



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-15/0769 of 12 November 2015

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the Deutsches Institut für Bautechnik **European Technical Assessment:** Trade name of the construction product ABZ injection system FIX-V or FIX-V Winter Product family Bonded anchor for use in concrete to which the construction product belongs Manufacturer ABZ-Zaunsysteme GmbH Virmondstraße 137 47877 Willich DEUTSCHLAND Manufacturing plant ABZ-Zaunsysteme GmbH Plant 1 This European Technical Assessment 20 pages including 3 annexes which form an integral part contains of this assessment This European Technical Assessment is Guideline for European technical approval of "Metal anchors for use in concrete", ETAG 001 Part 5: "Bonded issued in accordance with Regulation (EU) anchors", April 2013, No 305/2011, on the basis of used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011.

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European Technical Assessment ETA-15/0769

Page 2 of 20 | 12 November 2015

English translation prepared by DIBt

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Page 3 of 20 | 12 November 2015

Specific Part

1 Technical description of the product

The "ABZ Injection system FIX-V or FIX-V Winter for concrete" is a bonded anchor consisting of a cartridge with injection mortar ABZ-FIX V or ABZ-FIX V Winter and a steel element. The steel element consist of a commercial threaded rod with washer and hexagon nut in the range of M8 to M30 or a reinforcing bar in the range of diameter 8 to 32 mm.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between metal part, injection mortar and concrete.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance tension and shear loads	See Annex C 1 to C 4
Displacements under tension and shear loads	See Annex C 5 / C 6

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	No performance assessed

3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances there may be requirements (e.g. transposed European legislation and national laws, regulations and administrative provisions) applicable to the products falling within the scope of this European Technical Assessment. In order to meet the provisions of Regulation (EU) No 305/2011, these requirements need also to be complied with, when and where they apply.

3.4 Safety in use (BWR 4)

The essential characteristics regarding Safety in use are included under the Basic Works Requirement Mechanical resistance and stability.



European Technical Assessment ETA-15/0769

Page 4 of 20 | 12 November 2015

English translation prepared by DIBt

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with guideline for European technical approval ETAG 001, April 2013 used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011 the applicable European legal act is: [96/582/EC]. The system to be applied is: 1

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable EAD

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

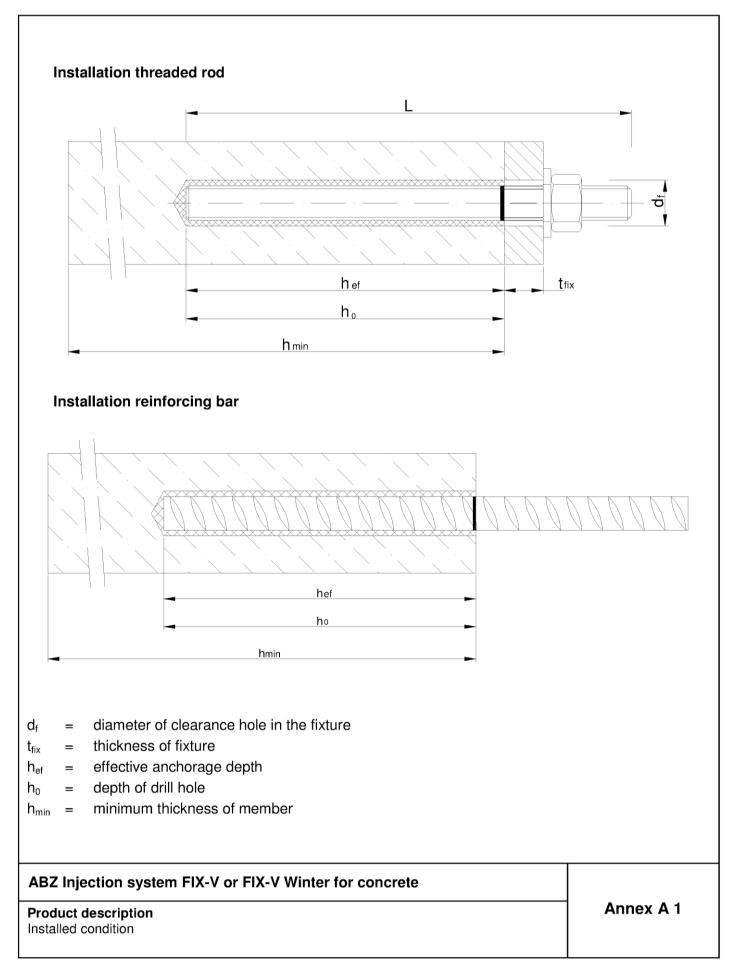
Issued in Berlin on 12 November 2015 by Deutsches Institut für Bautechnik

