



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-14/0207 of 4 July 2014

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

Deutsches Institut für Bautechnik

Ripple Injection System R-Fix for concrete

Bonded Anchor with Anchor rod for use in concrete

RIPPLE CONSTRUCTION PRODUCTS PVT LTD+ Corp. Office: 303 & 403, ROYAL ARCADIA Above SBI Bank, Balkampet Main Road S R NAGAR, HYDERABAD - 500 038 INDIA PinCode -INDIEN

RIPPLE CONSTRUCTION PRODUCTS PVT LTD+ Plant 1

27 pages including 3 annexes which form an integral part of this assessment

Guideline for European technical approval of "Metal anchors for use in concrete", ETAG 001 Part 5: "Bonded anchors", April 2013,

used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011.



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Specific Part

1 Technical description of the product

The "Ripple Injection system R-Fix for concrete" is a bonded anchor consisting of a cartridge with injection mortar Ripple R-Fix and a steel element. The steel element consist of a commercial threaded rod with washer and hexagon nut in the range of M8 to M30 or a reinforcing bar in the range of diameter 8 to 32 mm.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between metal part, injection mortar and concrete.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance for tension loads in non-cracked concrete	See Annex C 1 / C 4 / C 7 / C 10
Characteristic resistance for tension loads in cracked concrete	See Annex C 2 / C 5 / C 8 / C 11
Characteristic resistance for shear loads in cracked and non-cracked concrete	See Annex C 3 / C 6 / C 9 / C 12
Displacements under tension and shear loads	See Annex C 13 / C 14

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	No performance determined (NPD)

3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances there may be requirements (e.g. transposed European legislation and national laws, regulations and administrative provisions) applicable to the products falling within the scope of this European Technical Assessment. In order to meet the provisions of Regulation (EU) No 305/2011, these requirements need also to be complied with, when and where they apply.

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3.4 Safety in use (BWR 4)

The essential characteristics regarding Safety in use are included under the Basic Works Requirement Mechanical resistance and stability.

3.5 Protection against noise (BWR 5)

Not applicable.

3.6 Energy economy and heat retention (BWR 6)

Not applicable.

3.7 Sustainable use of natural resources (BWR 7)

The sustainable use of natural resources was not investigated.

3.8 General aspects

The verification of durability is part of testing the essential characteristics. Durability is only ensured if the specifications of intended use according to Annex B are taken into account.

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

According to Decision of the Commission of 24 June 1996 (96/582/EC) (OJ L 254 of 08.10.96 p. 62-65), the system of assessment and verification of constancy of performance (see Annex V and Article 65 Paragraph 2 to Regulation (EU) No 305/2011) given in the following table applies.

Product	Intended use	Level or class	System
Metal anchors for use in concrete (heavy-duty type)	For fixing and/or supporting concrete structural elements or heavy units such as cladding and suspended ceilings	_	1

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

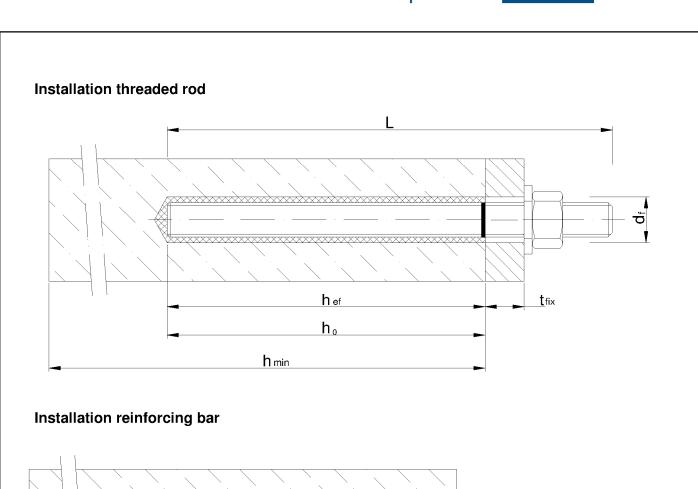
Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at Deutsches Institut für Bautechnik.

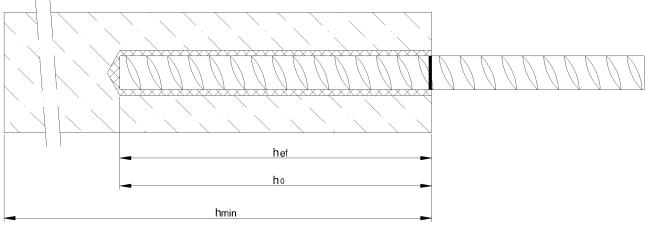
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Uwe Bender Head of Department Beglaubigt: Baderschneider

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d_f = diameter of clearance hole in the fixture

 t_{fix} = thickness of fixture

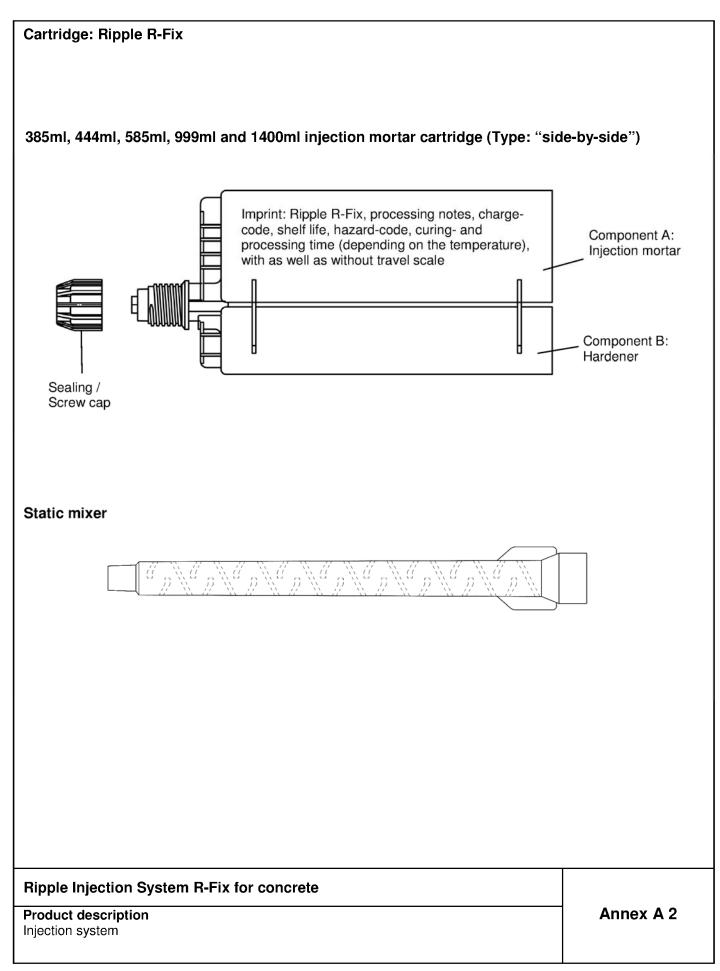
h_{ef} = effective anchorage depth

 h_0 = depth of drill hole

 h_{min} = minimum thickness of member

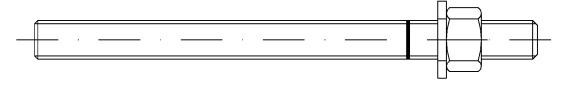
Ripple Injection System R-Fix for concrete	
Product description Installed condition	Annex A 1

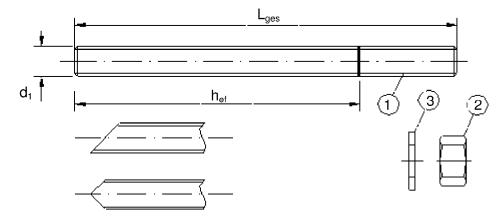






Threaded rod M8, M10, M12, M16, M20, M24, M27, M30 with washer and hexagon nut

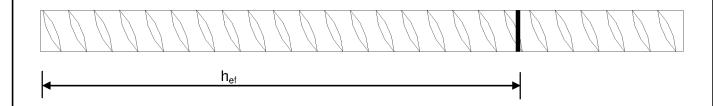




Commercial standard threaded rod with:

- Materials, dimensions and mechanical properties acc. Table A1
- Inspection certificate 3.1 acc. to EN 10204:2004
- Marking of embedment depth

Reinforcing bar \varnothing 8, \varnothing 10, \varnothing 12, \varnothing 14, \varnothing 16, \varnothing 20, \varnothing 25, \varnothing 28, \varnothing 32



- Minimum value of related rip area f_{R,min} according to EN 1992-1-1:2004+AC:2010
- Rib height of the bar shall be in the range 0,05d ≤ h ≤ 0,07d
 (d: Nominal diameter of the bar; h: Rip height of the bar)

Ripple Injection System R-Fix for concrete	
Product description Threaded rod and reinforcing bar	Annex A 3

Z36703.14



Part	, 9	Material
	, zinc plated ≥ 5 μm acc. to EN ISO 4042:19 , hot-dip galvanised ≥ 40 μm acc. to EN ISO	999 or O 1461:2009 and EN ISO 10684:2004+AC:2009
1	Anchor rod	Steel, EN 10087:1998 or EN 10263:2001 Property class 4.6, 5.8, 8.8, EN 1993-1-8:2005+AC:2009
2	Hexagon nut, EN ISO 4032:2012	Steel acc. to EN 10087:1998 or EN 10263:2001 Property class 4 (for class 4.6 rod) EN ISO 898-2:2012, Property class 5 (for class 5.8 rod) EN ISO 898-2:2012, Property class 8 (for class 8.8 rod) EN ISO 898-2:2012
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Steel, zinc plated or hot-dip galvanised
Stain	less steel	
1	Anchor rod	Material 1.4401 / 1.4404 / 1.4571, EN 10088-1:2005, > M24: Property class 50 EN ISO 3506-1:2009 ≤ M24: Property class 70 EN ISO 3506-1:2009
2	Hexagon nut, EN ISO 4032:2012	Material 1.4401 / 1.4404 / 1.4571 EN 10088:2005, > M24: Property class 50 (for class 50 rod) EN ISO 3506-2:2009 ≤ M24: Property class 70 (for class 70 rod) EN ISO 3506-2:2009
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4401, 1.4404 or 1.4571, EN 10088-1:2005
High	corrosion resistance steel	
1	Anchor rod	Material 1.4529 / 1.4565, EN 10088-1:2005, > M24: Property class 50 EN ISO 3506-1:2009 ≤ M24: Property class 70 EN ISO 3506-1:2009
2	Hexagon nut, EN ISO 4032:2012	Material 1.4529 / 1.4565 EN 10088-1:2005, > M24: Property class 50 (for class 50 rod) EN ISO 3506-2:2009 ≤ M24: Property class 70 (for class 70 rod) EN ISO 3506-2:2009
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4529 / 1.4565, EN 10088-1:2005
Reinf	orcing bars	
1	Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C f_{yk} and k according to NDP or NCL of EN 1992-1-1/NA:2013 $f_{uk} = f_{tk} = k \cdot f_{yk}$



Specifications of intended use

Anchorages subject to:

- Static and quasi-static loads: M8 to M30, Rebar Ø8 to Ø32.
- Seismic action for Performance Category C1: M12 to M30, Rebar Ø12 to Ø32.

Base materials:

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000.
- Strength classes C20/25 to C50/60 according to EN 206-1:2000.
- Non-cracked concrete: M8 to M30, Rebar Ø8 to Ø32.
- Cracked concrete: M12 to M30, Rebar Ø12 to Ø32.

Temperature Range:

- I: 40 °C to +40 °C (max long term temperature +24 °C and max short term temperature +40 °C)
- II: 40 °C to +60 °C (max long term temperature +43 °C and max short term temperature +60 °C)
- III: 40 °C to +72 °C (max long term temperature +43 °C and max short term temperature +72 °C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc coated steel, stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist

(high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to supports, etc.).
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work
- Anchorages under static or quasi-static actions are designed in accordance with:
 - EOTA Technical Report TR 029 "Design of bonded anchors", Edition September 2010 or
 - CEN/TS 1992-4:2009
- Anchorages under seismic actions (cracked concrete) are designed in accordance with:
 - EOTA Technical Report TR 045 "Design of Metal Anchors under Seismic Action", Edition February 2013
 - Anchorages shall be positioned outside of critical regions (e.g. plastic hinges) of the concrete structure.
 - Fastenings in stand-off installation or with a grout layer are not allowed.

Installation:

Electronic copy of the ETA by DIBt: ETA-14/020

- Dry or wet concrete: M8 to M30, Rebar Ø8 to Ø32.
- Flooded holes (not sea water): M8 to M30, Rebar Ø8 to Ø32.
- Hole drilling by hammer or compressed air drill mode.
- Overhead installation allowed.
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.

Ripple Injection System R-Fix for concrete	
Intended Use Specifications	Annex B 1



Table B1: Installation Anchor size	n parameters fo	or threa	aded ro	M 12	M 16	M 20	M 24	M 27	M 30
Nominal drill hole diameter	d ₀ [mm] =	10	12	14	18	24	28	32	35
	h _{ef,min} [mm] =	60	60	70	80	90	96	108	120
Effective anchorage depth	h _{ef,max} [mm] =	96	120	144	192	240	288	324	360
Diameter of clearance hole in the fixture	d _f [mm] ≤	ım]≤ 9 12 14 18 22 26		30	33				
Diameter of steel brush	d _b [mm] ≥	d _b [mm] ≥ 12 14 16 20 26 30		30	34	37			
Torque moment	T _{inst} [Nm] ≤	10	20	40	80	120	160	180	200
This is a set that we	t _{fix,min} [mm] >	0							
Thickness of fixture	t _{fix,max} [mm] <		1500						
Minimum thickness of member	h _{min} [mm]	h _{ef} + 30 mm ≥ 100 mm							
Minimum spacing	s _{min} [mm]	40	50	60	80	100	120	135	150
Minimum edge distance	c _{min} [mm]	40	50	60	80	100	120	135	150

Table B2: Installation parameters for rebar

Rebar size		Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Nominal drill hole diameter	diameter d_0 [mm] =		14	16	18	20	24	32	35	40
Effective anchorage depth	h _{ef,min} [mm] =	60	60	70	75	80	90	100	112	128
Enective anchorage depth	h _{ef,max} [mm] =	96	120	144	168	192	240	300	336	384
Diameter of steel brush	d _b [mm] ≥	d _b [mm] ≥ 14 1		18	20	22	26	34	37	41,5
Minimum thickness of member	h _{min} [mm]		30 mm 0 mm	$\Pi_{-i} + 2\Pi_{0}$						
Minimum spacing	s _{min} [mm]	40	50	60	70	80	100	125	140	160
Minimum edge distance	c _{min} [mm]	40	50	60	70	80	100	125	140	160
·										

Ripple Injection System R-Fix for concrete	
Intended Use	Annex B 2
Installation parameters	



Steel brush



Table B3: Parameter cleaning and setting tools

Threaded Rod	Rebar	d₀ Drill bit - Ø	d₅ Brush - Ø	d _{b,min} min. Brush - Ø	Piston plug			
(mm)	(mm)	(mm)	(mm)	(mm)	(No.)			
M8		10	12	10,5				
M10	8	12	14	12,5				
M12	10	14	16	14,5	No			
	12	16	18	16,5	piston plug required			
M16	14	18	20	18,5	<u>'</u>			
	16	20	22	20,5				
M20	20	24	26	24,5	# 24			
M24		28	30	28,5	# 28			
M27	25	32	34	32,5	# 32			
M30	28	35	37	35,5	# 35			
	32	40	41,5	40,5	# 38			





Hand pump (volume 750 ml)

Recommended compressed air tool (min 6 bar) Drill bit diameter (d₀): 10 mm to 20 mm Drill bit diameter (d₀): 10 mm to 40 mm



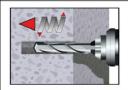
Piston plug for overhead or horizontal installation

Drill bit diameter (d₀): 24 mm to 40 mm

Ripple Injection System R-Fix for concrete	
Intended Use Cleaning and setting tools	Annex B 3

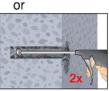


Installation instructions

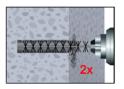


1. Drill with hammer drill a hole into the base material to the size and embedment depth required by the selected anchor (Table B1 or Table B2). In case of aborted drill hole: the drill hole shall be filled with mortar





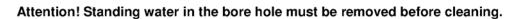






or





2a. Starting from the bottom or back of the bore hole, blow the hole clean with compressed air (min. 6 bar) or a hand pump (Annex B 3) a minimum of two times. If the bore hole ground is not reached an extension shall be used.

