



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-07/0299 of 9 November 2017

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the **European Technical Assessment:**

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

Deutsches Institut für Bautechnik

Injection System VME

Post-installed rebar connection with Injection System VME

Metall-Kunststoff-Technik GmbH & Co. KG Auf dem Immel 2 67685 Weilerbach **DEUTSCHLAND**

Werk 1, D Werk 2, D

19 pages including 3 annexes which form an integral part of this assessment

EAD 330087-00-0601



European Technical Assessment ETA-07/0299

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Specific Part

1 Technical description of the product

The subject of this approval is the post-installed rebar connection, by anchoring or overlap connection joint, of reinforcing bars (rebars) in existing structures made of normal weight concrete, using the "Injection system VME" in accordance with the regulations for reinforced concrete construction.

Reinforcing bars made of steel with a diameter ϕ from 8 to 28 mm or the tension anchor ZA sizes M12, M16, M20 and M24 according to Annex A and injection adhesive VME are used for rebar connections. The rebar is placed into a drilled hole filled with injection mortar and is anchored via the bond between rebar, injection mortar and concrete.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the rebar connections of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Amplification factor α_{lb} , Bond resistance f_{bd}	See Annex C 1

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance		
Reaction to fire	Rebar connnections satisfy requirements for Class A1		
Resistance to fire	See Annex C 2 – C 3		

Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with European Assessment Document EAD No. 330087-00-0601, the applicable European legal act is: [96/582/EC].

The system(s) to be applied is (are): 1

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5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

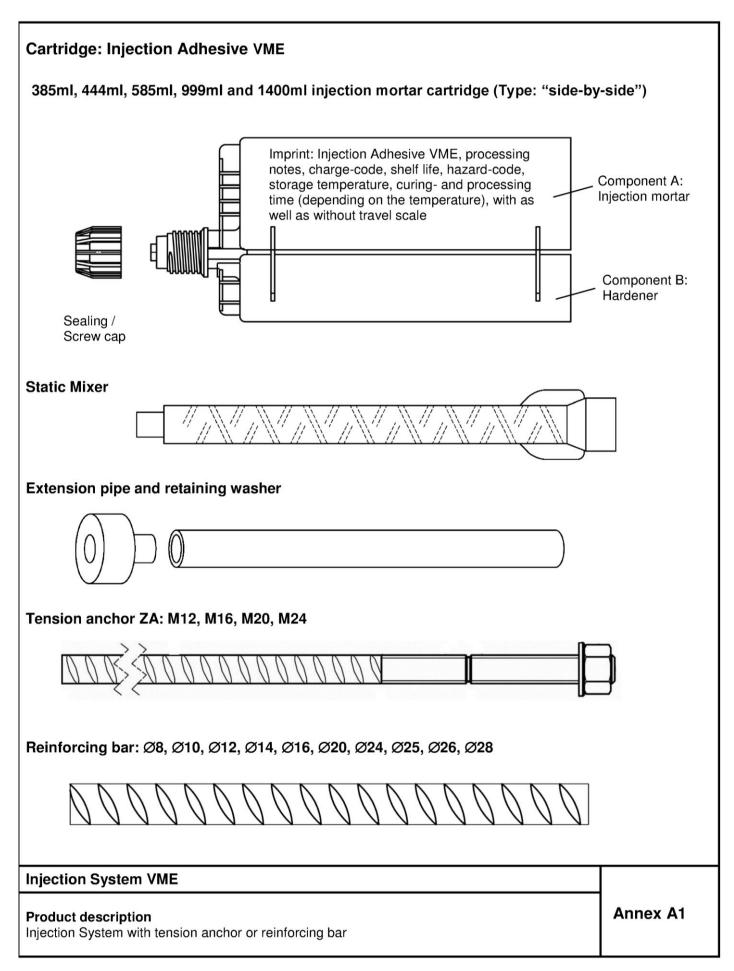
Issued in Berlin on 9 November 2017 by Deutsches Institut für Bautechnik

BD Dipl.-Ing. Andreas Kummerow Head of Department

*beglaubigt:*Lange

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Application examples (reinforcing bar)