Uwe Bender Head of Department *beglaubigt:* G. Lange

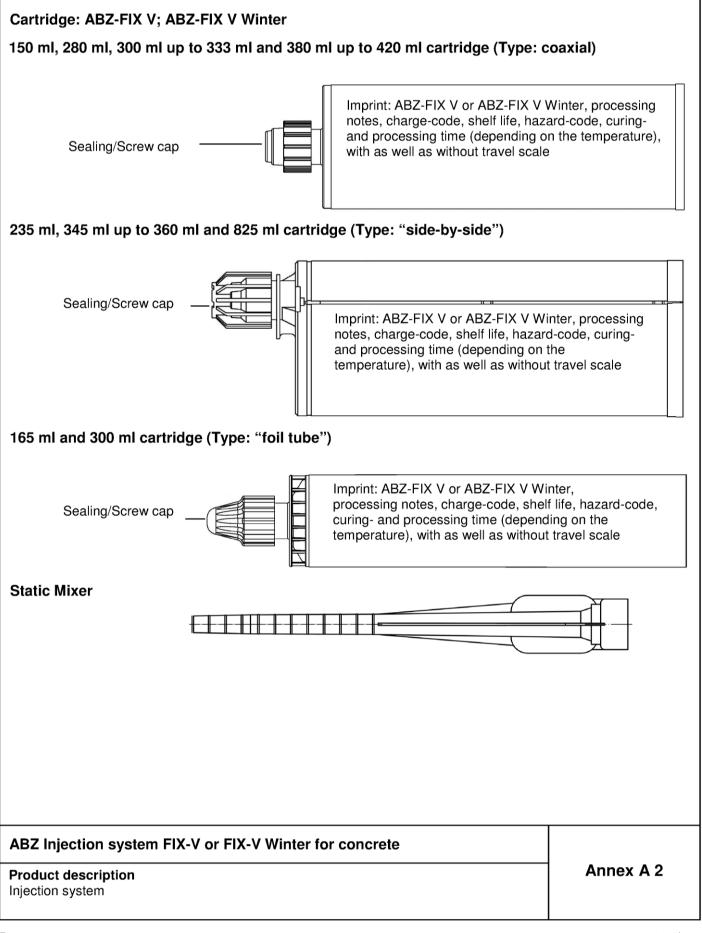
Page 5 of European Technical Assessment ETA-15/0769 of 12 November 2015

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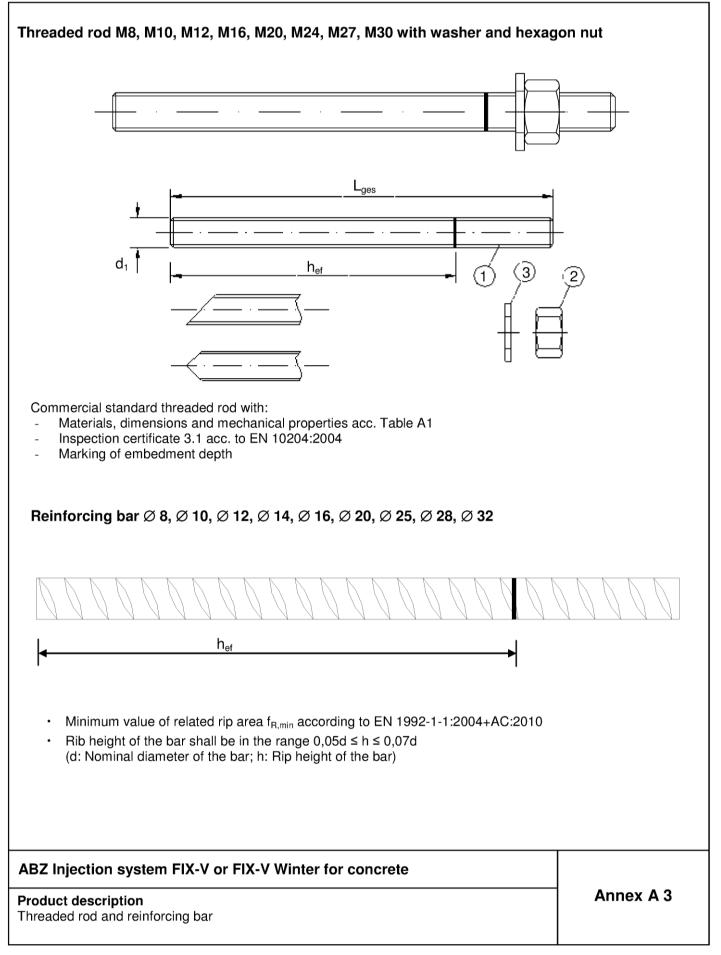




