



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

of 12 July 2017

ETA-15/0476

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of Deutsches Institut für Bautechnik

TUF-S

Fastener for the rear fixing of facade panels made of highpressure decorative laminates (HPL) according to EN 438-7:2005

SFS intec AG Rosenbergsaustraße 10 9435 HEERBRUGG SCHWEIZ

Werke der SFS intec AG

16 pages including 3 annexes which form an integral part of this assessment

European Assessment Document (EAD) 330030-00-0601



European Technical Assessment ETA-15/0476

Page 2 of 16 | 12 July 2017

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Page 3 of 16 | 12 July 2017

European Technical Assessment ETA-15/0476 English translation prepared by DIBt

Specific Part

1 Technical description of the product

The TUF-S-6xL is special anchor made of stainless steel for fixing HPL-facade panels according to EN 438-7:2015 to metal substructures. The anchor consits of a mandrel made of carbon steel zinced and a stainless steel sleeve. The anchor is put in a drill hole and placed by pulling out the mandrel. The pull out of the mandrel widens the body of the sleeve and punches the thread of the sleeve into the façade panel.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchors of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance for tension and shear loads	See Annex C 1
Anchor distances	See Annex C 1

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance				
Reaction to fire	Class A1				
Resistance to fire	No performance assessed				

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with EAD No. 330030-00-0601 the applicable European legal act is: [97/161/EG]. The system to be applied is: 2+



European Technical Assessment ETA-15/0476

Page 4 of 16 | 12 July 2017

English translation prepared by DIBt

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

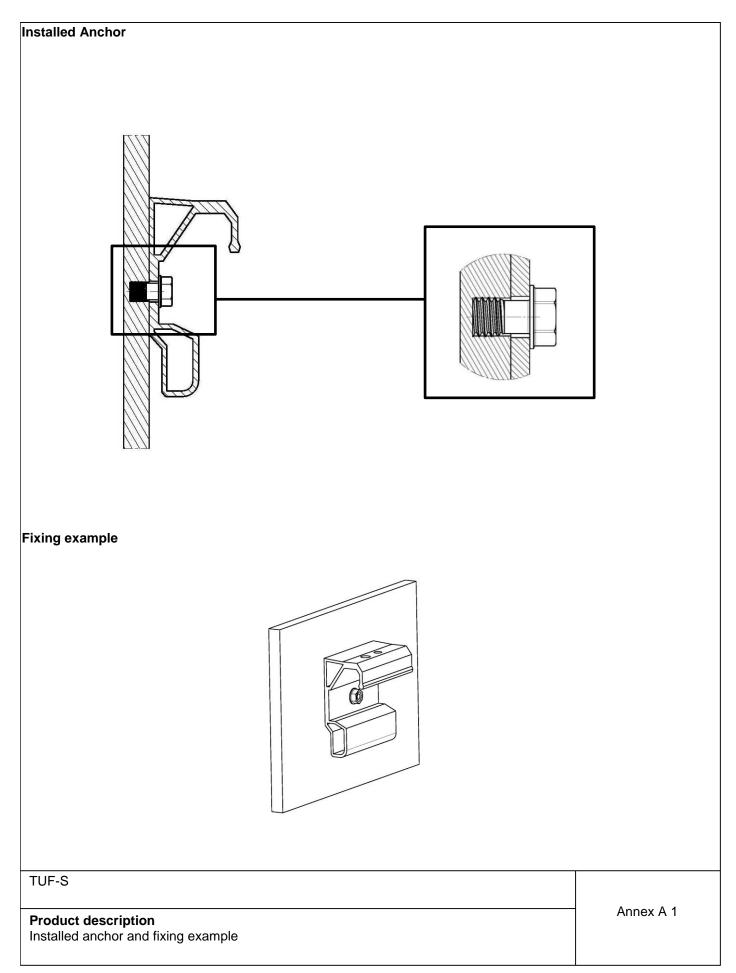
Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