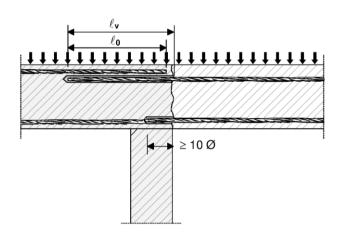


Figure A1: Overlap joint in slabs and beams

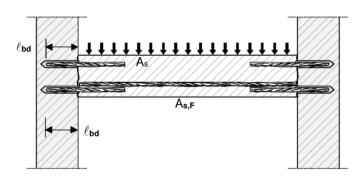


Figure A3: End anchoring of slabs or beams, designed as simply supported

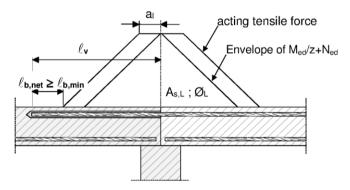


Figure A5: Anchoring of reinforcement to cover line of acting tensile force

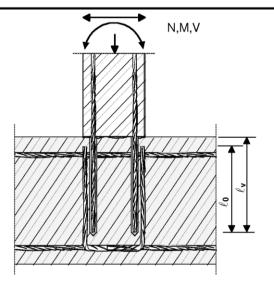


Figure A2: Overlap joint in a foundation of a column or wall where the rebars are stressed in tension

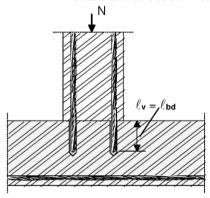


Figure A4: Rebar connection of components stressed primarily in compression. The rebars are stressed in compression.

Note to Figure A1 – A5:

No transverse reinforcement acc. to EN 1992-1-1 is pictured.

The shear transfer between old and new concrete shall be designed according to EN 1992-1-1.

Notations and definitions of anchorages and overlap joints see Annex B2

Injection System VME

Product description

Applications examples of post-installed rebars

Annex A2



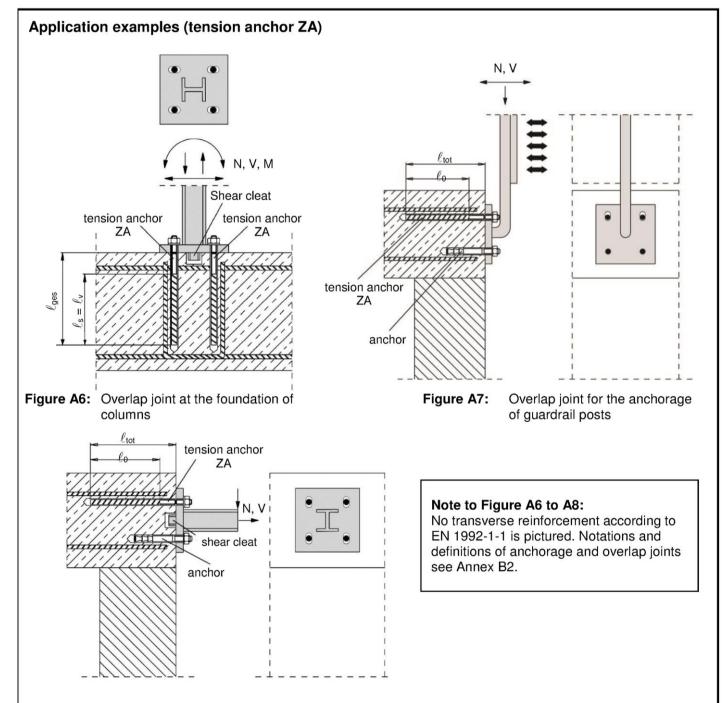


Figure A8: Overlap joint for the anchorage of cantilevers

The tension anchor ZA can only transfer forces towards the bar axis.

Tension forces must be transferred by an overlap joint into the present reinforcement of the member. The transmission of shear forces must be ensured by additional measures, e.g. by shear cleats or anchors with an European Technical Assessment (ETA). In the anchor plate the holes for the tension anchors must be designed as slotted holes towards the shear force.