The hand-pump can be used for anchor sizes up to bore hole diameter 20 mm.

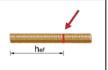
For bore holes larger than 20 mm or deeper 240 mm, compressed air (min. 6 bar) must be used.

- 2b. Check brush diameter (Table B3) and attach the brush to a drilling machine or a battery screwdriver. Brush the hole with an appropriate sized wire brush > d_{b.min} (Table B3) a minimum of two times. If the bore hole ground is not reached with the brush, a brush extension shall be used (Table B3).
- 2c. Finally blow the hole clean again with compressed air (min. 6 bar) or a hand pump (Annex B 3) a minimum of two times. If the bore hole ground is not reached an extension shall be used.

The hand-pump can be used for anchor sizes up to bore hole diameter 20 mm. For bore holes larger than 20 mm or deeper 240 mm, compressed air (min. 6 bar) must be used.

After cleaning, the bore hole has to be protected against re-contamination in an appropriate way, until dispensing the mortar in the bore hole. If necessary, the cleaning repeated has to be directly before dispensing the mortar. In-flowing water must not contaminate the bore hole again.







- Attach a supplied static-mixing nozzle to the cartridge and load the cartridge into the correct dispensing tool. Cut off the foil tube clip before use. For every working interruption longer than the recommended working time (Table B4) as well as for new cartridges, a new static-mixer shall be used.
- 4. Prior to inserting the anchor rod into the filled bore hole, the position of the embedment depth shall be marked on the anchor rods.
- 5. Prior to dispensing into the anchor hole, squeeze out separately a minimum of three full strokes and discard non-uniformly mixed adhesive components until the mortar shows a consistent grey colour. For foil tube cartridges is must be discarded a minimum of six full strokes.

Ripple Injection System R-Fix for concrete

Intended Use

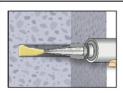
Installation instructions

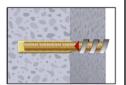
Annex B 4

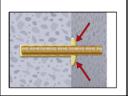
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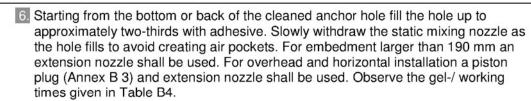


Installation instructions (continuation)



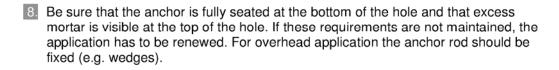


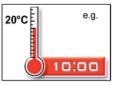


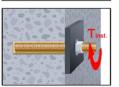


7. Push the threaded rod or reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached.

The anchor should be free of dirt, grease, oil or other foreign material.







- 9. Allow the adhesive to cure to the specified time prior to applying any load or torque. Do not move or load the anchor until it is fully cured (attend Table B4).
- 10. After full curing, the add-on part can be installed with the max. torque (Table B2) by using a calibrated torque wrench.

Table B4: Minimum curing time

Concrete temperature	Gelling- working time	Minimum curing time in dry concrete	Minimum curing time in wet concrete
≥ 5 °C	120 min	50 h	100 h
≥ + 10 °C	90 min	30 h	60 h
≥ + 20 °C	30 min	10 h	20 h
≥ + 30 °C	20 min	6 h	12 h
≥ + 40 °C	12 min	4 h	8 h

Ripple Injection System R-Fix for concrete	
Intended Use	Annex B 5
Installation instructions (continuation)	
Curing time	



Anchor size threaded ro	d			M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Steel failure					1						00
Characteristic tension resi	stance,	N _{Rk,s}	[kN]	15	23	34	63	98	141	184	224
Steel, property class 4.6 Characteristic tension resi Steel, property class 5.8	stance,	N _{Rk,s}	[kN]	18	29	42	78	122	176	230	280
Characteristic tension resi Steel, property class 8.8	stance,	N _{Rk,s}	[kN]	29	46	67	125	196	282	368	449
Characteristic tension resistance, Stainless steel A4 and HCR, property class 50 (>M24) and 70 (≤ M24)		$N_{Rk,s}$	[kN]	26	41	59	110	171	247	230	281
Combined pull-out and o	concrete cone failure										
Characteristic bond resista	ance in non-cracked con	crete C20/2	5								
Temperature range I:	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	15	15	15	14	13	12	12	12
40°C/24°C	flooded bore hole	τ _{Rk,ucr}	[N/mm²]	15	14	13	10	9,5	8,5	7,5	7,0
Temperature range II: 60°C/43°C	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	9,5	9,5	9,0	8,5	8,0	7,5	7,5	7,5
	flooded bore hole	τ _{Rk,ucr}	[N/mm²]	9,5	9,5	9,0	8,5	7,5	7,0	6,5	6,0
Temperature range III:	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	8,5	8,5	8,0	7,5	7,0	7,0	6,5	6,5
72°C/43°C	flooded bore hole	τ _{Rk,ucr}	[N/mm²]	8,5	8,5	8,0	7,5	7,0	6,0	5,5	5,5
		C30/37					1,	04			
Increasing factors for cond Ψ_c	crete	C40/50					1,	08			
		C50/60					1,	10			
Splitting failure											
Edge distance		C _{cr,sp}	[mm]	$1.0 \cdot h_{ef} \le 2 \cdot h_{ef} \left(2.5 - \frac{h}{h_{ef}} \right) \le 2.4 \cdot h_{ef}$							
Axial distance		S _{cr,sp}	[mm]				2 0	cr,sp			
Install safety factor (dry ar	nd wet concrete)	γ ₂	•		1	,2			1	,4	
Install safety factor (floode	ed bore hole)	γ ₂					1	,4			

Characteristic values of resistance for threaded rods under tension loads in non-cracked concrete	
Performances Characteristic values of resistance for threaded rods under tension loads in non-cracked concrete (Design according to TR 029)	Annex C 1



Anchor size threaded i	rod			M 12	M 16	M 20	M24	M 27	M 30	
Steel failure						•			'	
Characteristic tension re Steel, property class 4.6		N _{Rk,s} =N ⁰ _{Rk,s,seis}	[kN]	34	63	98	141	184	224	
Characteristic tension re Steel, property class 5.8	esistance,	N _{Rk,s} =N ⁰ _{Rk,s,seis}	[kN]	42	78	122	176	230	280	
Characteristic tension re Steel, property class 8.8		N _{Rk,s} =N ⁰ _{Rk,s,seis}	[kN]	67	125	196	282	368	449	
Characteristic tension re Stainless steel A4 and F property class 50 (>M24	esistance, ICR,	N _{Rk,s} =N ⁰ _{Rk,s,seis}	[kN]	59	110	171	247	230	281	
	concrete cone failure									
Characteristic bond resis	stance in cracked concret	e C20/25								
	d	τ _{Rk,cr}	[N/mm²]	7,5	6,5	6,0	5,5	5,5	5,5	
Temperature range I:	dry and wet concrete	τ ⁰ Rk,seis	[N/mm²]	5,2	4,4	4,1	3,9	3,9	3,9	
40°C/24°C		$ au_{ m Rk,cr}$	[N/mm²]	7,5	6,0	5,0	4,5	4,0	4,0	
	flooded bore hole	$ au^0_{ m Rk,seis}$	[N/mm ²]	5,2	4,1	3,5	3,1	3,0	2,8	
		$ au_{ m Rk,cr}$	[N/mm²]	4,5	4,0	3,5	3,5	3,5	3,5	
Temperature range II:	dry and wet concrete	τ ⁰ _{Rk,seis}	[N/mm²]	3,2	2,7	2,5	2,3	2,3	2,3	
60°C/43°C		$ au_{ m Rk,cr}$	[N/mm²]	4,5	4,0	3,5	3,5	3,5	3,5	
	dry and wet concrete	τ ⁰ Rk,seis	[N/mm²]	3,2	2,7	2,5	2,3	2,3	2,3	
		$ au_{Rk,cr}$	[N/mm²]	4,0	3,5	3,0	3,0	3,0	3,0	
Temperature range III:	dry and wet concrete	τ ⁰ _{Rk,seis}	[N/mm²]	2,9	2,4	2,2	2,1	2,1	2,1	
72°C/43°C	dry and wet concrete flooded bore hole	$ au_{Rk,or}$	[N/mm²]	4,0	3,5	3,0	3,0	3,0	3,0	
	llooded bore note	τ ⁰ _{Rk,seis}	[N/mm²]	2,9	2,4	2,2	2,1	2,1	2,1	
ncreasing factors for co	ncrete	C30/37				1,	04			
only static or quasi-stat		C40/50				1,	08			
Ψc		C50/60				1,	10			
Splitting failure										
Edge distance		C _{cr,sp}	[mm]		$1.0 \cdot h_{ef} \le 2 \cdot h_{ef} \left(2.5 - \frac{h}{h_{ef}} \right) \le 2.4 \cdot h_{ef}$					
Axial distance		S _{cr,sp}	s _{cr,sp} [mm]		2 C _{cr,sp}					
nstallation safety factor	(dry and wet concrete)	γ2		1,2 1,4				,4		
Installation safety factor	(flooded bore hole)	γ ₂				1	,4			