Table A1: Materials

Part	Designation	Material					
	zinc plated ≥ 5 μm acc. to EN ISO 4042:19						
Steel,	hot-dip galvanised ≥ 40 μm acc. to EN ISC	O 1461:2009 and EN ISO 10684:2004+A	C:2009				
1	Anchor rod	Steel, EN 10087:1998 or EN 10263:200 Property class 4.6, 4.8, 5.8, 8.8, EN 199 $A_5 > 8\%$ fracture elongation					
2	Hexagon nut, EN ISO 4032:2012	Steel acc. to EN 10087:1998 or EN 102 Property class 4 (for class 4.6 or 4.8 rod Property class 5 (for class 5.8 rod) EN IS Property class 8 (for class 8.8 rod) EN IS) EN ISO 898-2:2012, SO 898-2:2012,				
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Steel, zinc plated or hot-dip galvanised					
Stain	less steel						
1	Anchor rod	Material 1.4401 / 1.4404 / 1.4571, EN 10 > M24: Property class 50 EN ISO $3506^{-1} \le M24$: Property class 70 EN ISO $3506^{-1} A_5 > 8\%$ fracture elongation	1:2009				
2	Hexagon nut, EN ISO 4032:2012	Material 1.4401 / 1.4404 / 1.4571 EN 10088:2005, > M24: Property class 50 (for class 50 rod) EN ISO 3506-2:2009 ≤ M24: Property class 70 (for class 70 rod) EN ISO 3506-2:2009					
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4401, 1.4404 or 1.4571, EN 1	10088-1:2005				
High	corrosion resistance steel						
1	Anchor rod	Material 1.4529 / 1.4565, EN 10088-1:20 > M24: Property class 50 EN ISO 3506- \leq M24: Property class 70 EN ISO 3506- A ₅ > 8% fracture elongation	1:2009				
2	Hexagon nut, EN ISO 4032:2012	Material 1.4529 / 1.4565 EN 10088-1:20 > M24: Property class 50 (for class 50 ro ≤ M24: Property class 70 (for class 70 ro	od) EN ISO 3506-2:2009				
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4529 / 1.4565, EN 10088-1:20	005				
Reinf	orcing bars						
1	Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C f_{yk} and k according to NDP or NCL of EN $f_{uk} = f_{tk} = k \cdot f_{yk}$	1992-1-1/NA:2013				
ABZ	Injection system FIX-V or FIX-V Winte	er for concrete					
	luct description		Annex A 4				



Specifications of intended use

Anchorages subject to:

- Static and quasi-static loads: M8 to M30, Rebar Ø8 to Ø32.
- Seismic action for Performance Category C1: M12 to M30, Rebar Ø12 to Ø32.

Base materials:

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000.
- Strength classes C20/25 to C50/60 according to EN 206-1:2000.
- · Non-cracked concrete: M8 to M30, Rebar Ø8 to Ø32.
- Cracked concrete: M12 to M30, Rebar Ø12 to Ø32.

Temperature Range:

- I: 40 °C to +40 °C (max long term temperature +24 °C and max short term temperature +40 °C)
- II: 40 °C to +80 °C (max long term temperature +50 °C and max short term temperature +80 °C)
- III: 40 °C to +120 °C (max long term temperature +72 °C and max short term temperature +120 °C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc coated steel, stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other
 particular aggressive conditions exist

(high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to supports, etc.).
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Anchorages under static or quasi-static actions are designed in accordance with:
 - EOTA Technical Report TR 029 "Design of bonded anchors", Edition September 2010 or
 - CEN/TS 1992-4:2009
- Anchorages under seismic actions are designed in accordance with:
 - EOTA Technical Report TR 045 "Design of Metal Anchors under Seismic Action", Edition February 2013
 - Anchorages shall be positioned outside of critical regions (e.g. plastic hinges) of the concrete structure.
 - Fastenings in stand-off installation or with a grout layer are not allowed.

Installation:

- Dry or wet concrete: M8 to M30, Rebar Ø8 to Ø32.
- Flooded holes (not sea water): M8 to M16, Rebar Ø8 to Ø16.
- Hole drilling by hammer or compressed air drill mode.
- · Overhead installation allowed.
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.

ABZ Injection system FIX-V or FIX-V Winter for concrete

Intended Use

Specifications



Anchor size		M 8	M 10	M 12	M 16	M 20	M 24	M 27	M 30
Nominal drill hole diameter	d ₀ [mm] =	10	12	14	18	24	28	32	35
Effective encharge depth	h _{ef,min} [mm] =	60	60	70	80	90	96	108	120
Effective anchorage depth	h _{ef,max} [mm] =	160	200	240	320	400	480	540	600
Diameter of clearance hole in the fixture $d_f [mm] \le 9$ 1214182226				26	30	33			
Diameter of steel brush	Diameter of steel brush $d_b \text{[mm]} \ge 12$ 14 16 20 26 30 3				34	37			
Torque moment	T _{inst} [Nm] ≤	10	20	40	80	80 120 160 180 200			200
Thickness of fixture	t _{fix,min} [mm] >				()	-	-	
Thickness of fixture	t _{fix,max} [mm] <				15	00			
Minimum thickness of member	h _{min} [mm]	$\frac{h_{ef} + 30 \text{ mm}}{\geq 100 \text{ mm}} \qquad h_{ef} + 2d_0$							
Minimum spacing	s _{min} [mm]	40	50	60	80	100	120	135	150
Minimum edge distance	c _{min} [mm]	40	50	60	80	100	120	135	150

Table B2: Installation parameters for rebar

Rebar size		Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Nominal drill hole diameter	d ₀ [mm] =	12	14	16	18	20	24	32	35	40
Effective anchorage depth	$h_{ef,min} [mm] =$	60	60	70	75	80	90	100	112	128
	h _{ef,max} [mm] =	160	200	240	280	320	400	480	540	640
Diameter of steel brush	d _b [mm] ≥	14	16	18	20	22	26	34	37	41,5
Minimum thickness of member	h _{min} [mm]	h _{ef} + 30 mm ≥ 100 mm					h _{ef} + 2d ₀)		
Minimum spacing	s _{min} [mm]	40	50	60	70	80	100	125	140	160
Minimum edge distance	c _{min} [mm]	40	50	60	70	80	100	125	140	160