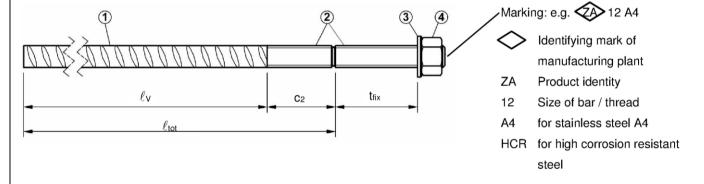
Injection System VME	
Product description Applications examples of tension anchor ZA	Annex A3

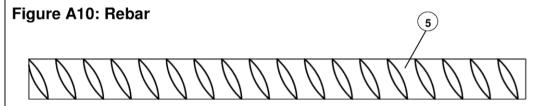


Table A1: Material

Part	Description		Material										
Tens	sion anchor ZA		ZA	vz		ZA A4			ZA HCR				
		M12	M16	M20	M24	M12	M16	M20	M24	M12	M16	M20	M24
1	Rebar	1	Class B according to NDP or NCL of EN 1992-1-1 $f_{uk} = f_{tk} = k \cdot f_{yk}$										
2	Threaded rod	Steel, zinc plated acc. to EN 10087:1998 or EN 10263:2001				Stainless steel, 1.4362, 1.4401, 1.4404, 1.4571, EN 10088:2014			High corrosion resistant steel, 1.4529, 1.4565, EN 10088:2014				
	f _{yk} [N/mm²]	640					640 560		640		560		
3	Washer	Steel, zinc plated				Stainless steel			High corrosion resistant steel				
4	Hexagon nut	Steel, zinc plated acc. to EN 10087:1998 or EN 10263:2001			1.440		eel, 1.4 -04, 1.4 014		steel,	corrosi 1.4529 0088:20	9, 1.456		
Rebar													
5	Rebar EN 1992-1-:2004+ AC:2010, Annex C	Bars and de-coiled rods class B or C f_{yk} and k according to NDP or NCL of EN 1992-1-1 $f_{uk} = f_{tk} = k \cdot f_{yk}$											

Figure A9: Tension anchor ZA, marking





Injection System VME	
Product description Materials, Marking	Annex A4



Specifications of intended use

Anchorages subject to:

- Static or quasi static action
- Fire exposure

Base material:

- Reinforced or unreinforced normal weight concrete acc. to EN 206-1:2000.
- Strength classes C12/15 to C50/60 acc. to EN 206-1:2000.
- Maximum chloride concrete of 0,40 % (CL 0,40) related to the cement content acc. to EN 206-1:2000.
- Non-carbonated concrete.

Note: In case of a carbonated surface of the existing concrete structure the carbonated layer shall be removed in the area of the post-installed rebar connection with a diameter of \emptyset + 60 mm prior to the installation of the new rebar.

The depth of concrete to be removed shall correspond to at least the minimum concrete cover in accordance with EN 1992-1-1:2004+AC:2010.

The foregoing may be neglected if building components are new and not carbonated and if building components are in dry conditions.

Temperature range:

· - 40 °C to +80 °C (max. long term temperature +80 °C and max. short term temperature +50 °C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc plated steel, stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure including industrial and marine environment or exposure to
 permanently damp internal condition, if no particular aggressive conditions exist (stainless steel or high corrosion
 resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions (high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where deicing materials are used).

Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position
 of the anchor is indicated on the design drawings (e.g. position of the anchor relative to reinforcement or to
 supports, etc.).
- · Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Anchorages are designed in accordance with EN 1992-1-1 and Annex B2.
- The actual position of the reinforcement in the existing structure shall be determined on the basis of the construction documentation and taken into account when designing.

Installation:

- Dry or wet concrete
- The rebar connection must not be placed in water-filled boreholes.
- · Overhead installation admissible
- Check the position of the existing rebars (if the position of existing rebars is not known, it shall be determined using a rebar detector suitable for this purpose as well as on the basis of the construction documentation and then marked on the building component for the overlap joint)
- · Hole drilling by hammer drill, vacuum drill, compressed air drill or diamond drill mode.
- While curing of the injection mortar, the temperature of the member shall not fall below +5°C and must not exceed +40°C.
- The installation of post-installed rebar or tension anchor ZA shall be done only by suitable trained installer and
 under supervision on site; the conditions under which an installer may be considered as suitable trained and the
 conditions for supervision on site are up to the member states in which the installation is done.

