shear loads in cracked and non-cracked concrete, (Design according to CEN/TS 1992-4 or TR 045)



Anchor size thread	led rod		M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Non-cracked conc	rete C20/25									
Temperature range I:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,021	0,023	0,026	0,031	0,036	0,041	0,045	0,049
40°C/24°C	$\delta_{N\infty}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,030	0,033	0,037	0,045	0,052	0,060	0,065	0,071
Temperature range II:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119
80°C/50°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172
Temperature range III:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119
120°C/72°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172
Cracked concrete	C20/25									
Temperature range I:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]					0,0	)70		
40°C/24°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm <sup>2</sup> )]		-			0,1	05		
Temperature range II:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]					0,1	70		
80°C/50°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm <sup>2</sup> )]		-			0,2	245		
Temperature range III:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]					0,1	70		
120°C/72°Č	$\delta_{N\infty}$ -factor	[mm/(N/mm <sup>2</sup> )]		-			0,2	245		

<sup>1)</sup> Calculation of the displacement

 $\delta_{\text{N0}} = \delta_{\text{N0}}\text{-factor} \ \cdot \ \tau;$ 

 $\delta_{N\infty} = \delta_{N\infty} \text{-factor} \quad \cdot \ \tau;$ 

# Table C14: Displacements under shear load<sup>1)</sup> (threaded rod)

	eaded rod		M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
For non-cracked	d concrete C2	0/25	•							
All temperature	$\delta_{V0}$ -factor	[mm/(kN)]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V\infty}\text{-}factor$	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05
For cracked cor	ncrete C20/25									
All temperature	$\delta_{V0}$ -factor	[mm/(kN)]			0,11	0,10	0,09	0,08	0,08	0,07
ranges	$\delta_{V\infty}$ -factor	[mm/(kN)]		-	0,17	0,15	0,14	0,13	0,12	0,10
	Injustion Su	retarn for conor								
TILCA TIM V+ Performances	Injection Sy	stem for concre	ete						nex C <sup>-</sup>	13