haracteristic values of resistance for threaded rods under tension loads in cracked concrete	
Performances Characteristic values of resistance for threaded rods under tension loads in cracked concrete (Design according to TR 029 or TR 045)	Annex C 2

Installation safety factor

English translation prepared by DIBt



1,0

Anchor size threaded rod			М 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Steel failure without lever arm				1						
Characteristic shear resistance.	$V_{Rk,s}$	[kN]	7	12	17	31	49	71	92	112
Steel, property class 4.6	V ⁰ _{Rk,s,seis}	[kN]	-	-	12	22	34	50	65	78
Characteristic shear resistance.	$V_{Rk,s}$	[kN]	9	15	21	39	61	88	115	140
Steel, property class 5.8	V ⁰ _{Rk,s,seis}	[kN]	-	-	15	27	43	62	81	98
Characteristic shear resistance,	$V_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Steel, property class 8.8	$V^0_{Rk,s,seis}$	[kN]	-	-	24	44	69	99	129	157
Characteristic shear resistance, Stainless steel A4 and HCR.	$V_{Rk,s}$	[kN]	13	20	30	55	86	124	115	140
property class 50 (>M24) and 70 (≤ M24)	V ⁰ _{Rk,s,seis}	[kN]	-	-	21	39	60	87	81	98
Steel failure with lever arm				•						
Characteristic bending moment,	M ⁰ _{Rk,s}	[Nm]	15	30	52	133	260	449	666	900
Characteristic bending moment, Steel, property class 4.6	M ⁰ _{Rk,s,seis}	[Nm]	No Performance Determined (NPD)							
Characteristic bending moment,	$M^0_{Rk,s}$	[Nm]	19	37	65	166	324	560	833	1123
Steel, property class 5.8	M ⁰ _{Rk,s,seis}	[Nm]			No Perf	ormance I	Determine	d (NPD)		
Characteristic bending moment,	M ⁰ _{Rk,s}	[Nm]	30	60	105	266	519	896	1333	1797
Steel, property class 8.8	M ⁰ _{Rk,s,seis}	[Nm]			No Perf	ormance I	Determine	d (NPD)		
Characteristic bending moment,	M ⁰ _{Rk,s}	[Nm]	26	52	92	232	454	784	832	1125
Stainless steel A4 and HCR, property class 50 (>M24) and 70 (≤ M24)	M ⁰ _{Rk,s,seis}	[Nm]			No Perf	ormance I	Determine	d (NPD)		
Concrete pry-out failure		•								
Factor k in equation (5.7) of Technical Report TR 029 for the design of Bonded Anchors	k	[-]				2	,0			
Installation safety factor	γ ₂					1	,0			

Characteristic values of resistance for threaded rods under shear loads in cracked and non-cracked	
Performances Characteristic values of resistance for threaded rods under shear loads in cracked and non-cracked concrete, (Design according to TR 029 or TR 045)	Annex C 3

 γ_2

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	aracteristic v								n loa	ds in			
Anchor size reinforcing	bar			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
Steel failure					•	•	•		•		•		
Characteristic tension resi	stance	N _{Rk,s}	[kN]	$A_s \cdot f_{uk}$									
Combined pull-out and o	concrete cone failur	е											
Characteristic bond resista	ance in uncracked co	ncrete C20)/25										
Temperature range I:	dry and wet concrete	τ _{Rk,ucr}	[N/mm²]	14	14	13	13	12	12	11	11	11	
40°C/24°C	flooded bore hole	$ au_{ m Rk,ucr}$	[N/mm²]	14	13	11	10	9,5	8,5	7,5	7,0	6,0	
Temperature range II:	dry and wet concrete	τ _{Rk,ucr}	[N/mm²]	8,5	8,5	8,0	8,0	7,5	7,0	7,0	6,5	6,5	
60°C/43°C	flooded bore hole	τ _{Rk,ucr}	[N/mm²]	8,5	8,5	8,0	8,0	7,5	7,0	6,0	5,5	5,0	
Temperature range III:	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	7,5	7,5	7,5	7,0	7,0	6,5	6,0	6,0	6,0	
72°C/43°C	flooded bore hole	$ au_{ m Rk,ucr}$	[N/mm ²]	7,5	7,5	7,5	7,0	7,0	6,0	5,5	5,0	4,5	
		C30/37	•			•	•	1,04			•		
Increasing factors for cond ψ_c	crete	C40/50						1,08					
		C50/60						1,10					
Splitting failure													
Edge distance		C _{cr,sp}	[mm]			1,0 · h _{et}	. ≤ 2 · h _e	_{ef} (2,5 -	$\left(\frac{h}{h_{ef}}\right) \le 1$	2,4 · h _{ef}			
Axial distance		S _{cr,sp}	[mm]	2 c _{cr,sp}									
Installation safety factor (concrete)	lry and wet	γ2				1,2				1	,4		
Installation safety factor (f	looded bore hole)	γ ₂			_			1,4			_		

Ripple Injection System R-Fix for concrete	
Performances Characteristic values of resistance for rebar under tension loads in non-cracked concrete (Design according to TR 029)	Annex C 4



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Anchor size reinforcing	g bar			Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
Steel failure						•	-	•	•		
Characteristic tension re	esistance	N _{Rk,s} =N ⁰ _{Rk,s,seis}	[kN]				A _s • f _{uk}				
Combined pull-out and	d concrete cone failure										
Characteristic bond resis	stance in cracked concret	te C20/25									
		$ au_{ m Rk,cr}$	[N/mm²]	7,5	7,0	6,5	6,0	5,5	5,5	5,5	
Temperature range I:	dry and wet concrete	τ ⁰ Rk,seis	[N/mm²]	5,2	4,7	4,4	4,1	3,9	3,9	3,9	
40°C/24°C		$ au_{ m Rk,cr}$	[N/mm²]	7,5	6,5	6,0	5,0	4,5	4,0	4,0	
	flooded bore hole	τ ⁰ _{Rk,seis}	[N/mm²]	5,2	4,6	4,1	3,5	3,0	2,9	2,7	
		$ au_{ m Rk,cr}$	[N/mm²]	4,5	4,0	4,0	3,5	3,5	3,5	3,5	
Temperature range II:	dry and wet concrete	τ ⁰ _{Rk,seis}	[N/mm²]	3,2	2,8	2,7	2,5	2,3	2,3	2,3	
60°C/43°C		$ au_{ m Rk,cr}$	[N/mm²]	4,5	4,0	4,0	3,5	3,5	3,5	3,0	
	flooded bore hole	τ ⁰ _{Rk,seis}	[N/mm²]	3,2	2,8	2,7	2,5	2,3	2,3	2,3	
		τ _{Rk,cr}	[N/mm²]	4,0	3,5	3,5	3,0	3,0	3,0	3,0	
Temperature range III:	dry and wet concrete	τ ⁰ _{Rk,seis}	[N/mm²]	2,9	2,6	2,4	2,2	2,1	2,1	2,1	
72°C/43°C		$ au_{ m Rk,cr}$	[N/mm²]	4,0	3,5	3,5	3,0	3,0	3,0	3,0	
	flooded bore hole	τ ⁰ _{Rk,seis}	[N/mm²]	2,9	2,6	2,4	2,2	2,1	2,1	2,1	
	1	C30/37		1,04							
Increasing factors for co (only static or quasi-stati		C40/50		1,08							
$\Psi_{ ext{c}}$		C50/60		1,10							
Splitting failure				•							
Edge distance		C _{cr,sp}	[mm]		1,0 · h	_{ef} ≤2·h	ef 2,5 -	$\frac{h}{h_{ef}}$ ≤ 2	2,4 · h _{ef}		
Axial distance		S _{cr,sp}	[mm]				2 c _{cr,sp}	01 /			
Installation safety factor	(dry and wet concrete)	γ ₂			1,2			1	,4		
Installation safety factor	(flooded bore hole)	γ2					1,4				
Rinnle Injection	System R-Fix for	r concrete									