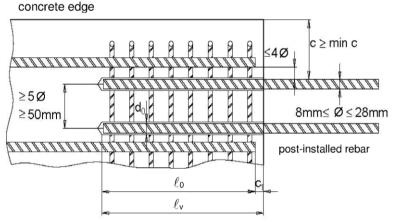
Injection System VME	
Intended use Specifications of intended use	Annex B1



General construction rules for post-installed reinforcing bars and tension anchor ZA

- Bond strength f_{bd} acc. to EN 1992-1-1 may be applied.
- The shear transfer between old and new concrete shall be designed acc. to EN 1992-1-1.
- If the clear distance of overlapping bars is greater than 4 Ø, the lap length must be increased by a length equal to the clear space where it exceeds 4 Ø.
- The minimum concrete cover acc. to EN 1992-1-1 shall be kept.

Figure B1: Post-installed rebars



- concrete cover of post-installed reinforcement
 - concrete cover at from
- c₁ concrete cover at front end of cast
 - in-place rebar
- min c minimum concrete cover according
 - Table B1

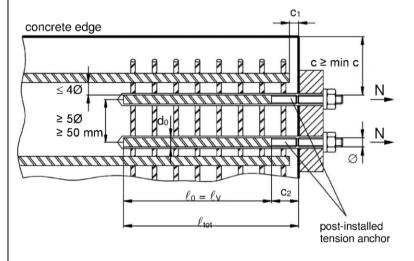
С

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С

- Ø diameter of post-installed rebar
- ℓ_0 lap length acc. to EN 1992-1-1
- ℓ_v embedment depth $\ell_v \ge \ell_0 + c_1$
 - nominal drill bit diameter according to
 - Annex B4, Table B4

Figure B2: Tension anchor ZA



- concrete cover of post-installed
- tension anchor ZA
- c₁ concrete cover at front side of
 - cast-in-place rebar
- min c minimum concrete cover
 - acc. to Table B1
 - c₂ length of bonded thread
 - $(c_2 > c_1; see Table B1)$
 - ∅ diameter of tension anchor
- ℓ_0 lap length acc. to EN 1992-1-1
- ℓ_{tot} embedment depth $\ell_{tot} \ge \ell_0 + c_2$
- d₀ nominal drill bit diameter according
 - Annex B4, Table B4

Injection System VME

Intended use

General construction rules

Annex B2



Table B1: Minimum concrete cover min c¹⁾ of post-installed rebars and tension anchor ZA depending on drilling method and drilling tolerances

Drilling method	Rebar diameter	min c (without drilling aid device)	min c (<u>with</u> drilling aid device)
Hammer drilling,	< 25 mm	30 mm + 0,06 ℓ _v ≥ 2 Ø	30 mm + 0,02 ℓ _v ≥ 2 Ø
vacuum drilling, diamond core drilling	≥ 25 mm	40 mm + 0,06 ℓ _v ≥ 2 Ø	40 mm + 0,02 ℓ _v ≥ 2 Ø
Compressed air	< 25 mm	50 mm + 0,08 ℓ _v	50 mm + 0,02 ℓ _v
drilling	≥ 25 mm	60 mm + 0,08 ℓ _v	60 mm + 0,02 ℓ _v

¹⁾ Minimum concrete cover acc. to EN 1992-1-1 must be observed.

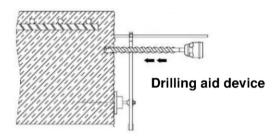


Table B2: Dimensions and installation parameters of tension anchor ZA

Anchor size				M12	M16	M20	M24
Rebar diameter	Ø	[mm]	12	16	20	25	
Cross-section area (th	readed part)	As	[mm ²]	84	157	245	353
Width across nut flats		SW	[mm]	19	24	30	36
Effective embedment	depth	ℓ_{V}	[mm]	$\ell_{\rm v} = \ell_{\rm tot}$ - C ₂			
Length of bonded steel, zinc plated		C 2	[mm]	≥ 20	≥ 20	≥ 20	≥ 20
thread A4 / HCR			' '	≥ 100	≥ 100	≥ 100	≥ 100
Maximum installation torque		T _{inst}	[Nm]	50	100	150	150