Issued in Berlin on 12 July 2017 by Deutsches Institut für Bautechnik

BD Dipl.-Ing. Andreas Kummerow Head of Department *beglaubigt:* Aksünger

Page 5 of European Technical Assessment ETA-15/0476 of 12 July 2017

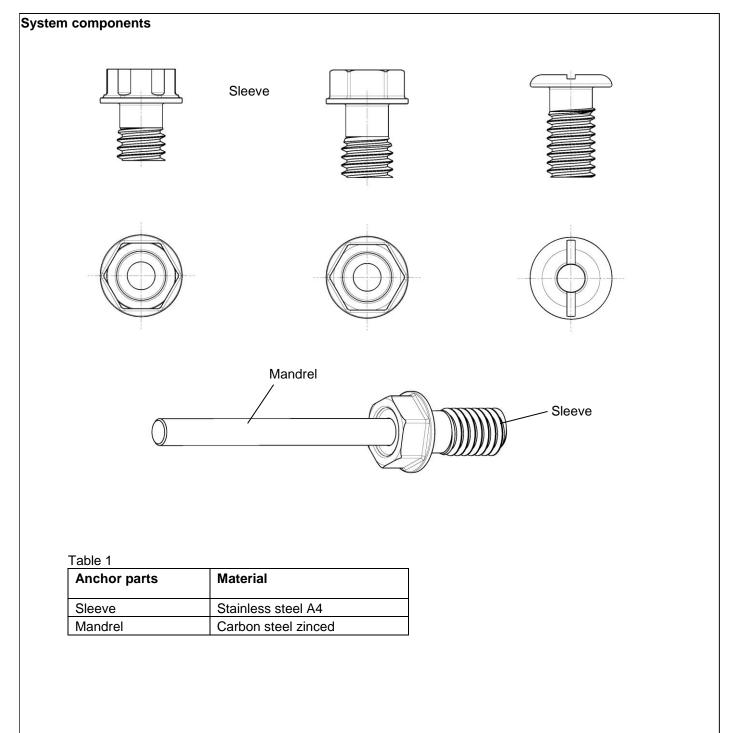




Page 6 of European Technical Assessment ETA-15/0476 of 12 July 2017

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Product description System components Annex A 2

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Specifications of intended use

Anchorages subject to

Static and quasi-static loads

Base material

• The façade panel made of HPL shall correspond to Annex B 4

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions.
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist.

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

• The design of the façade panels and their fixing is carried out according to the conditions given in Annex B 2 and B 3.

Installation

- Each façade panel shall be fixed technically strain-free with at least four anchors in a rectangular arrangement.
- The substructure is constructed such that the façade panels are fixed technically strain free via skids (loose bearings) and one fixed point (fixed bearing).
- The thickness of the fixing member (clamp or panel load-bearing profile) shall be at least 2,0 mm and shall be at least made of aluminum with $R_m \ge 215 \text{ N/mm}^2$.
- The drillings are done at the factory or on site. The drillings are executed with special drill bits made available by SFS intec AG. The drillings are executed by skilled personnel.
- The façade panel is pre-drilled with diameter Ø 5,9 mm to 6,0 mm.
- The drilling is always in a 90°- angle to the panel's surface.
- The minimum edge distance of the drilling is 40,0 mm.
- The clamps are predrilled with diameter Ø 6,5 mm to 7,0 mm.
- The geometry of the drill hole shall be checked minimum on 1% of all drillings.
- The façade panels, their fixings as well as the substructure including its connection to wall brackets and their connection to the construction works are designed for the respective case of application under the responsibility of an engineer skilled in the field of façade construction.
- The panels are installed by skilled specialists and the laying instructions of the manufacturer shall be paid attention to.
- Overhead mounting is not possible

Annex B 1

Intended use Specifications

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Design method

Loads

The design loads shall be calculated on basis of EN 1990. The combination of loads shall be equal to EN 1990. The loads shall be specified according to EN 1991-1-1 to EN 1991-1-7. Corresponding national regulations shall be taken into consideration. The unfavorable combination is decisive. Where necessary for the design of the anchor and the façade panel several combinations shall be analyzed separately.