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Anchor size reinforcing bar			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32		
Steel failure without lever arm				•				•		•			
Characteristic shear resistance	$V_{Rk,s}$	[kN]	0,50 • A _s • f _{uk}										
Characteristic shear resistance	V ⁰ _{Rk,s,seis}	[kN]	$0.35 \cdot A_s \cdot f_{uk}$										
Steel failure with lever arm		•											
	M ⁰ _{Rk,s}	[Nm]	1.2 · W _{el} · f _{uk}										
Characteristic bending moment	M ⁰ _{Rk,s,seis}	[Nm]	No Performance Determined (NPD)										
Concrete pry-out failure			<u> </u>										
Factor k in equation (5.7) of Technical Report TR 029 for the design of bonded anchors	k	[-] 2,0											
Installation safety factor	γ2		1,0										
Concrete edge failure													
Installation safety factor	γ2				γ ₂ 1,0								

Ripple Injection System R-Fix for concrete	
Performances Characteristic values of resistance for rebar under shear loads in cracked and non-cracked concrete, (Design according to TR 029 or TR 045)	Annex C 6



Anchor size threaded roo				M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Steel failure											
Characteristic tension resis	tance,	N _{Rk.s}	[kN]	15	23	34	63	98	141	184	224
Steel, property class 4.6 Characteristic tension resis Steel, property class 5.8	stance,	N _{Rk,s}	[kN]	18 29 42 78 122 176 23						230	280
Characteristic tension resis Steel, property class 8.8	N _{Rk,s}	[kN]	29	46	67	125	196	282	368	449	
Characteristic tension resis Stainless steel A4 and HCI property class 50 (>M24) a	$N_{Rk,s}$	[kN]	26	41	59	110	171	247	230	281	
Combined pull-out and co	oncrete failure										
Characteristic bond resista	nce in non-cracked concrete	e C20/25									
Temperature range I:	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	15	15	15	14	13	12	12	12
40°C/24°C	flooded bore hole	$ au_{ m Rk,ucr}$	[N/mm²]	15	14	13	10	9,5	8,5	7,5	7,0
Temperature range II:	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	9,5	9,5	9,0	8,5	8,0	7,5	7,5	7,5
60°C/43°C	flooded bore hole	$ au_{ m Rk,ucr}$	[N/mm²]	9,5	9,5	9,0	8,5	7,5	7,0	6,5	6,0
Temperature range III:	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	8,5	8,5	8,0	7,5	7,0	7,0	6,5	6,5
72°C/43°C	flooded bore hole	$ au_{ m Rk,ucr}$	[N/mm ²]	8,5	8,5	8,0	7,5	7,0	6,0	5,5	5,5
		C30/37	1,04								
Increasing factors for conc Ψ _c	rete	C40/50		1,08							
10		C50/60	1,10								
Factor according to CEN/TS 1992-4-5 Section	6.2.2.3	k ₈	[-]	10,1							
Concrete cone failure		•									
Factor according to CEN/TS 1992-4-5 Section 6.2.3.1			[-]	10,1							
Edge distance	C _{cr,N}	[mm]	1,5 h _{et}								
Axial distance		S _{cr,N}	[mm]				3,0	h _{ef}			
Splitting failure											
Edge distance			[mm]	$1,0 \cdot h_{ef} \le 2 \cdot h_{ef} \left(2,5 - \frac{h}{h_{ef}} \right) \le 2,4 \cdot h_{ef}$							
Axial distance		S _{cr,sp}	[mm]	2 C _{cr,sp}							
Installation safety factor (di	y and wet concrete)	γ2	-	1,2					1	,4	
Installation safety factor (flo	ooded bore hole)	γ ₂	1,4								

Ripple Injection System R-Fix for concrete	
Performances Characteristic values of resistance for threaded rods under tension loads in non-cracked concrete (Design according to CEN/TS 1992-4)	Annex C 7