Table B3: Maximum embedment depth

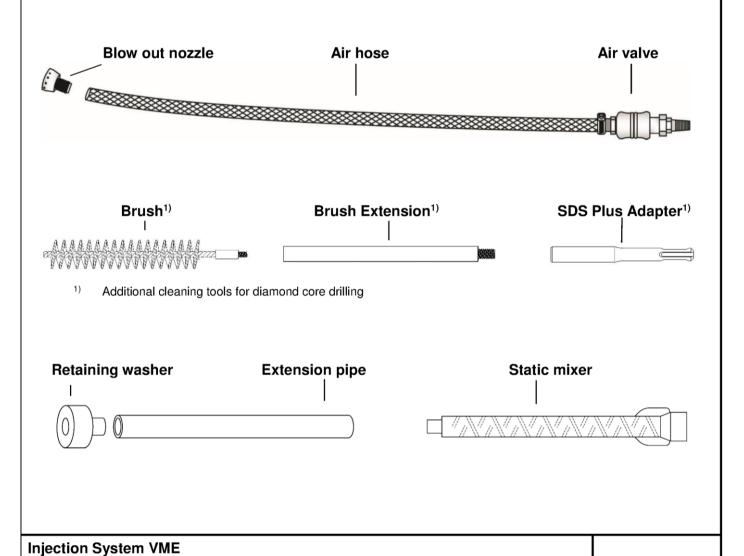
Rebar / Tension Anchor ZA	Motar temperature	Max. embedment depth $\ell_{ m v,max}$
Ø 8 to 12 mm	+5°C to +19°C	130 cm
	≥20°C	200 cm
Ø 14 to 28 mm	+5°C to +19°C	200 cm
Ø 14 to 26 mm	≥20° C	280 cm

Injection System VME	
Intended use Minimum concrete cover, installation parameters, maximum embedment depth	Annex B3



Table B4: Cleaning and installation tools

Rebar Ø	Tension anchor ZA	Drill bit diameter d₀	Blow out nozzle Ø	Brush Ø (diamond drilling)	Extension pipe (outer Ø)	Retaining washer Ø
[mm]	[-]	[mm]	[mm]	[mm]	[mm]	[mm]
8	-	12	10	13,0		10
10	-	14	10	15,5	10	13
12	M12	16	14	17,5		15
14	-	18	14	19,5		17
16	M16	20	17	22		19
20	M20	25	17	27		24
24	-	30	27	32	16	29
25	M24	32	27	34		31
26	-	32	27	34		31
28	-	35	27	37		34



Intended use

Cleaning and Installation tools

Annex B4



Table B5: Working time and curing time

Temperature in the bore hole	Working time	Minimum curing time
+5°C to +9°C	60 min	72 h
+10°C to +19°C	45 min	36 h
+20°C to +29°C	30 min	10 h
+30°C to +39°C	20 min	6 h
+40°C	12 min	4 h

Note: A mortar temperature > +20 ° C reduces the press out forces and accelerates the injection of the mortar. When using the adhesive in wet concrete the curing times have to be doubled.

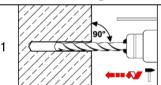
Installation instructions

Preperation:

- Remove carbonated concrete and roughen concrete surface prior to installation of post-installed rebars.
- Determine position and diameter of existing reinforcement. Only the admissible force of the existing reinforcement can be transmitted into the post-installed rebar connection
- c. The drilling aid device must be adjusted towards the existing reinforcement
- The concreting joints must be roughened to at least such an extent that the aggregates protrude

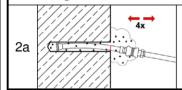
Making and cleaning of hammer drilled, vacuum drilled and compressed air drilled holes

Bore hole drilling



Select drill bit diameter according to Table B4. Build drill hole by hammer drilling, vacuum drilling or compressed air drilling. In case of aborted holes, the bore holes must be filled with mortar.

Cleaning



Assemble provided blow out nozzles, air hoses and air valve and connect to compressed air (≥ 6 bar). Place marking on the air hose at a distance from the end equal to the drill hole depth. Open compressed air valve and blow out **4x** from top to bottom and reverse. The air hose marking has to get below the concrete surface in order to ensure cleaning to the very bottom of the hole.