The typical fundamental combination for façade panels considers loads from dead load $F_{Ek,G}$ (permanent loads) and wind $F_{Ek,w}$ (leading variable load)

According to EN 1990 the following fundamental combination depending on the load direction results for a vertical façade panel:

Fundamental combination for loads parallel to the panel: Fundamental combination for loads perpendicular to the panel:

$F_{EdII} = F_{Ek,G} \cdot \gamma_{G}$
$\textbf{F}_{\text{Ed}^{\perp}} = \textbf{F}_{\text{Ek}, w} \cdot \gamma_{Q}$
mit $\gamma_{\rm G} = 1,35; \gamma_{\rm Q} = 1,50$

Resistance:

$$\begin{split} \mathsf{N}_{\mathsf{Rd}} &= \frac{\mathsf{N}_{\mathsf{Rk}}}{\mathsf{Y}_{\mathsf{M}}} \bullet \alpha_{\mathsf{F0}} \bullet \alpha_{\mathsf{bend}} \bullet \alpha_{\mathsf{wet}} \\ \mathsf{V}_{\mathsf{Rd}} &= \frac{\mathsf{V}_{\mathsf{Rk}}}{\mathsf{Y}_{\mathsf{M}}} \bullet \alpha_{\mathsf{F0}} \bullet \alpha_{\mathsf{wet}} \end{split}$$

 $\sigma_{Rd} = \frac{\sigma_{Rk}}{\sigma_{Rd}}$

γм

with:

 N_{Rk} = characteristic tension resistance according to Annex C 1, Table 5 to 7

 V_{Rk} = characteristic shear resistance according to Annex C 1, Table 5 to 7

 σ_{Rk} = characteristing bending stress according to EN 438:2016

 α_{F0} = If the façade panels do not meet the minimum requirements according to Annex B 4, Table 2, the characteristic values of load bearing capacity have to be multiplied additionally by α_{F0} :

$$\alpha_{F0} = min\left\{\frac{\sigma_{f,L,min}}{130 \ N/mm^2}; \frac{E_{L,min}}{14000 \ N/mm^2}; \frac{\sigma_{f,T,min}}{100 \ N/mm^2}; \frac{E_{T,min}}{10000 \ N/mm^2}; 1\right\}$$

 α_{bend} = reduction factor of bearing of facade panel

The bending angle of the façade panel $\beta = \arctan\left(\frac{u_{max}}{L/2}\right)$ $u_{max} \int \frac{1}{\sqrt{L/2}} \int \frac{1$

Annex B 2

Intended use Design method

Page 9 of European Technical Assessment ETA-15/0476 of 12 July 2017

 $\frac{N_{Ed}}{N_{Rd}} \le 1$

 $\frac{V_{Ed}}{V_{Rd}} \leq 1$

 $\frac{\sigma_{\rm Ed}}{\sigma_{\rm Rd}} \leq 1$

 $\frac{V_{Ed}}{V_{Rd}} + \frac{N_{Ed}}{N_{Rd}} \leq 1$

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Verification

The calculation shall be carried out in a linear elastic manner. The stiffness of the substructure shall be considered for the respective case of application.

For the determined anchor loads it shall be verified, that the following equations are met.

Equation 1:

Equation 2:

Equation 3:

with:

N_{Ed} = design value of the tensile force acting on the anchor

 V_{Ed} = design value of the shear force acting on the anchor

 N_{Rd} = design value of the tensile load bearing capacity of the anchor

 V_{Rd} = design value of the shear load bearing capacity of the anchor

For the determined panel loads it shall be verified, that the following equation according is met:

Equation 4:

with:

 σ_{Ed} = design value of the bending stress of the façade panel

 σ_{Rd} = design value of the bending stress resistance of the façade panel

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Intended use Design method

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Requirements to the façade panels

The HPL façade panels shall be classified "EDS" or "EDF" according to EN 438-6:2014.