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Steel failure Characteristic renson resistance, Steel, property (slass 4.6)	Anchor size threaded rod	l			M 12	M 16	M 20	M24	M27	МЗ
Steel, property class 4.6 Niss = Ni Tiss exists IKN] 34 63 98 141 194	Steel failure									
Side property class 2,0		stance,	N NO	[kN]	34	63	98	141	184	224
Steel, property class 5.8 Nink, = Th' Pia, 2.500 RN 42		stance.		+ ' '						
Steel, property class 8.8 Nisk, = Nisk, sees IkN 59 110 171 247 230	Steel, property class 5.8	,		[KN]	42	/8	122	1/6	230	28
Stainless steel A4 and HCR, property class 50 (>M24) and 70 (≤M24) N _{Rus} = N ⁰ (Russion) [RN] 59 110 171 247 230 Combined pull-out and concrete failure Characteristic bond resistance in cracked concrete C20/25 Temperature range I: flooded bore hole flooded floo	Steel, property class 8.8		$N_{Rk,s} = N^0_{Rk,s,seis}$	[kN]	67	125	196	282	368	44
Characteristic bond resistance in cracked concrete C20/25 Temperature range I: dry and wet concrete flooded bore hole Temperature range I: flooded bore hole Temperature range II: flooded bore hole Temperature range III: flooded bore flooded flooded bore flooded	Stainless steel A4 and HCF	₹,	$N_{Rk,s} = N_{Rk,s,seis}^0$	[kN]	59	110	171	247	230	28
Tips.cr [N/mm²] 7.5 6.5 6.0 5.5 5.5	Combined pull-out and co	oncrete failure								
Temperature range I: 40°C/24°C flooded bore hole	Characteristic bond resista	nce in cracked concrete C2	20/25							
Temperature range I: 40°C/24°C flooded bore hole		dry and wat concrete	$ au_{ m Rk,cr}$	[N/mm ²]	7,5	6,5	6,0	5,5	5,5	5,5
Flooded bore hole Foreign Flooded bore hole Foreign Flooded bore hole Floo	Femperature range I: 40°C/24°C		$ au^0_{ m Rk,seis}$	[N/mm²]	5,2	4,4	4,1	3,9	3,9	3,9
Temperature range II: 60°C/43°C Temperature range II: 60°C/43°C Temperature range II: 60°C/43°C Temperature range III: 60°C/43°C Temperature range III: 60°C/43°C Temperature range III: 70°C/18,seis Temperature range III: 70°C/18,seis Temperature range III: 70°C/18,seis Temperature range III: 72°C/43°C Temperature range III: 74°C/43°C Temper				[N/mm ²]	7,5	6,0	5,0	4,5	4,0	4,0
Temperature range II: 60°C/43°C Tolk costs Tolk c			τ ⁰ _{Rk,seis}	+	 '			<u> </u>		2,8
## Flooded bore hole	dry and wet concrete			-	ł	· '				3,5
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$			τ ⁰ _{Rk,seis}	+	· ·	· ·		,	<u> </u>	2,
Temperature range III: dry and wet concrete Trik.cr [N/mm²] 4,0 3,5 3,0 3	00 0/10 0	flooded bore hole		-	<u> </u>	<u> </u>				3, 2,
					1					3,
$ \frac{ \tau_{\text{Fk,cr}} }{ \tau^{\circ}_{\text{Fk,sols}} } = \frac{ \tau_{\text{Fk,cr}} }{ \tau^{\circ}_{\text{Fk,sols}} } = \frac{ \left[\text{N/mm}^{2} \right] }{ \tau^{\circ}_{\text{Fk,sols}} } = \frac{ \left[\text{N/mm}^{2} \right] }{ 2,9} = \frac{2,4}{2,4} = \frac{2,2}{2,1} = \frac{2,1}{2,1} = 2,1$	Tomporatura rango III:	dry and wet concrete	-	+	- '	<u> </u>		,		2,
flooded bore hole $\tau^0_{\text{Rik,selis}}$ $[\text{N/mm}^2]$ 2,9 2,4 2,2 2,1 2,1 2,1 2,1 2,0 2,4 2,2 2,1 2,1 2,1 2,1 2,0 2,4 2,2 2,1				+				· ·		3,0
Increasing factors for concrete (only static or quasi-static actions) $V_{c} = \frac{C40/50}{C50/60} = \frac{1,08}{1,10}$ Factor according to CEN/TS 1992-4-5 Section 6.2.2.3 $k_{8} = \begin{bmatrix} -1 \\ -1 \end{bmatrix} = \frac{7,2}{7,2}$ Concrete cone failure Factor according to CEN/TS 1992-4-5 Section 6.2.3.1 $k_{cr} = \begin{bmatrix} -1 \\ -1 \end{bmatrix} = \frac{7,2}{7,2}$ Edge distance $c_{cr,N} = \begin{bmatrix} -1 \\ -1 \end{bmatrix} = \frac{7,2}{7,2}$ Edge distance $c_{cr,N} = \begin{bmatrix} -1 \\ -1 \end{bmatrix} = \frac{1,5 h_{el}}{3,0 h_{el}}$ Splitting failure Edge distance $c_{cr,sp} = \begin{bmatrix} -1 \\ -1 \end{bmatrix} = \frac{1,0 \cdot h_{ef}}{1,0 \cdot h_{ef}} \le 2 \cdot h_{ef} \left(2,5 - \frac{h}{h_{ef}} \right) \le 2,4 \cdot h_{ef}$ Axial distance $c_{cr,sp} = \begin{bmatrix} -1 \\ -1 \end{bmatrix} = \frac{1,0 \cdot h_{ef}}{1,0 \cdot h_{ef}} \le 2 \cdot h_{ef} \left(2,5 - \frac{h}{h_{ef}} \right) \le 2,4 \cdot h_{ef}$ Installation safety factor (dry and wet concrete) $c_{r,sp} = \begin{bmatrix} -1 \\ -1 \end{bmatrix} = \frac{1,0 \cdot h_{ef}}{1,0 \cdot h_{ef}} \le 2 \cdot h_{ef} \left(2,5 - \frac{h}{h_{ef}} \right) \le 2,4 \cdot h_{ef}$	flooded bore hole		-	[N/mm²]	2,9	2,4	2,2	2,1	2,1	2,
$\begin{array}{c} \text{(only static or quasi-static actions)} \\ \text{V_0} \\ \\ \text{$C50/60$} \\ \\ \text{$C50/60$} \\ \\ \text{$C50/60$} \\ \\ \text{$C50/60$} \\ \\ \text{$1,10$} \\ \\ \text{$C50/60$} \\ \\ \text{$1,10$} \\ \\ \text{$C50/60$} \\ \\ \text{$1,10$} \\ \\ \text{$C60/7S$ 1992-4-5 Section 6.2.2.3} \\ \\ \text{$Concrete cone failure} \\ \\ \text{$Factor according to} \\ \text{CEN/TS 1992-4-5 Section 6.2.3.1} \\ \text{K_{cr}} \\ \text{$[-]$} \\ \text{$7,2$} \\ \\ \text{CEN/TS 1992-4-5 Section 6.2.3.1} \\ \text{$Edge distance} \\ \text{$C_{cr,N}$} \\ \text{$[mm]$} \\ \text{$1,5$ h_{el}} \\ \text{$Axial distance} \\ \text{$Splitting failure} \\ \\ \text{$Edge distance} \\ \text{$C_{cr,sp}$} \\ \text{$[mm]$} \\ \text{$1,0$ \cdot h_{ef}} \leq 2 \cdot h_{ef} \left(2,5 - \frac{h}{h_{ef}}\right) \leq 2,4 \cdot h_{ef} \\ \text{$Axial distance} \\ \text{$S_{cr,sp}$} \\ \text{$[mm]$} \\ \text{2 $c_{cr,sp}$} \\ \\ \text{$Installation safety factor (dry and wet concrete)} \\ \text{γ_2} \\ \text{$1,2$} \\ \text{$1,4$} \\ \\ $1,4$$	Increasing factors for cons	roto	C30/37				1,	04		
Factor according to CEN/TS 1992-4-5 Section 6.2.2.3 k_8 [-] 7.2 Concrete cone failure Factor according to CEN/TS 1992-4-5 Section 6.2.3.1 k_{cr} [-] 7.2 Edge distance Cor.N [mm] $1.5 h_{ef}$ Axial distance $s_{cr.N}$ [mm] $3.0 h_{ef}$ Splitting failure Edge distance $c_{cr.sp}$ [mm] $1.0 \cdot h_{ef} \le 2 \cdot h_{ef} \left(2.5 - \frac{h}{h_{ef}} \right) \le 2.4 \cdot h_{ef}$ Axial distance $s_{cr.sp}$ [mm] $2 c_{cr.sp}$ Installation safety factor (dry and wet concrete) γ_2 1.2 1.4	(only static or quasi-static a		C40/50		1,08					
CEN/TS 1992-4-5 Section 6.2.2.3 R_8 Γ	$\Psi_{ m c}$		C50/60				1,	10		
Factor according to CEN/TS 1992-4-5 Section 6.2.3.1 k_{cr} [-] 7,2 Edge distance $c_{cr,N}$ [mm] 1,5 h_{el} Axial distance $s_{cr,N}$ [mm] 3,0 h_{el} Splitting failure Edge distance $c_{cr,sp}$ [mm] $c_{$		6.2.2.3	k ₈	[-]			7	,2		
CEN/TS 1992-4-5 Section 6.2.3.1 Edge distance $c_{cr,N}$ [mm] $c_{cr,N}$ [mm] Axial distance Splitting failure Edge distance $c_{cr,sp}$ [mm] $c_{cr,sp}$ $c_{cr,sp}$ [mm] $c_{cr,sp}$ $c_{$	Concrete cone failure									
Edge distance $c_{cr,N}$ [mm]		6.2.3.1	k _{cr}	[-]			7	,2		
Splitting failure Edge distance $c_{cr,sp}$ $[mm]$ $1,0 \cdot h_{ef} \le 2 \cdot h_{ef} \left(2,5 - \frac{h}{h_{ef}}\right) \le 2,4 \cdot h_{ef}$ Axial distance $s_{cr,sp}$ $[mm]$ $2 \cdot c_{cr,sp}$ Installation safety factor (dry and wet concrete) γ_2 $1,2$ $1,4$			C _{cr,N}	[mm]	1,5 h _{ef}					
Edge distance $ c_{cr,sp} \qquad [mm] \qquad 1,0 \cdot h_{ef} \leq 2 \cdot h_{ef} \left(2,5 - \frac{h}{h_{ef}}\right) \leq 2,4 \cdot h_{ef} $ Axial distance $ s_{cr,sp} \qquad [mm] \qquad 2 \cdot c_{cr,sp} $ Installation safety factor (dry and wet concrete) $ \gamma_2 \qquad \qquad 1,2 \qquad 1,4 $	Axial distance	S _{cr,N}	[mm]	3,0 h _{ef}						
Axial distance $s_{cr,sp}$ [mm] $2 c_{cr,sp}$ Installation safety factor (dry and wet concrete) γ_2 1,2 1,4	Splitting failure			<u> </u>						
Installation safety factor (dry and wet concrete) γ_2 1,2 1,4	Edge distance	C _{cr,sp}	[mm]	$1.0 \cdot h_{ef} \le 2 \cdot h_{ef} \left(2.5 - \frac{h}{h_{ef}} \right) \le 2.4 \cdot h_{ef}$						
	Axial distance		S _{cr,sp}							
Installation exfects (feeder (fleeded bare hele)	Installation safety factor (dr	ry and wet concrete)	γ ₂	- I	1	,2		1	,4	
installation safety factor (nooded bore note) γ_2	Installation safety factor (flo	ooded bore hole)	γ2				1	,4		