Injection System VME

Intended use

Working and curing time, Installation instructions – Preperation; Making and cleaning of hammer drilled, vacuum drilled and compressed air drilled holes **Annex B5**

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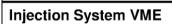


Installation instruction (continuation) Marking and cleaning of diamond core drilled holes Bore hole drilling Drill hole by using a diamond drill with diamond drill bit. 1a Remove drill core at least up to the nominal hole depth and check drill hole depth. 1b Cleaning Flush drill hole with water, staring from the bottom, until clear water gets out of the 2a drill hole. Choose brush suitable to the hole. Brush bore hole 2x using a cordless screwdriver 11111111111111 2b or a drilling machine. Use brush extension for deep holes. Flush drill hole again with water, staring from the bottom, until clear water gets out of 2c the drill hole. Assemble provided blow out nozzles, air hoses and air valve and connect to 2x compressed air (≥ 6 bar). Place marking on the air hose at a distance from the end equal to the drill hole depth. Open compressed air valve and blow out 2x from top to 2d bottom and reverse. The air hose marking has to get below the concrete surface in order to ensure cleaning to the very bottom of the hole. 2e Brush the hole again 2x (according to step 2b). 2f Blow out again 2x (according to step 2d).

Injection System VME Intended use Installation instructions Making and cleaning of diamond core drilled holes Annex B6

Installation Instruction (continuation)

Injection into bore hole За Mark the position of the embedment depth ℓ_{v} on the rebar. Check bore hole depth by inserting rebar or tension anchor up to the marking in the 3b empty hole. Prepare cartridge with static mixer, extension pipe and retaining washer. Extension 4 pipe must correspond the hole depth. Attach marking length ℓ_m on extension pipe for injection of adhesive: a) Estimation: $\ell_{\rm m} = 1/3 * h_1$ [mm] b) Precise formula for h_1 optimum volume (compressed air drilling): 5 $l_m = h_1 * (1.2 * \frac{\phi^2}{d_0^2} - 0.2)$ [mm] Length from the end of the retailing washer to the mark on the mixer extension drill hole depth = embedment depth (ℓ_{V} resp. ℓ_{tot}) h₁ Ø rebar diameter do nominal drill bit diameter When using hammer drill or diamond core, the length ℓ_m may be multiplied by 1,10. Prior to applying, discard at least a 10 cm line of mortar (forerun) until the mortar 6 shows a consistent colour. Never use this mortar! min. 10cm Fill the hole starting from the bottom with injection mortar VME without air bubbles. 7 Inject until the marking ℓ_m on the extension pipe can be seen.



Intended use

Installation instructions Injection into bore hole Annex B7

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Installation Instruction (continuation)

Push the rebar or tension anchor into the hole while turning slightly until the embedment depth is reached. The bar should be free of dirt, grease, oil or other foreign material. Excessive adhesive must exceed out of the drill hole. Otherwise pull out rebar, drill out hole after curing of the adhesive and start again from No. 2. Keep curing time according to Annex B5, Table B5. Do not move or load the rod during curing time. After curing, the rod can be loaded.

Injection System VME		
Intended use Installation instructions	Annex E	38
Installation of rebar or tension anchor		



Minimum anchorage length and minimum lap length

The minimum anchorage length $\ell_{b,min}$ and the minimum lap length $\ell_{0,min}$ according to EN 1992-1-1 ($\ell_{b,min}$ acc. to Eq. 8.6 and Eq. 8.7 and $\ell_{0,min}$ acc. to Eq. 8.11) shall be multiplied by the amplification factor α_{lb} acc. to Table C1.