ABZ Injection system FIX-V or FIX-V Winter for concrete

Intended Use Installation parameters Annex B 2



Steel brush



Table B3: Parameter cleaning and setting tools

Threaded Rod	Rebar	d₀ Drill bit - Ø	d _⊳ Brush - Ø	d _{b,min} min. Brush - Ø	Piston plug	
(mm)	(mm)	(mm)	(mm)	(mm)	(No.)	
M8		10	12	10,5		
M10	8	12	14	12,5		
M12	10	14	16	14,5	No	
	12	16	18	16,5	piston plug required	
M16	14	18	20	18,5		
	16	20	22	20,5		
M20	20	24	26	24,5	# 24	
M24		28	30	28,5	# 28	
M27	25	32	34	32,5	# 32	
M30	28	35	37	35,5	# 35	
	32	40	41,5	40,5	# 38	



Hand pump (volume 750 ml) Drill bit diameter (d₀): 10 mm to 20 mm – uncracked concrete



Recommended compressed air tool (min 6 bar) Drill bit diameter (d_0): 10 mm to 40 mm



Piston plug for overhead or horizontal installation Drill bit diameter (d₀): 24 mm to 40 mm

ABZ Injection system FIX-V or FIX-V Winter for concrete

Intended Use Cleaning and setting tools

Annex B 3



Installation inst	ructions							
	1. Drill with hammer drill a hole into the base material to the size a depth required by the selected anchor (Table B1 or Table B2). I drill hole: the drill hole shall be filled with mortar							
	Attention! Standing water in the bore hole must be removed	d before cleaning.						
4x	2a. Starting from the bottom or back of the bore hole, blow the hole compressed air (min. 6 bar) or a hand pump (Annex B 3) a mini the bore hole ground is not reached an extension shall be used	mum of four times. If						
or	The hand-pump can only be used for anchor sizes in uncracked bore hole diameter 20mm or embedment depth up to 240mm.	d concrete up to						
Ax	Compressed air (min. 6 bar) can be used for all sizes in cracked concrete.	d and uncracked						
	2b. Check brush diameter (Table B3) and attach the brush to a drill or a battery screwdriver. Brush the hole with an appropriate size							
	> $d_{b,min}$ (Table B3) a minimum of four times. If the bore hole ground is not reached with the brush, a brush e							
4x	shall be used (Table B3).							
or	2c. Finally blow the hole clean again with compressed air (min. pump (Annex B 3) a minimum of four times. If the bore hole gro an extension shall be used. The hand-pump can <u>only</u> be used uncracked concrete up to bore hole diameter 20mm or embedn 240mm. Compressed air (min. 6 bar) can be used for all sizes i uncracked concrete.	und is not reached for anchor sizes in nent depth up to						
	After cleaning, the bore hole has to be protected against re							
4x	an appropriate way, until dispensing the mortar in the bore hole. If necessary, the cleaning repeated has to be directly before dispensing the mortar. In-flowing water must not contaminate the bore hole again.							
	3. Attach a supplied static-mixing nozzle to the cartridge and load correct dispensing tool. Cut off the foil tube clip before use. For every working interruption longer than the recommended w (Table B4 or B5) as well as for new cartridges, a new static-mix	orking time						
(nel degree operation and the state of the second state of the sec	4. Prior to inserting the anchor rod into the filled bore hole, the pose embedment depth shall be marked on the anchor rods.	sition of the						
min. 3 full stroke	5. Prior to dispensing into the anchor hole, squeeze out separately full strokes and discard non-uniformly mixed adhesive compone shows a consistent grey colour. For foil tube cartridges is must be minimum of six full strokes.	nts until the mortar						
ABZ Injection sys	tem FIX-V or FIX-V Winter for concrete							

Intended Use

Annex B 4

Installation instructions



	ead and horizontal installation a piston
7. Push the threaded rod or reinforcing bar in ensure positive distribution of the adhesiv	
The anchor should be free of dirt, grease,	oil or other foreign material.
	the bottom of the hole and that excess hese requirements are not maintained, the ead application the anchor rod should be
9. Allow the adhesive to cure to the specifie Do not move or load the anchor until it is	
10. After full curing, the add-on part can be in (Table B2) by using a calibrated torque w	

ABZ Injection system FIX-V or FIX-V Winter for concrete

Intended Use Installation instructions (continuation) Annex B 5



Table B4		aximum W 3Z-FIX V	orking time and minimum curing	time				
Concre	te temp	erature	Gelling- / working time	Minimum curing time in dry concrete ¹⁾				
-10 °C	to	-4°C	90 min ²⁾	24 h ²⁾				
-5 °C	to	-1°C	90 min	14 h				
0 °C	to	+4°C	45 min	7 h				
+5 °C	to	+9°C	25 min	2 h				
+ 10 °C	to	+19°C	15 min	80 min				
+ 20 °C	to	+29°C	6 min	45 min				
+ 30 °C	to	+34°C	4 min	25 min				
+ 35 °C	to	+39°C	2 min	20 min				
>	→ + 40 °C	;	1,5 min	15 min				
Cartride	ge temp	erature	+5°C to	°C to +40°C				
¹ In wet co ¹ Cartridge Table B5	tempera	ature must be aximum We	e must be doubled. e at min. +15°C. orking time and minimum curing	time				
Concret		3Z-FIX V W	/inter Gelling- / working time	Minimum curing time in dry concrete ¹⁾				

75 min

55 min

35 min

20 min

10 min

6 min

6 min

-20°C to +10°C

In wet concrete the curing time must be doubled.