Anchor size reinfo	orcing bar		Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Non-cracked cone	crete C20/	25		1			I	I	1	1	
Temperature range I: 40°C/24°C	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,021	0,023	0,026	0,028	0,031	0,036	0,043	0,047	0,052
	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,030	0,033	0,037	0,041	0,045	0,052	0,061	0,071	0,07
Temperature range II: 80°C/50°C	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,12
	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,18
Temperature range III: 120°C/72°C	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,12
	$\delta_{N\infty}\text{-factor}$	[mm/(N/mm <sup>2</sup> )]	0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,18
Cracked concrete	C20/25										
Temperature range I: 40°C/24°C	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]						0,070			
	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm <sup>2</sup> )]	-	-				0,105			
Temperature range II: 80°C/50°C	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]						0,170			
	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm <sup>2</sup> )]	-   .	-				0,245			
Temperature range III:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]						0,170			
120°C/72°C	S factor	[mm/(N/mm <sup>2</sup> )]	1.	-				0.045			
$  \  \  \  \  \  \  \  \  \  \  \  \  \$	· τ; · τ;	- , ,-	hear lo	oad <sup>1)</sup> (r	ebar)			0,245			
$  \  \  \  \  \  \  \  \  \  \  \  \  \$	ie displacen • τ; • τ; <b>isplacen</b>	nent	hear lo	oad <sup>1)</sup> (r ∅10	ebar) Ø 12	Ø 14	Ø 16	0,245 Ø 20	Ø 25	Ø 28	Ø 3
<sup>1)</sup> Calculation of th $\delta_{N0} = \delta_{N0}$ -factor $\delta_{N\infty} = \delta_{N\infty}$ -factor <b>Table C16: D</b>	ie displacen • τ; • τ; isplacen prcing bar	nent under s	1		-	Ø 14	Ø 16		Ø 25	Ø 28	Ø3
<sup>1)</sup> Calculation of th $\delta_{N0} = \delta_{N0}$ -factor $\delta_{N\infty} = \delta_{N\infty}$ -factor <b>Table C16: D</b> Anchor size reinfor Non-cracked conc	ie displacen • τ; • τ; isplacen prcing bar	nent under s	1		-	Ø 14 0,04	Ø <b>16</b> 0,04		Ø <b>25</b> 0,03	Ø <b>28</b> 0,03	
<sup>1)</sup> Calculation of th $\delta_{N0} = \delta_{N0}$ -factor $\delta_{N\infty} = \delta_{N\infty}$ -factor <b>Table C16: D</b> Anchor size reinfo	isplacen · τ; · τ; isplacen prcing bar crete C20/2 δ <sub>v0</sub> -factor	nent under s	Ø 8	Ø 10	Ø 12			Ø 20			0,03
<sup>1)</sup> Calculation of th $\delta_{N0} = \delta_{N0}$ -factor $\delta_{N\infty} = \delta_{N\infty}$ -factor <b>Table C16: D</b> Anchor size reinfor Non-cracked conc All temperature	e displacen · τ; · τ; isplacen prcing bar crete C20/2 δ <sub>V0</sub> -factor δ <sub>V∞</sub> -factor	nent under s 25 [mm/(kN)]	Ø 8 0,06	Ø <b>10</b>	Ø <b>12</b>	0,04	0,04	Ø <b>20</b> 0,04	0,03	0,03	0,03
<sup>1)</sup> Calculation of th $\delta_{N0} = \delta_{N0}$ -factor $\delta_{N\infty} = \delta_{N\infty}$ -factor <b>Table C16: D</b> Anchor size reinfor Non-cracked conce All temperature ranges Cracked concrete	e displacen · τ; · τ; isplacen prcing bar crete C20/2 δ <sub>V0</sub> -factor δ <sub>V∞</sub> -factor	nent under s 25 [mm/(kN)]	Ø 8 0,06	Ø <b>10</b>	Ø <b>12</b>	0,04	0,04	Ø <b>20</b> 0,04	0,03	0,03	Ø 32 0,03 0,04
<sup>1)</sup> Calculation of th $\delta_{N0} = \delta_{N0}$ -factor $\delta_{N\infty} = \delta_{N\infty}$ -factor <b>Table C16: D</b> Anchor size reinfor Non-cracked conc All temperature anges <b>Cracked concrete</b> All temperature anges <sup>1)</sup> Calculation of th $\delta_{V0} = \delta_{V0}$ -factor	he displacent $\tau;$ $\tau;$ isplacent prcing bar prcing bar prete C20/2 $\delta_{V0}$ -factor $\delta_{V\infty}$ -factor $\delta_{V\infty}$ -factor $\delta_{V\infty}$ -factor $\delta_{V\infty}$ -factor he displacent $\cdot$ V;	ent nent under s 25 [mm/(kN)] [mm/(kN)] [mm/(kN)]	Ø 8 0,06	Ø <b>10</b>	Ø <b>12</b> 0,05 0,08	0,04 0,06	0,04	Ø <b>20</b> 0,04 0,05	0,03	0,03	0,03
<sup>1)</sup> Calculation of th $\delta_{NO} = \delta_{NO}$ -factor $\delta_{Noo} = \delta_{Noo}$ -factor <b>Table C16: D</b> Anchor size reinfor Non-cracked conc All temperature ranges <b>Cracked concrete</b> All temperature ranges <sup>1)</sup> Calculation of th	he displacent $\tau;$ $\tau;$ isplacent prcing bar prcing bar prete C20/2 $\delta_{V0}$ -factor $\delta_{V\infty}$ -factor $\delta_{V\infty}$ -factor $\delta_{V\infty}$ -factor $\delta_{V\infty}$ -factor he displacent $\cdot$ V;	ent nent under s 25 [mm/(kN)] [mm/(kN)] [mm/(kN)]	Ø 8 0,06	Ø <b>10</b>	Ø <b>12</b> 0,05 0,08 0,11	0,04 0,06 0,11	0,04 0,06 0,10	Ø <b>20</b> 0,04 0,05 0,09	0,03 0,05 0,08	0,03 0,04 0,07	0,03

Displacements (rebar)