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Table C9:	Characteristic values of resistance for threaded rods under shear loads in cracked
	and non-cracked concrete (Design according to CEN/TS 1992-4 or TR 045)

Anchor size threaded rod			М 8	M 10	M 12	M 16	M 20	M24	M 27	M 30	
Steel failure without lever arm			l		ı	l	l	l			
Characteristic shear resistance,	$V_{Rk,s}$	[kN]	7	12	17	31	49	71	92	112	
Steel, property class 4.6	$V^0_{\text{Rk,s,seis}}$	[kN]	-	-	12	22	34	50	65	78	
Characteristic shear resistance,	$V_{Rk,s}$	[kN]	9	15	21	39	61	88	115	140	
Steel, property class 5.8	$V^0_{\text{Rk,s,seis}}$	[kN]	-	-	15	27	43	62	81	98	
Characteristic shear resistance,	$V_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224	
Steel, property class 8.8	$V^0_{Rk,s,seis}$	[kN]	-	-	24	44	69	99	129	157	
Characteristic shear resistance, Stainless steel A4 and HCR.	$V_{Rk,s}$	[kN]	13	20	30	55	86	124	115	140	
property class 50 (>M24) and 70 (≤ M24)	V ⁰ _{Rk,s,seis}	[kN]	-	-	21	39	60	87	81	98	
Ductility factor according to CEN/TS 1992-4-5 Section 6.3.2.1	k ₂		0,8								
Steel failure with lever arm											
Characteristic bending moment,	M ⁰ _{Rk,s}	[Nm]	15	30	52	133	260	449	666	900	
Steel, property class 4.6	M ⁰ _{Rk,s,seis} [Nm]				No Perfe	ormance [Determine	ed (NPD)			
Characteristic bending moment,	$M^0_{Rk,s}$	[Nm]	19 37 65 166 324 560 833							1123	
Steel, property class 5.8	$M^0_{Rk,s,seis}$	[Nm]	No Performance Determined (NPD)								
Characteristic bending moment,	$M^0_{Rk,s}$	[Nm]	30	60	105	266	519	896	1333	1797	
Steel, property class 8.8	$M^0_{Rk,s,seis}$	[Nm]	No Performance Determined (NPD)								
Characteristic bending moment,	M ⁰ _{Rk,s}	[Nm]	26	52	92	232	454	784	832	1125	
Stainless steel A4 and HCR, property class 50 (>M24) and 70 (\leq M24) $M^0_{Rk,s,seis}$ [Nm]			No Performance Determined (NPD)								
Concrete pry-out failure	-										
Factor in equation (27) of CEN/TS 1992-4-5 Section 6.3.3	k ₃	2,0									
Installation safety factor	γ2					1	,0				
Concrete edge failure ³⁾											
Effective length of anchor	I _t	[mm]				I _f = min(h	et; 8 d _{nom})				
Outside diameter of anchor	d _{nom}	[mm]	8	10	12	16	20	24	27	30	
Installation safety factor	γ ₂					1	,0				

Ripple Injection	System	R-Fix for	concrete
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Performances

Characteristic values of resistance for threaded rods under shear loads in cracked and non-cracked concrete, (Design according to CEN/TS 1992-4 or TR 045)

Annex C 9



Anchor size reinforcing b	Anchor size reinforcing bar					Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
Steel failure													
Characteristic tension resis	tance	N _{Rk,s}	[kN]	A _s · f _{uk}									
Combined pull-out and co	oncrete failure		1										
Characteristic bond resistar	nce in non-cracked concre	te C20/2	5										
Temperature range I:	dry and wet concrete	τ _{Rk,ucr}	[N/mm²]	14	14	13	13	12	12	11	11	11	
40°C/24°C	flooded bore hole	$ au_{Rk,ucr}$	[N/mm²]	14	13	11	10	9,5	8,5	7,5	7,0	6,0	
Temperature range II:	dry and wet concrete	$ au_{Rk,ucr}$	[N/mm²]	8,5	8,5	8,0	8,0	7,5	7,0	7,0	6,5	6,5	
60°C/43°C	flooded bore hole	$ au_{ m Rk,ucr}$	[N/mm²]	8,5	8,5	8,0	8,0	7,5	7,0	6,0	5,5	5,0	
Temperature range III:	dry and wet concrete	τ _{Rk,ucr}	[N/mm²]	7,5	7,5	7,5	7,0	7,0	6,5	6,0	6,0	6,0	
72°C/43°C	flooded bore hole	$ au_{ m Rk,ucr}$	[N/mm²]	7,5	7,5	7,5	7,0	7,0	6,0	5,5	5,0	4,5	
-		C30/37		1,04									
Increasing factors for concr ψ_c	rete	C40/50		1,08									
		C50/60	C50/60 1,10										
Factor according to CEN/TS 1992-4-5 Section (6.2.2.3	k ₈	[-]	10,1									
Concrete cone failure													
Factor according to CEN/TS 1992-4-5 Section (6.2.3.1	k _{ucr}	[-]	10,1									
Edge distance		C _{cr,N}	[mm]	1,5 h _{et}									
Axial distance			[mm]					3,0 h _{ef}					
Splitting failure													
Edge distance			[mm]	$1.0 \cdot h_{ef} \le 2 \cdot h_{ef} \left(2.5 - \frac{h}{h_{ef}} \right) \le 2.4 \cdot h_{ef}$									
Axial distance			[mm]	2 C _{cr,sp}									
Installation safety factor (dr	y and wet concrete)	γ ₂	•	1,2			1	1,4					
Installation safety factor (flo	oded bore hole)	γ ₂		1,4									

Ripple Injection System R-Fix for concrete	
Performances Characteristic values of resistance for rebar under tension loads in non-cracked concrete (Design according to CEN/TS 1992-4)	Annex C 10



Anchor size reinforcing	bar			Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Steel failure				•			•	•		•
Characteristic tension res	sistance	N _{Rk,s} =N ⁰ _{Rk,s,seis}	[kN]				A _s • f _{uk}			
Combined pull-out and	concrete failure									
Characteristic bond resist	tance in cracked concrete	: C20/25								
		Tou	[N/mm²]	7,5	7,0	6,5	6,0	5,5	5,5	5.5
	dry and wet concrete	τ _{Rk,cr} τ ⁰ _{Rk,seis}	ļ · · · ·							,
Temperature range I: 40°C/24°C	range I:		[N/mm²]	5,2	4,7	4,4	4,1	3,9	3,9	3,9
10 0/21 0	flooded bore hole	τ _{Rk,cr}	[N/mm ²]	7,5	6,5	6,0	5,0	4,5	4,0	4,0
		τ ⁰ _{Rk,seis}	[N/mm ²]	5,2	4,6	4,1	3,5	3,0	2,9	2,7
dry and wet concrete		τ _{Rk,cr}	[N/mm²]	4,5	4,0	4,0	3,5	3,5	3,5	3,5
		$ au^0_{ m Rk,seis}$	[N/mm ²]	3,2	2,8	2,7	2,5	2,3	2,3	2,3
60°C/43°C			[N/mm²]	4,5	4,0	4,0	3,5	3,5	3,5	3,0
flooded bore hole		τ ⁰ _{Rk,seis}	[N/mm²]	3,2	2,8	2,7	2,5	2,3	2,3	2,3
		τ _{Rk,cr}	[N/mm²]	4,0	3,5	3,5	3,0	3,0	3,0	3,0
	dry and wet concrete	τ ⁰ _{Rk,seis}	[N/mm²]	2,9	2,6	2,4	2,2	2,1	2,1	2,1
Temperature range III: 72°C/43°C			[N/mm²]	4,0	3,5	3,5	3,0	3,0	3.0	3,0
	flooded bore hole	τ _{Rk,cr}	<u> </u>	,	<u> </u>	-		<u> </u>	,	
		τ ⁰ _{Rk,seis}	[N/mm ²]	2,9	2,6	2,4	2,2	2,1	2,1	2,1
Increasing factors for con (only static or quasi-static		C30/37 C40/50					1,04			
ψ_c	actions)	C50/60		1,10						
Factor according to CEN/TS 1992-4-5 Section	n 6 2 2 3	k ₈	[-]	7,2						
Concrete cone failure	11 0.2.2.0		1	l						
Factor according to		k _{cr}	[-]				7,2			
CEN/TS 1992-4-5 Section	n 6.2.3.1									
Edge distance Axial distance		C _{cr,N}	[mm]	1,5 h _{ef}						
		S _{cr,N}	[mm]				3,0 h _{ef}			
Splitting failure		T	1	Ι						
Edge distance		C _{cr,sp}	[mm]		1,0 ·	$h_{ef} \le 2 \cdot h$	$\frac{h}{h_{ef}}$ $\leq 2.4 \cdot h_{ef}$			
Axial distance		S _{cr,sp}	[mm]				2 c _{cr,sp}			
Installation safety factor (dry and wet concrete)	γ2			1,2			1	,4	
Installation safety factor (flooded bore hole)	γ ₂					1,4			