-16°C

-11°C

-4°C

-1°C

+4°C

+9°C

ABZ Injection system FIX-V or FIX-V Winter for concrete

Intended Use Curing time

-20 °C

-15 °C

-10 °C

-5 °C

0°C

+5 °C

to

to

to

to

to

to

+ 10 °C

Cartridge temperature

Annex B 6

24 h

16 h

10 h

5 h

2,5 h

80 Min

60 Min



Anchor size threaded	rod			M 8	M 10	M 12	M 16	M 20	M24	M27	M30
Steel failure											
Characteristic tension re	esistance	N _{Rk,s} =N _{Rk,s,seis}	[kN]				$A_{s}\boldsymbol{\cdot} f_{uk}$				
Combined pull-out and	d concrete failure										
Characteristic bond resi	stance in non-cracked co	ncrete C20/25									
Temperature range I:	dry and wet concrete	$ au_{\mathrm{Rk},\mathrm{ucr}}$	[N/mm ²]	10	12	12	12	12	11	10	9
40°C/24°C	flooded bore hole								not adr	nissible	
Temperature range II:	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm ²]	7,5	9	9	9	9	8,5	7,5	6,5
80°C/50°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm ²]	5,5	6,5	6,5	6,5		not adr	nissible	
Temperature range III:	dry and wet concrete	$\tau_{\rm Rk,ucr}$	[N/mm ²]	5,5	6,5	6,5	6,5	6,5	6,5	5,5 5	
120°C/72°C	flooded bore hole	$\tau_{\rm Rk,ucr}$	[N/mm ²]	4,0	5,0	5,0	5,0		not adr	nissible	
Characteristic bond resi	stance in cracked concre	te C20/25									
	dry and wat concrete	$\tau_{Rk,cr}$	[N/mm ²]			5,5	5,5	5,5	5,5	6,5	6,5
Temperature range I:	dry and wet concrete	$\tau_{Rk,seis}$	[N/mm ²]		nissible	3,7	3,7	3,7	3,8	4,5	4,5
40°C/24°C	flooded bore hole	$ au_{Rk,cr}$	[N/mm ²]	not au	nissible	5,5	5,5		not admissible		
	housed bore hole	$\tau_{\text{Rk,seis}}$	[N/mm ²]			3,7	3,7		not admissible		
	dry and wet concrete	$\tau_{\rm Rk,cr}$	[N/mm ²]			4,0	4,0	4,0	4,0	4,5	4,5
Temperature range II:	dry and wet concrete	$\tau_{\text{Rk,seis}}$	[N/mm²]		nissible	2,7	2,7	2,7	2,8	3,1	3,1
30°C/50°C	flooded bore hole	$\tau_{\rm Rk,cr}$	[N/mm²]		IIISSIDIE	4,0	4,0		not adr	nissible	
	housed bore hole	$\tau_{Rk,seis}$	[N/mm ²]			2,7	2,7		not adr	nissible	
	dry and wet concrete	$\tau_{\rm Rk,cr}$	[N/mm²]	not admissible		3,0	3,0	3,0	3,0	3,5	3,5
Temperature range III:	dry and wet concrete	$\tau_{Rk,seis}$	[N/mm ²]			2,0	2,0	2,0	2,1	2,4	2,4
120°C/72°C	flooded bore hole	$\tau_{\rm Rk,cr}$	[N/mm ²]			3,0	3,0	not admissible			
		$ au_{Rk,seis}$	[N/mm ²]	2,0 2,0 not adn						nissible	
		C25/3	0	1,02							
		C30/3	7	1,04							
Increasing factors for co (only static or quasi-stat		C35/4	5	1,07							
ψ_c	,	C40/5	-	1,08							
		C45/5	_				,	09			
	1	C50/6	0					10			
Factor according to CEN/TS 1992-4-5	Non-cracked concrete	k ₈	[-]),1			
Section 6.2.2.3	Cracked concrete						7	,2			
Concrete cone failure											
Factor according to CEN/TS 1992-4-5	Non-cracked concrete	k _{ucr}	[-]				10),1			
Section 6.2.3.1	Cracked concrete	k _{cr}	[-]				7	,2			
Edge distance		C _{cr,N}	[mm]				1,5	i h _{ef}			
Axial distance		S _{cr,N}	[mm]				3,0	h _{ef}			
Installation safety factor	(dry and wet concrete)	γ2 = γinst		1,0				1,2			
Installation safety factor	(flooded bore hole)	$\gamma_2 = \gamma_{inst}$. 1	,4			not adr	nissible	

