



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-17/0570 of 7 September 2017

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

Deutsches Institut für Bautechnik

VJ Technology Injection system 420+ Hybrid for concrete

Bonded anchor for use in concrete

VJ Technology Brunswick Road; Cobbs Wood Ind. Estate ASHFORD KENT TN23 1EN . GROSSBRITANNIEN

VJ Technology Plant 1

24 pages including 3 annexes which form an integral part of this assessment

ETAG 001 Part 5: "Bonded anchors", April 2013, used as EAD according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011.



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Z41369.17 8.06.01-195/17



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Specific Part

1 Technical description of the product

The "VJ Technology Injection system 420+ Hybrid for concrete" is a bonded anchor consisting of a cartridge with injection mortar 420+ Hybrid and a steel element. The steel element consist of a commercial threaded rod with washer and hexagon nut in the range of M8 to M30, reinforcing bar in the range of diameter $\emptyset 8$ to $\emptyset 32$ mm or internal threaded rod IG-M6 to IG-M20.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between metal part, injection mortar and concrete.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance for static and quasi-static action and seismic performance categories C1, C2	See Annex C 1 to C 7
Displacements	See Annex C 8 to C 10

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	No performance assessed

3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances there may be requirements (e.g. transposed European legislation and national laws, regulations and administrative provisions) applicable to the products falling within the scope of this European Technical Assessment. In order to meet the provisions of Regulation (EU) No 305/2011, these requirements need also to be complied with, when and where they apply.

3.4 Safety in use (BWR 4)

The essential characteristics regarding Safety in use are included under the Basic Works Requirement Mechanical resistance and stability.

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4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with guideline for European technical approval ETAG 001, April 2013 used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011 the applicable European legal act is: [96/582/EC].

The system to be applied is: 1

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

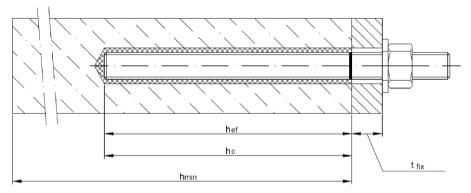
Issued in Berlin on 7 September 2017 by Deutsches Institut für Bautechnik

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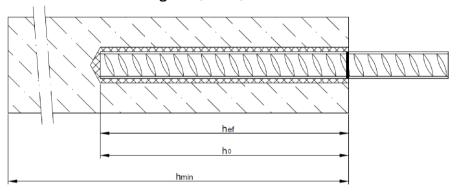
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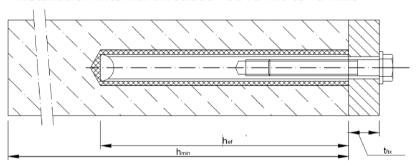
Installation threaded rod M8 to M30



Installation reinforcing bar Ø8 to Ø32



Installation internal threaded rod IG-M6 to IG-M20



 t_{fix} = thickness of fixture

h_{ef} = effective anchorage depth

 $h_0 = depth of drill hole$

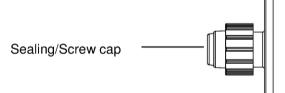
 h_{min} = minimum thickness of member

VJ Technology Injection system 420+ Hybrid for concrete Product description Installed condition Annex A 1



Cartridge: 420+ Hybrid

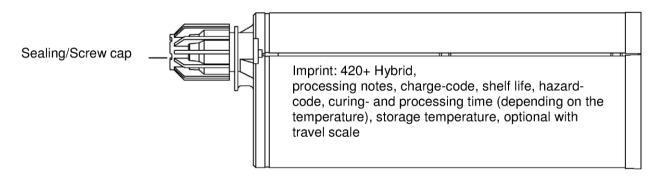
150 ml, 280 ml, 300 ml up to 333 ml and 380 ml up to 420 ml cartridge (Type: coaxial)



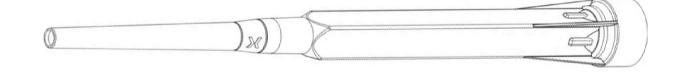
Imprint: 420+ Hybrid,

processing notes, charge-code, shelf life, hazard-code, curing- and processing time (depending on the temperature), storage temperature, optional with travel scale