The minimum requirements for the façade panels are documented in the following table

Table 2: Minimum requirements for the façade panel

Characteristic values for the facade panel	Thickness of the panel	h≥	[mm]	8
	Bending stress ¹⁾		N/mm ²	≥ 100
			IN/11111	≥ 130
	Bending modulus	$E_{T}^{3)}$	N/mm²	10000
		$E_{L}^{(3)}$	IN/11111	14000
	Maximum mass increase according to EN 438-2:2016-06, section 15 (Resistance to wet conditions)	δ _w	[%]	2,00

1) σ_{fm} according to EN ISO 178:2013-09

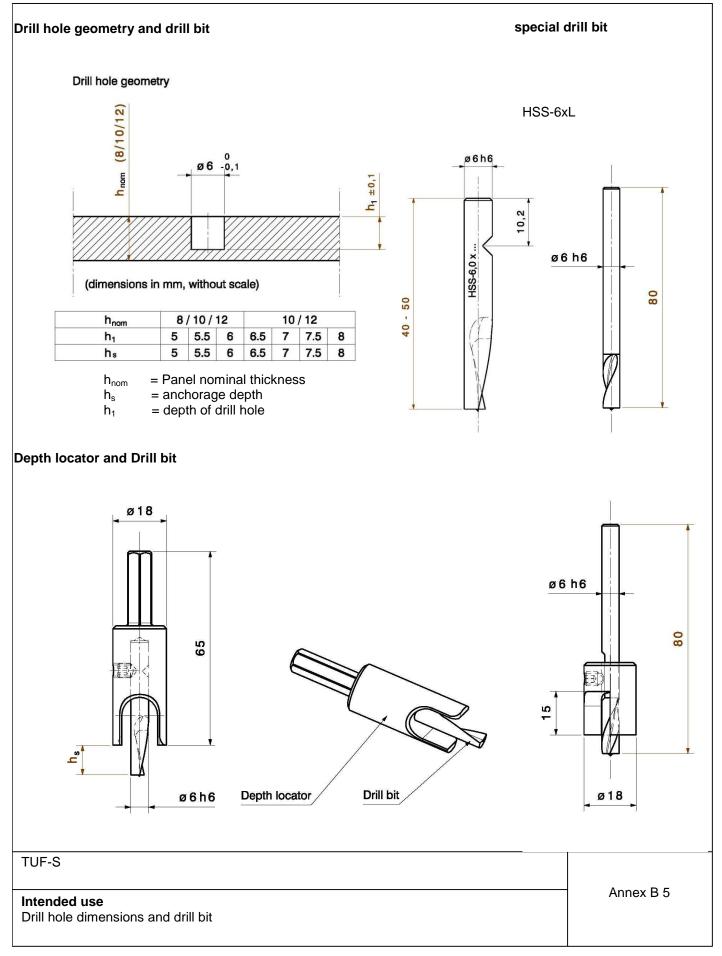
- 2) $\sigma_{fm,T}$: Bending strength transverse
- $\sigma_{fm,L}$: Bending strength longitudinal
- 3) E_T : Bending modulus transverse
 - E_L : Bending modulus longitudinal

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Intended use Requirements to the HPL-facade panels