ABZ Injection system FIX-V or FIX-V Winter for concrete

Performances

Characteristic values of resistance for threaded rods under tension loads

Annex C 1



Anchor size threaded rod			M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Steel failure without lever arm										
	V _{Rk,s}	[kN]	0,50 • A _s • f _{uk}							
Characteristic shear resistance	V _{Rk,s,seis}	[kN]	not admissible				0,35 ·	A _s ∙ f _{uk}		
Ductility factor according to CEN/TS 1992-4-5 Section 6.3.2.1	k ₂					0	,8			
Steel failure with lever arm										
Oberesteristic banding memori	M ⁰ _{Rk,s}	[Nm]				1.2 • V	V _{el} ∙ f _{uk}			
Characteristic bending moment	M ⁰ _{Rk,s,seis}	[Nm]			No Perfo	ormance l	Determine	ed (NPD)		
Concrete pry-out failure										
Factor k₃ in equation (27) of CEN/TS 1992-4-5 Section 6.3.3 Factor k in equation (5.7) of Technical Report TR 029	k ₍₃₎	K ₍₃₎		2,0						
Installation safety factor	$\gamma_2 = \gamma_{inst}$		1,0							
Concrete edge failure										
Effective length of anchor	l,	[mm]				l _f = min(h	l _{ef} ; 8 d _{nom})			
Outside diameter of anchor	d _{nom}	[mm]	8	10	12	16	20	24	27	30
Installation safety factor	$\gamma_2 = \gamma_{inst}$					1	,0			

Performances

Characteristic values of resistance for threaded rods under shear loads

Annex C 2



Anchor size reinforcin	ıg bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Steel failure												
Characteristic tension re	esistance	N _{Rk,s} = N _{Rk,s,seis}	[kN]					$A_{s} \boldsymbol{\cdot} f_{uk}$				
Combined pull-out and	d concrete failure											
Characteristic bond resi	istance in non-cracked co	ncrete C20	/25									
Temperature range I:	dry and wet concrete	$\tau_{\text{Rk,ucr}}$	[N/mm²]	10	12	12	12	12	12	11	10	8,5
40°C/24°C	flooded bore hole	$\tau_{\text{Rk,ucr}}$	[N/mm ²]	7,5	8,5	8,5	8,5	8,5		not adr	nissible	
Temperature range II:	dry and wet concrete	$\tau_{\text{Rk,ucr}}$	[N/mm²]	7,5	9	9	9	9	9	8,0	7,0	6,0
80°C/50°C	flooded bore hole	$\tau_{\text{Rk,ucr}}$	[N/mm ²]	5,5	6,5	6,5	6,5	6,5		not adr	nissible	
Temperature range III:	dry and wet concrete	$\tau_{\text{Rk},\text{ucr}}$	[N/mm²]	5,5	6,5	6,5	6,5	6,5	6,5	6,0	5,0	4,5
120°C/72°C	flooded bore hole	$\tau_{\text{Rk,ucr}}$	[N/mm²]	4,0	5,0	5,0	5,0	5,0		not adr	nissible	
Characteristic bond resi	istance in cracked concre	te C20/25										
	day and wat apparets	$\tau_{\text{Rk,cr}}$	[N/mm ²]			5,5	5,5	5,5	5,5	5,5	6,5	6,5
Temperature range I:	dry and wet concrete	$\tau_{\rm Rk,seis}$	[N/mm ²]	1	a la a lla La	3,7	3,7	3,7	3,7	3,8	4,5	4,5
40°C/24°C	flander de serve la sta	$\tau_{\rm Rk,cr}$	[N/mm ²]	not admissible		5,5	5,5	5,5		not adr	nissible	
	flooded bore hole	$\tau_{\rm Rk,seis}$	[N/mm ²]	1		3,7	3,7	3,7		not adr	nissible	
	days and such a success.	$\tau_{\rm Rk,cr}$	[N/mm ²]			4,0	4,0	4,0	4,0	4,0	4,5	4,5
Temperature range II:	dry and wet concrete	$\tau_{\rm Rk,sels}$	[N/mm ²]			2,7	2,7	2,7	2,7	2,8	3,1	3,1
80°C/50°C		$\tau_{\rm Rk,cr}$	[N/mm ²]	not adr	nissible	4,0	4,0	4,0		not adr	nissible	L
1	flooded bore hole	$\tau_{\rm Rk,seis}$	[N/mm ²]	1		2,7	2,7	2,7		not adr	nissible	
		$\tau_{\rm Rk,cr}$	[N/mm ²]	not admissible		3,0	3,0	3,0	3,0	3,0	3,5	3,5
Temperature range III:	dry and wet concrete	$\tau_{\rm Rk,seis}$	[N/mm ²]			2,0	2,0	2,0	2,0	2,1	2,4	2,4
120°C/72°C		$\tau_{\rm Rk,cr}$	[N/mm ²]			3,0	3,0	3,0		not adr	nissible	
	flooded bore hole	$\tau_{\rm Rk,seis}$	[N/mm ²]	1		2,0	2,0	2,0	not admissible			
		C2	5/30	1,02								
		C3	0/37	1,04								
Increasing factors for co		C3	5/45	1,07								
(only static or quasi-stat Ψ_c	lic actions)	C4	0/50	1,08								
10		C4	5/55	1,09								
		C5	0/60					1,10				
Factor according to	Non-cracked concrete							10,1				
CEN/TS 1992-4-5 Section 6.2.2.3	Cracked concrete	k ₈	[-]	7,2								
Concrete cone failure		L	1									
Factor according to	Non-cracked concrete	k _{ucr}	[-]					10,1				
CEN/TS 1992-4-5 Section 6.2.3.1	Cracked concrete	k _{cr}	[-]					7,2				
Edge distance		C _{cr,N}	[mm]					1,5 h _{ef}				
Axial distance		S _{cr,N}	[mm]					3,0 h _{ef}				
Installation safety factor	(dry and wet concrete)	$\gamma_2 = \gamma_{inst}$		1,0					,2			
Installation safety factor		$\gamma_2 = \gamma_{inst}$.,-		1,4			-	not adr	nissible	