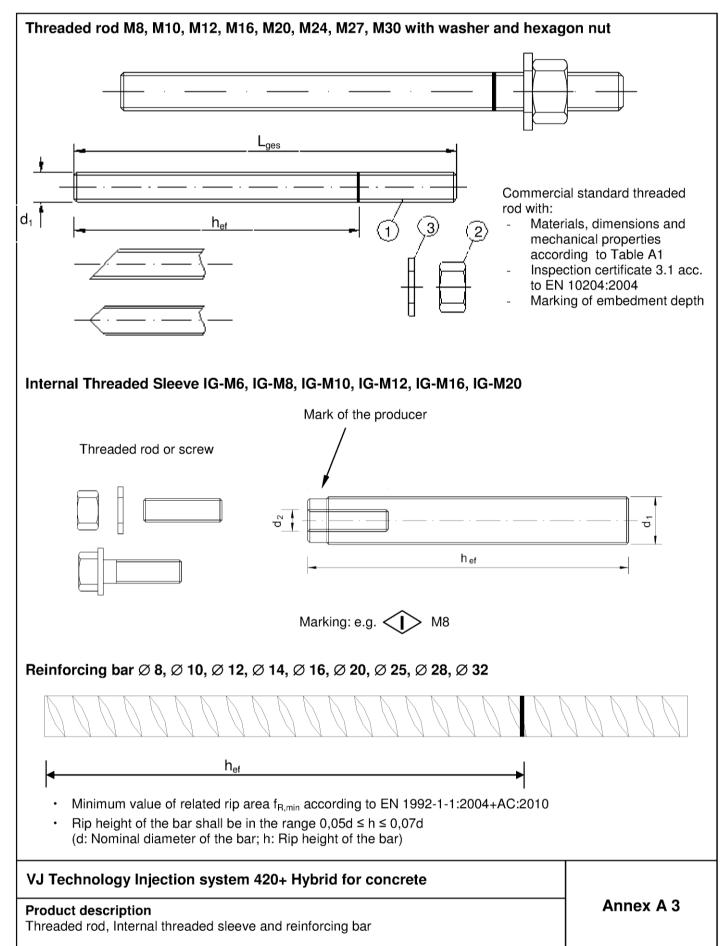
235 ml, 345 ml up to 360 ml and 825 ml cartridge (Type: "side-by-side")



Static Mixer



VJ Technology Injection system 420+ Hybrid for concrete Product description Injection system Annex A 2



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Designation	Material			
Steel, zinc plated ≥ 5 µm acc. to EN ISO 4042:1				
Steel, hot-dip galvanised ≥ 40 µm acc. to EN IS				
Anchor rod	Steel, EN 10087:1998 or EN 10263:2001 Property class 4.6, 4.8, 5.6, 5.8, 8.8, EN 1993-1- 8:2005+AC:2009 A ₅ > 12% fracture elongation			
Hexagon nut, EN ISO 4032:2012	Steel acc. to EN 10087:1998 or EN 10263:2001 Property class 4 (for class 4.6 and 4.8 rod) EN ISO 898-2:2012, Property class 5 (for class 5.6 and 5.8 rod) EN ISO 898-2:2012, Property class 8 (for class 8.8 rod) EN ISO 898-2:2012			
Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Steel, zinc plated Property class 5.6, 5.8 and 8.8 EN ISO 8	98-1:2013		
Internal threaded rod	Steel, zinc plated			
Stainless steel				
Anchor rod	Material 1.4401 / 1.4404 / 1.4571, EN 10088-1:2005, > M24: Property class 50 EN ISO 3506-1:2009 ≤ M24: Property class 70 EN ISO 3506-1:2009 A ₅ > 12% fracture elongation			
Hexagon nut, EN ISO 4032:2012	Material 1.4401 / 1.4404 / 1.4571 EN 10088:2005, > M24: Property class 50 (for class 50 rod) EN ISO 3506-2:2009 ≤ M24: Property class 70 (for class 70 rod) EN ISO 3506-2:2009			
Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4401, 1.4404 or 1.4571, EN 10088-1:2005			
Internal threaded rod	Stainless steel: 1.4401 / 1.4404 / 1.4571, EN 10088-1:2014 Property class 70 (for class 70 rod) EN ISO 3506-1:2009			
High corrosion resistant steel				
Anchor rod	Material 1.4529 / 1.4565, EN 10088-1:20 > M24: Property class 50 EN ISO 3506-1 ≤ M24: Property class 70 EN ISO 3506-1 A ₅ > 12% fracture elongation	:2009 :2009		
Hexagon nut, EN ISO 4032:2012	Material 1.4529 / 1.4565 EN 10088-1:200 > M24: Property class 50 (for class 50 ro ≤ M24: Property class 70 (for class 70 ro	d) EN ISO 3506-2:2009		
Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4529 / 1.4565, EN 10088-1:20	05		
Internal threaded rod	Stainless steel: 1.4529 / 1.4565, EN 10088-1:2014 Property class 70 (for class 70 rod) EN ISO 3506-1:2009			
Reinforcing bars				
Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C f_{yk} and k according to NDP or NCL of EN 1992-1-1/NA:2013 $f_{uk} = f_{tk} = k \cdot f_{yk}$			
VJ Technology Injection system 420+ Hyl	brid for concrete			