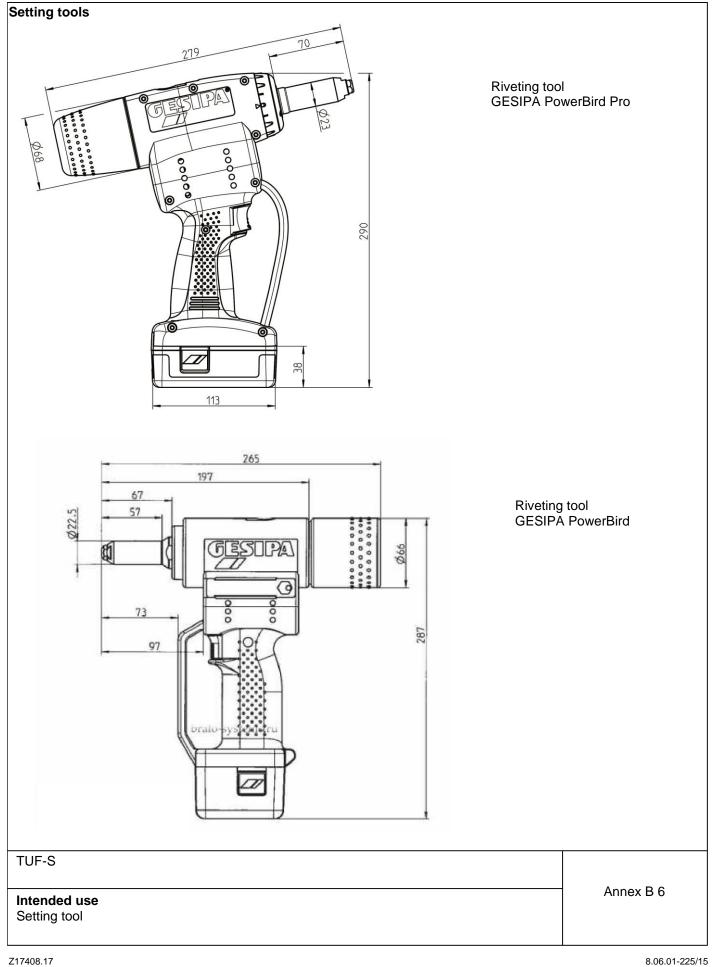
Page 11 of European Technical Assessment ETA-15/0476 of 12 July 2017





Page 12 of European Technical Assessment ETA-15/0476 of 12 July 2017

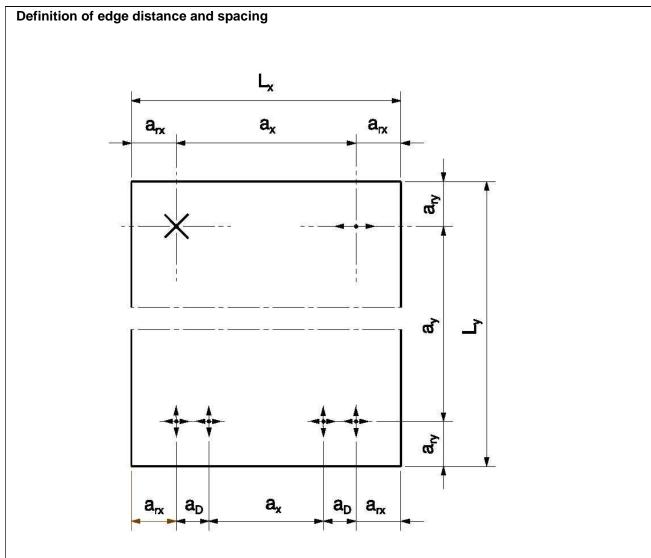




Page 13 of European Technical Assessment ETA-15/0476 of 12 July 2017

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Legend:

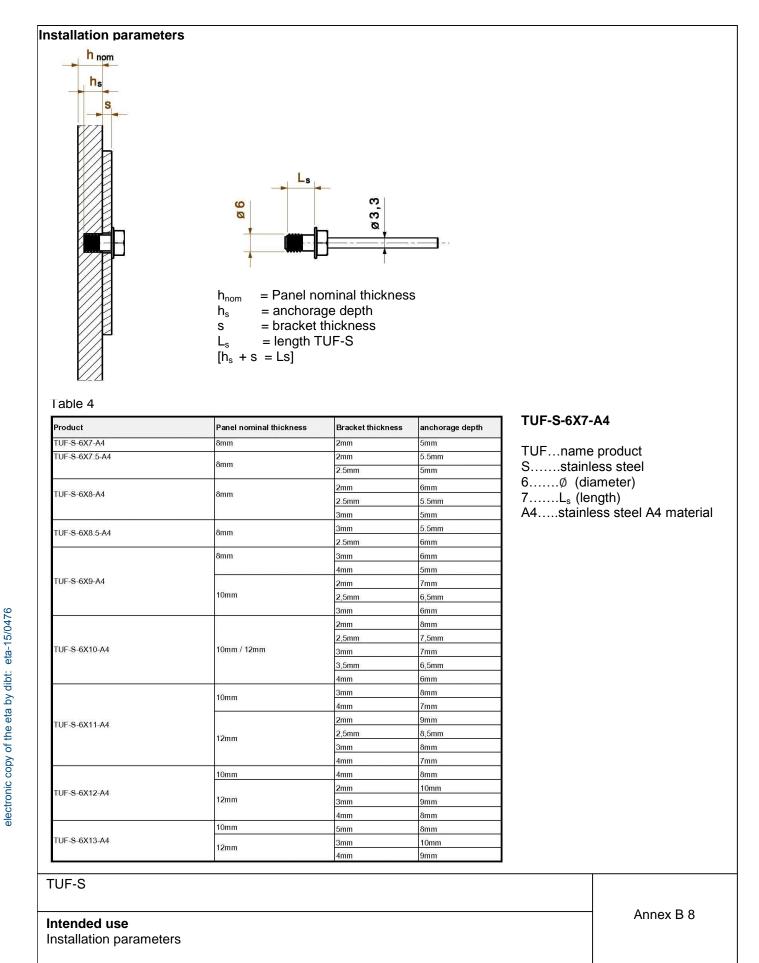
- $a_{rx,y}$ = edge distance distance of an anchor to the panel edge
- a_{x,y} = spacing between outer anchors in adjoining groups or between single anchors distance between anchors
- a_D = spacing of anchors in an anchor group
- L_x = greater length of the façade panel
- L_y = smaller length of the façade panel
- \times = fixed point (fixed bearing)
- ++ = horizontal skid (loose bearing)
- +++ = horizontal and vertical skid (loose bearing)

TUF-S

Intended use Definition of edge distance and spacing

Page 14 of European Technical Assessment ETA-15/0476 of 12 July 2017

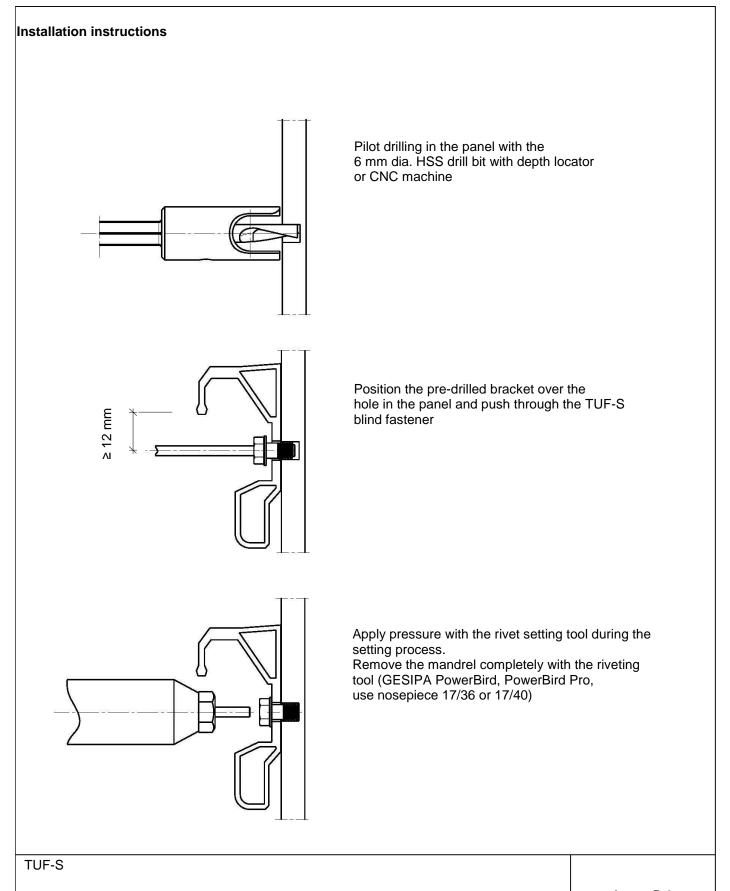




Page 15 of European Technical Assessment ETA-15/0476 of 12 July 2017

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Intended use Installation instructions