ABZ Injection system FIX-V or FIX-V Winter for concrete

Performances

Characteristic values of resistance for rebar under tension loads



Anchor size reinforcing bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32			
Steel failure without lever arm														
	$V_{Rk,s}$	[kN]				0,	50 • A _s •	f _{uk}						
Characteristic shear resistance	V ⁰ _{Rk,s,seis}	[kN]	not 0,35 • A _s • f _{uk}											
Ductility factor according to CEN/TS 1992-4-5 Section 6.3.2.1	k ₂	k ₂			0,8									
Steel failure with lever arm														
	M ⁰ _{Rk,s}	[Nm]	1.2 • W _{el} • f _{uk}											
Characteristic bending moment	M ⁰ _{Rk,s,seis}	[Nm]			No Pe	erformar	nce Dete	rmined	(NPD)					
Concrete pry-out failure														
Factor k₃ in equation (27) of CEN/TS 1992-4-5 Section 6.3.3 Factor k in equation (5.7) of Fechnical Report TR 029	k ₍₃₎						2,0							
nstallation safety factor	$\gamma_2 = \gamma_{inst}$						1,0							
Concrete edge failure														
Effective length of anchor	l _f	[mm]				$I_f = m$	nin(h _{ef} ; 8	d _{nom})						
Dutside diameter of anchor	d _{nom}	[mm]	8	10	12	14	16	20	25	28	32			
nstallation safety factor	$\gamma_2 = \gamma_{inst}$						1,0							

ABZ Injection system FIX-V or FIX-V Winter for concrete

Performances

Characteristic values of resistance for rebar under shear loads

Annex C 4

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Anchor size thread	led rod		M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Non-cracked conc	rete C20/25									
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]	0,021	0,023	0,026	0,031	0,036	0,041	0,045	0,049
40°C/24°C	$\delta_{N\infty}$ -factor	[mm/(N/mm ²)]	0,030	0,033	0,037	0,045	0,052	0,060	0,065	0,071
Temperature range II:	δ_{N0} -factor	[mm/(N/mm ²)]	0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119
80°C/50°C	$\delta_{N\infty}\text{-factor}$	[mm/(N/mm ²)]	0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172
Temperature range III:	δ_{N0} -factor	[mm/(N/mm ²)]	0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119
120°C/72°C	$\delta_{N\infty}\text{-factor}$	[mm/(N/mm ²)]	0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172
Cracked concrete	C20/25									
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]					0,0)70		
40°C/24°C	$\delta_{N\infty}\text{-}factor$	[mm/(N/mm ²)]					0,1	05		
Temperature range II:	δ_{N0} -factor	[mm/(N/mm ²)]					0,1	70		
80°C/50°C	$\delta_{N\infty}\text{-}factor$	[mm/(N/mm ²)]					0,2	245		
Temperature range III:	δ_{N0} -factor	[mm/(N/mm ²)]					0,1	70		
120°C/72°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm ²)]					0,2	245		

¹⁾ Calculation of the displacement $\delta_{N0} = \delta_{N0}$ -factor $\cdot \tau$;

 τ : action bond stress for tension

 $\delta_{N_{\infty}} = \delta_{N_{\infty}}$ -factor $\cdot \tau$;

 Table C6:
 Displacements under shear load¹⁾ (threaded rod)

Anchor size thre	eaded rod		M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
For non-cracked	l concrete C2	0/25								
All temperature	δ_{V0} -factor	[mm/(kN)]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V\infty}\text{-}factor$	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05
For cracked con	crete C20/25									
All temperature	δ_{V0} -factor	[mm/(kN)]			0,11	0,10	0,09	0,08	0,08	0,07
ranges	$\delta_{V_{\infty}}$ -factor	[mm/(kN)]		-	0,17	0,15	0,14	0,13	0,12	0,10
AB7 Injection	system FIX.	V or FIX-V Winter	for conc	roto						
Performances Displacements (th	-							An	nex C	5