Specifications of intended use

Anchorages subject to:

- Static and quasi-static loads: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.
- Seismic action for Performance Category C1: M8 to M30 (except hot-dip galvanised rods), Rebar Ø8 to Ø32.
- Seismic action for Performance Category C2: M12 (except hot-dip galvanised rods)

Base materials:

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000.
- Strength classes C20/25 to C50/60 according to EN 206-1:2000.
- Non-cracked concrete: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.
- Cracked concrete: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.

Temperature Range:

- I: 40 °C to +80 °C (max long term temperature +50 °C and max short term temperature +80 °C)
- II: 40 °C to +120 °C (max long term temperature +72 °C and max short term temperature +120 °C)
- III: 40 °C to +160 °C (max long term temperature +100 °C and max short term temperature +160 °C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc coated steel, stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist (high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The
 position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to
 reinforcement or to supports, etc.).
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Anchorages under static or quasi-static actions are designed in accordance with:
 - EOTA Technical Report TR 029 "Design of bonded anchors", Edition September 2010 or
 - CEN/TS 1992-4:2009
- Anchorages under seismic actions (cracked concrete) are designed in accordance with:
 - EOTA Technical Report TR 045 "Design of Metal Anchors under Seismic Action", Edition February 2013
 - Anchorages shall be positioned outside of critical regions (e.g. plastic hinges) of the concrete structure.
 - Fastenings in stand-off installation or with a grout layer are not allowed.

Installation:

- · Dry or wet concrete.
- Hole drilling by hammer or compressed air drill mode.
- · Overhead installation allowed.
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.
- Fastening screws or threaded rods (incl. nut and washer) must comply with the appropriate material and property class of the Internal threaded rod.

VJ Technology Injection system 420+ Hybrid for concrete	
Intended Use Specifications	Annex B 1

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Table B1: Installation parameters for threaded rod									
Anchor size		М 8	M 10	M 12	M 16	M 20	M 24	M 27	M 30
Diameter of element	$d_1 = d_{nom} [mm] =$	8	10	12	16	20	24	27	30
Nominal drill hole diameter	d ₀ [mm] =	10	12	14	18	22	28	30	35
Effective anabarage depth	h _{ef,min} [mm] =	60	60	70	80	90	96	108	120
Effective anchorage depth	h _{ef,max} [mm] =	160	200	240	320	400	480	540	600
Diameter of clearance hole in the fixture ¹⁾	d _f [mm] =	9	12	14	18	22	26	30	33
Installation torque	T _{inst} [Nm] ≤	10	20	40 ²⁾	60	100	170	250	300
Minimum thickness of member	h _{min} [mm]	h _{ef} + 30 mm ≥ 100 mm				h _{ef} + 2d ₀			
Minimum spacing	s _{min} [mm]	40	50	60	75	95	115	125	140
Minimum edge distance	c _{min} [mm]	35	40	45	50	60	65	75	80

For larger clearance hole see TR029 section 1.1; for application under seismic loading the diameter of clearance hole in the fixture shall be at maximum $d_1 + 1$ mm or alternatively the annular gap between fixture and anchor rod shall be filled force-fit with mortar.