Table C1: Amplification factor α_{lb}

Concrete strength class	Drilling method	Amplification factor α _{lb}		
C12/15 to C50/60	Hammer drilling, vacuum drilling or compressed air drilling	1,0		
	Diamond coring	1,5		

Table C2: Design values of the ultimate bond resistance f_{bd}1)

Rebar Tension Anchor ZA		Concrete strength class								
		C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
Design value of bond resistance f _{bd}										
Hammer drilling, vacuum drilling or compressed air drilling	[N/mm²]	1,6	2,0	2,3	2,7	3,0	3,4	3,7	4,0	4,3
Diamond coring	[N/mm ²]	1,6	2,0	2,3	2,7	3,0	3,0	3,4	3,7	3,7

With η_1 =1,0 acc. to EN 1992-1-1 for good bond conditions (for all other bond conditions multiply the values by 0,7)

Injection System VME	
Performances Amplification factor Design values of ultimate bond resistance fbd	Annex C1



Design value of ultimate bond stress $f_{bd,fi}$ under fire exposure for concrete classes C12/15 to C50/60 (all drilling methods):

The design value of ultimate bond stress f_{bd,fi} under fire exposure will be calculated by the following equation:

 $f_{bd,fi} = k_{b,fi}(\theta) \cdot f_{bd} \cdot \gamma_c / \gamma_{M,fi}$

with: $\theta \le 270^{\circ}\text{C}$ $k_{\text{b,fi}}(\theta) = 9221.2 * \theta^{-1.747} / (f_{\text{bd}} * 4.3) \le 1.0$

 $\theta > 270^{\circ}\text{C}$ $k_{b,fi}(\theta) = 0$

f_{bd,fi} design value of ultimate bond stress in case of fire in N/mm²

 θ Temperature in °C in the mortar layer $k_{b,fi}(\theta)$ Reduction factor under fire exposure

f_{bd} Design value of the ultimate bond stress in N/mm² in cold condition according to

Table C2 considering concrete class, rebar diameter, drilling method and the bond

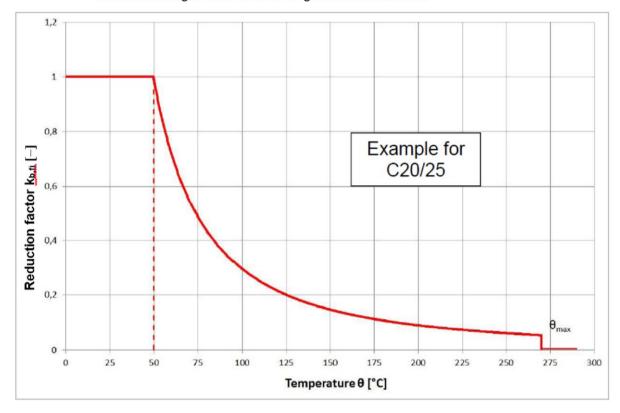
conditions according to EN 1992-1-1.

 γ_c partial factor acc. to EN 1992-1-1 $\gamma_{M,fi}$ partial factor acc. to EN 1992-1-2

For evidence under fire exposure the anchorage length shall be calculated according to EN 1992-1-1:2004+AC:2010 Equation 8.3 using the temperature-dependent ultimate bond stress fbd.fi.

Picture C1: Example graph of reduction factor $k_{b,fi}(\theta)$

Concrete strength class C20/25 for good bond conditions



Injection System VME

Performances

Design value of ultimate bond stress fbd,fi under fire exposure

Annex C2



Table C3: Characteristic tension strength in case of fire for tension anchor ZA, concrete strength class C12/15 to C50/60, acc. to Technical Report TR 020

Tension anchor ZA				M12	M16	M20	M24		
Steel, zinc plated	Steel, zinc plated								
Characteristic tension strength	R30	- σ _{Rk,s,fi}	[N/mm²]	20					
	R60			15					
	R90			13					
	R120				1	0			
Stainless steel A4, HCR									
Characteristic tension strength	R30		[N//n2]		3	0			
	R60	_			2	5			
	R90	σ _{Rk,s,fi}	[N/mm²]		2	0			
	R120	-			1	6			

Design value of the tension strength $\sigma_{\text{Rd,s,fi}}$ under fire exposure for tension anchor ZA

The design value of the steel strength $\sigma_{Rd,s,fi}$ under fire exposure will be calculated by the following equation:

 $\sigma_{Rd,s,fi} = \sigma_{Rk,s,fi} / \gamma_{M,fi}$

with:

 $\sigma_{\text{Rk,s,fi}}$ characteristic steel strength acc. to Table C3

 $\gamma_{M,fi}$ partial factor under fire exposure acc. to EN 1992-1-2

Injection System VME	
Performances Steel strength for tension anchor ZA under fire exposure	Annex C3