Characteristic va	lues of the and	nor with s	ingle cla	mp						
			U							
Setting depth ²		[mm]	5,0	5,5	6,0	6,5	7,0	7,5	8,0	
Characteristic resistance	Tension ¹	N _{Rk}	[kN]	1,12	1,26	1,40	1,65	1,90	1,97	2,04
	Shear	V _{Rk}	[kN]	2,78	2,89	2,99	3,28	3,57	3,79	4,00
C resistance Shear Shear Edge distance		ar	[mm]	≥ 40						
Spacing			[mm]	≥ 100						
	Setting depth ² Characteristic resistance Edge distance	Setting depth ² Characteristic resistance Shear Edge distance	Setting depth 2 Single Characteristic resistance Tension 1 N _{Rk} Shear V _{Rk} Edge distance a _r	Setting depth ² [mm] Characteristic resistance Tension ¹ N _{Rk} [kN] Shear V _{Rk} [kN] Edge distance a _r [mm]	Setting depth 2[mm]5,0Characteristic resistanceTension 1 N_{Rk} [kN]1,12Shear V_{Rk} [kN]2,78Edge distancear[mm]Incompare	Single clampSetting depth 2[mm] $5,0$ $5,5$ Characteristic resistanceTension 1 N_{Rk} [kN] $1,12$ $1,26$ Shear V_{Rk} [kN] $2,78$ $2,89$ Edge distancear[mm] $rest = 100000000000000000000000000000000000$	Single clamp Setting depth 2 [mm] 5,0 5,5 6,0 Characteristic resistance Tension 1 N _{Rk} [kN] 1,12 1,26 1,40 Characteristic resistance V _{Rk} [kN] 2,78 2,89 2,99 Edge distance ar [mm]	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Setting depth 2 [mm] 5,0 5,5 6,0 6,5 7,0 Characteristic resistance Tension 1 N _{Rk} [kN] 1,12 1,26 1,40 1,65 1,90 Characteristic resistance V _{Rk} [kN] 2,78 2,89 2,99 3,28 3,57 Edge distance ar [mm] ≥ 40	Single clamp Setting depth 2 [mm] 5,0 5,5 6,0 6,5 7,0 7,5 Characteristic resistance Tension 1 N _{Rk} [kN] 1,12 1,26 1,40 1,65 1,90 1,97 Shear V _{Rk} [kN] 2,78 2,89 2,99 3,28 3,57 3,79 Edge distance ar [mm] ≥ 40

Double clamp with 20,0 mm $\leq a_D < 40,0$ mm											
Characteristic values for two anchors	Setting depth ²			[mm]	5,0	5,5	6,0	6,5	7,0	7,5	8,0
	Characteristic resistance	Tension ¹	N _{Rk}	[kN]	1,93	2,03	2,11	2,41	2,71	2,71	2,71
		Shear	V _{Rk}	[kN]	4,85	4,85	4,85	5,83	6,80	6,80	6,80
	Edge distance		ar	[mm]	≥ 40						
Chai	Spacing			[mm]	≥ 100						

Table 7: Characteristic values of the anchor with double clamp (40,0 mm $\leq a_D < 100,0$ mm)

	Double clamp with 40,0 mm ≤ a _D < 100,0 mm												
Characteristic values for two anchors	Setting depth ²			[mm]	5,0	5,5	6,0	6,5	7,0	7,5	8,0		
	Characteristic resistance	Tension ¹	N _{Rk}	[kN]	2,07	2,26	2,44	3,17	3,89	3,89	3,89		
		Shear	V _{Rk}	[kN]	4,85	4,85	4,85	5,83	6,80	6,80	6,80		
	Edge distance		ar	[mm]	≥ 40								
Chai	Spacing			[mm]	≥ 100								

1 Values valid for bending angle of the façade panels $\beta \le 1,0^{\circ}$ (Definition of β see Annex B 2)

- 2 A minimum remaining panel thickness (panel thickness setting depth) of 2,0 mm is required.
 - For intermediate values of the setting depth, linear interpolation is possible.

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Performances Characteristic value of the anchor Annex C 1