Anchor size reinfo	orcing bar		Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Non-cracked con	crete C20/2	25									
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,021	0,023	0,026	0,028	0,031	0,036	0,043	0,047	0,052
40°C/24°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,030	0,033	0,037	0,041	0,045	0,052	0,061	0,071	0,07
Temperature range II:	δ_{N0} -factor	[mm/(N/mm ²)]	0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,12
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,18
Temperature range III:	δ_{N0} -factor	[mm/(N/mm ²)]	0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,12
120°C/72°C $\delta_{N_{\infty}}$ -factor [mm/(N/mm ²)]			0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,18
Cracked concrete	C20/25										
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]						0,070			
40°C/24°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]						0,105			
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]						0,170			
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm ²)]						0,245			
Temperature range III:	δ_{N0} -factor	[mm/(N/mm ²)]	-					0,170			
120°C/72°Č	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm ²)]						0,245			
¹⁾ Calculation of th $\delta_{N0} = \delta_{N0}$ -factor $\delta_{N\infty} = \delta_{N\infty}$ -factor Table C8: D	·τ; ·τ;	t: action bond τ: action bond									
$\begin{split} \delta_{N0} &= \delta_{N0} \text{-factor} \\ \delta_{N\infty} &= \delta_{N\infty} \text{-factor} \end{split}$ $\textbf{Table C8: } \textbf{D}$	τ; τ; isplacen	τ: action bond				Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø3
$\begin{split} \delta_{N0} &= \delta_{N0} \text{-factor} \\ \delta_{N\infty} &= \delta_{N\infty} \text{-factor} \end{split}$ Table C8: D Anchor size reinfor	τ; τ; isplacen prcing bar	τ: action bond	hear lo	ad ¹⁾ (r	ebar)	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø3
$\begin{array}{l} \delta_{N0} = \delta_{N0} \text{-factor} \\ \delta_{N\infty} = \delta_{N\infty} \text{-factor} \end{array}$	τ; τ; isplacen prcing bar	τ: action bond	hear lo	ad ¹⁾ (r	ebar)	Ø 14 0,04	Ø 16 0,04	Ø 20 0,04	Ø 25 0,03	Ø 28 0,03	
$\begin{split} \delta_{N0} &= \delta_{N0} \text{-} \text{factor} \\ \delta_{N\infty} &= \delta_{N\infty} \text{-} \text{factor} \end{split}$ Table C8: D Anchor size reinfor Non-cracked concord All temperature	τ; τ; orcing bar crete C20/2	τ: action bond	hear lo Ø 8	øad ¹⁾ (r Ø10	ebar) Ø 12						0,03
$\begin{split} \delta_{N0} &= \delta_{N0} \text{-} \text{factor} \\ \delta_{N\infty} &= \delta_{N\infty} \text{-} \text{factor} \end{split}$ Table C8: D Anchor size reinfor Non-cracked cond All temperature ranges	τ; τ; isplacent prcing bar crete C20/2 $δ_{V0}$ -factor $δ_{V\infty}$ -factor	τ: action bond	hear lo Ø 8 0,06	ø ad¹⁾ (r ∕ Ø 10 0,05	ebar) Ø 12 0,05	0,04	0,04	0,04	0,03	0,03	0,0
$\begin{split} \delta_{N0} &= \delta_{N0} \text{-factor} \\ \delta_{N\infty} &= \delta_{N\infty} \text{-factor} \end{split}$ Table C8: D Anchor size reinfo	τ; τ; isplacent prcing bar crete C20/2 $δ_{V0}$ -factor $δ_{V\infty}$ -factor	τ: action bond	hear lo Ø 8 0,06	ø ad¹⁾ (r ∕ Ø 10 0,05	ebar) Ø 12 0,05	0,04	0,04	0,04	0,03	0,03	Ø 3 0,03 0,04
$\begin{split} \delta_{N0} &= \delta_{N0}\text{-}\text{factor} \\ \delta_{N\infty} &= \delta_{N\infty}\text{-}\text{factor} \end{split}$ Table C8: D Anchor size reinfor Anchor size reinfor Non-cracked conc All temperature ranges Cracked concrete All temperature ranges ¹⁾ Calculation of th $\delta_{V0} = \delta_{V0}\text{-}\text{factor}$	τ; τ; isplacent prcing bar crete C20/2 $δ_{V0}$ -factor $δ_{V\infty}$ -factor C20/25 $δ_{V0}$ -factor $δ_{V\infty}$ -factor $δ_{V\infty}$ -factor he displacent V;	<pre>r: action bond nent under s 25 [mm/(kN)] [mm/(kN)] [mm/(kN)]</pre>	hear lo Ø 8 0,06 0,09	ø ad¹⁾ (r ∕ Ø 10 0,05	ebar) Ø 12 0,05 0,08	0,04	0,04	0,04	0,03	0,03	0,0
$\delta_{N0} = \delta_{N0}$ -factor $\delta_{N\infty} = \delta_{N\infty}$ -factor Table C8: D Anchor size reinfor Non-cracked conc All temperature ranges Cracked concrete All temperature ranges ¹⁾ Calculation of th	τ; τ; isplacent prcing bar crete C20/2 $δ_{V0}$ -factor $δ_{V\infty}$ -factor C20/25 $δ_{V0}$ -factor $δ_{V\infty}$ -factor $δ_{V\infty}$ -factor he displacent V;	<pre>r: action bond nent under s 25 [mm/(kN)] [mm/(kN)] [mm/(kN)] [mm/(kN)] ment</pre>	hear lo Ø 8 0,06 0,09	ø ad¹⁾ (r ∕ Ø 10 0,05	ebar) Ø 12 0,05 0,08 0,11	0,04 0,06 0,11	0,04 0,06 0,10	0,04 0,05 0,09	0,03 0,05 0,08	0,03 0,04 0,07	0,0

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