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Characteristic values of resistance for rebar under tension loads in cracked concrete

(Design according to CEN/TS 1992-4 or TR 045)

Installation safety factor



1,0

Table C12: Characteristic values of resistance for rebar under shear loads in crawand non-cracked concrete (Design according to CEN/TS 1992-4 or Telephone)												
Anchor size reinforcing bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
Steel failure without lever arm												
	$V_{Rk,s}$	[kN]	0,50 · A _s · f _{uk}									
Characteristic shear resistance	V ⁰ _{Rk,s,seis}	[kN]	0,35 • A _s • f _{uk}									
Ductility factor according to CEN/TS 1992-4-5 Section 6.3.2.1	k ₂		0,8									
Steel failure with lever arm			•									
Observation be realised as a second	M ^o _{Fik,s} [Nm] 1.2 • W _{el} • f _{uk}											
Characteristic bending moment	M ⁰ _{Rk,s,seis}	[Nm]		No Performance Determined (NPD)								
Concrete pry-out failure		•	•									
Factor in equation (27) of CEN/TS 1992-4-5 Section 6.3.3	k ₃		2,0									
Installation safety factor	γ2	γ ₂ 1,0										
Concrete edge failure			•									
Effective length of anchor	I _f	[mm]				I _f = m	nin(h _{ef} ; 8	d _{nom})				
Outside diameter of anchor	d _{nom}	[mm]	8	10	12	14	16	20	24	27	30	

γ2

Ripple Injection System R-Fix for concrete	
Performances Characteristic values of resistance for rebar under shear loads in cracked and non-cracked concrete, (Design according to CEN/TS 1992-4 or TR 045)	Annex C 12



Anchor size threaded rod			M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Non-cracked conc	rete C20/25									
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,011	0,013	0,015	0,020	0,024	0,029	0,032	0,035
40°C/24°C	δ _{N∞} -factor	[mm/(N/mm²)]	0,044	0,052	0,061	0,079	0,096	0,114	0,127	0,140
Temperature range II: 60°C/43°C	δ_{N0} -factor	[mm/(N/mm²)]	0,013	0,015	0,018	0,023	0,028	0,033	0,037	0,043
	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,050	0,060	0,070	0,091	0,111	0,131	0,146	0,161
Temperature range III: 72°C/43°C	δ_{N0} -factor	[mm/(N/mm²)]	0,013	0,015	0,018	0,023	0,028	0,033	0,037	0,043
	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,050	0,060	0,070	0,091	0,111	0,131	0,146	0,161
Cracked concrete	C20/25									
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]			0,032	0,037	0,042	0,048	0,053	0,058
40°C/24°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]		-	0,21	0,21	0,21	0,21	0,21	0,21
Temperature range II: 60°C/43°C	δ_{N0} -factor	[mm/(N/mm²)]			0,037	0,043	0,049	0,055	0,061	0,067
	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]			0,24	0,24	0,24	0,24	0,24	0,24
Temperature range III: 72°C/43°C	δ_{N0} -factor	[mm/(N/mm²)]			0,037	0,043	0,049	0,055	0,061	0,067
	δ _{N∞} -factor	[mm/(N/mm²)]		-	0,24	0,24	0,24	0,24	0,24	0,24

¹⁾ Calculation of the displacement

 $\delta_{\text{N0}} = \delta_{\text{N0}}\text{-factor} \ \cdot \tau;$ $\delta_{N\infty} = \delta_{N\infty}$ -factor $\cdot \tau$;

Table C14: Displacements under shear load (threaded rod)

Anchor size threaded rod			М 8	M 10	M 12	M 16	M 20	M24	M 27	М 30
For concrete C20	/25									
All temperature	δ_{V0} -factor	[mm/(kN)]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V\infty}$ -factor	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05

 $[\]begin{array}{l} ^{1)} \mbox{ Calculation of the displacement} \\ \delta_{V0} = \delta_{V0}\mbox{-factor } \cdot \mbox{ V}; \\ \delta_{V\infty} = \delta_{V\infty}\mbox{-factor } \cdot \mbox{ V}; \end{array}$

Ripple Injection System R-Fix for concrete	
Performances	Annex C 13
Displacements (threaded rods)	



Table C15: Displacements under tension load ¹⁾ (rebar)											
Anchor size reinfo	Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32		
Non-cracked cond	crete C20/2	25									
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,011	0,013	0,015	0,018	0,020	0,024	0,030	0,033	0,037
40°C/24°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,044	0,052	0,061	0,070	0,079	0,096	0,118	0,132	0,149
Temperature range II: 60°C/43°C	δ_{N0} -factor	[mm/(N/mm²)]	0,013	0,015	0,018	0,020	0,023	0,028	0,034	0,038	0,043
	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,050	0,060	0,070	0,081	0,091	0,111	0,136	0,151	0,172
Temperature range III: 72°C/43°C	δ_{N0} -factor	[mm/(N/mm²)]	0,013	0,015	0,018	0,020	0,023	0,028	0,034	0,038	0,043
	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,050	0,060	0,070	0,081	0,091	0,111	0,136	0,151	0,172
Cracked concrete	C20/25										
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]			0,032	0,035	0,037	0,042	0,049	0,055	0,061
40°C/24°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	·	-	0,21	0,21	0,21	0,21	0,21	0,21	0,21
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]			0,037	0,040	0,043	0,049	0,056	0,063	0,070
60°C/43°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]			0,24	0,24	0,24	0,24	0,24	0,24	0,24
Temperature range III: 72°C/43°C	δ_{N0} -factor	[mm/(N/mm²)]			0,037	0,040	0,043	0,049	0,056	0,063	0,070
	$\delta_{\text{N}_{\infty}}\text{-factor}$	[mm/(N/mm²)]		-	0,24	0,24	0,24	0,24	0,24	0,24	0,24

 $^{^{1)}}$ Calculation of the displacement $\delta_{N0}=\delta_{N0}\text{-factor}\ \cdot \tau;$

 $\delta_{N\infty} = \delta_{N\infty} \text{-factor } \cdot \tau;$

Table C16: Displacement under shear load 1) (rebar)

Anchor size reinforcing bar				Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
For concrete C20/25											
All temperature	δ_{V0} -factor	[mm/(kN)]	0,06	0,05	0,05	0,04	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V_{\infty}}$ -factor	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,04	0,04

¹⁾ Calculation of the displacement

$$\begin{split} &\delta_{\text{V0}} = \delta_{\text{V0}}\text{-factor} &\cdot \text{V}; \\ &\delta_{\text{V}_{\infty}} = \delta_{\text{V}_{\infty}}\text{-factor} &\cdot \text{V}; \end{split}$$

Ripple Injection System R-Fix for concrete	
Performances	Annex C 14
Displacements (rebar)	