Table B2: Installation parameters for rebar

Rebar size		Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Diameter of element	$d = d_{nom} [mm] =$	8	10	12	14	16	20	25	28	32
Nominal drill hole diameter	d ₀ [mm] =	12	14	16	18	20	25	32	35	40
Effective encloses doubt	h _{ef,min} [mm] =	60	60	70	75	80	90	100	112	128
Effective anchorage depth	h _{ef,max} [mm] =	160	200	240	280	320	400	500	560	640
Minimum thickness of member	h _{min} [mm]	h 30 mm								
Minimum spacing	s _{min} [mm]	40	50	60	70	75	95	120	130	150
Minimum edge distance	c _{min} [mm]	35	40	45	50	50	60	70	75	85

Table B3: Installation parameters for Internal threaded rod

Anchor size		IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20
Internal diameter of sleeve	d ₂ [mm] =	6	8	10	12	16	20
Outer diameter of sleeve ²⁾	$d_1 = d_{nom} [mm] =$	10	12	16	20	24	30
Nominal drill hole diameter	$d_0 [mm] =$	12	14	18	22	28	35
Effective anchorage depth	h _{ef,min} [mm] =	60	70	80	90	96	120
Effective anchorage depth	$h_{ef,max} [mm] =$	200	240	320	400	480	600
Diameter of clearance hole in the fixture ¹⁾	$d_f [mm] =$	7	9	12	14	18	22
Installation torque	T _{inst} [Nm] ≤	10	10	20	40	60	100
Thread engagement length Min/max	I_{IG} [mm] =	8/20	8/20	10/25	12/30	16/32	20/40
Minimum thickness of member	h _{min} [mm]	h _{ef} + 30 mm ≥ 100 mm			h _{ef} +	- 2d ₀	
Minimum spacing	s _{min} [mm]	50	60	75	95	115	125
Minimum edge distance	c _{min} [mm]	40	45	50	60	65	75

¹⁾ For larger clearance hole see TR029 section 1.1

With metric threads according to EN 1993-1-8:2005+AC:2009

VJ Technology Injection system 420+ Hybrid for concrete	
Intended Use Installation parameters	Annex B 2

²⁾ Maximum Torque moment for M12 with steel Grade 4.6 is 35 Nm



Table B4: Parameter cleaning and setting tools										
Marie Court and the Court of th			- Marine							
Rebar	Internal threaded rod	d₀ Drill bit - Ø			d _{b,min} min. Brush - Ø	Piston plug				
(mm)	(mm)	(mm)		(mm)	(mm)		1	→	1	
		10	RB10	11,5	10,5	-	-	-	-	
8	IG-M6	12	RB12	13,5	12,5	-	-	-	-	
10	IG-M8	14	RB14	15,5	14,5	-	-	-	-	
12		16	RB16	17,5	16,5	-	-	-	-	
14	IG-M10	18	RB18	20,0	18,5	VS18				
16		20	RB20	22,0	20,5	VS20				
	IG-M12	22	RB22	24,0	22,5	VS22				
20		25	RB25	27,0	25,5	VS25	h . >	h . >		
	IG-M16	28	RB28	30,0	28,5	VS28			all	
	10 11110	30	RB30	31,8	30,5	VS30	250 mm	250 mm		
	Rebar (mm) 8 10 12 14 16	Internal threaded rod (mm)	Internal threaded rod	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	

32

35

40

RB32

RB35

RB40

34,0

37,0

43,5



IG-M20

25

28

32

M30

MAC - Hand pump (volume 750 ml)Drill bit diameter (d_0) : 10 mm to 20 mm
Drill hole depth (h_0) : < 10 d_s Only in non-cracked concrete



VS32

VS35

VS40

32,5

35,5

40,5

CAC - Rec. compressed air tool (min 6 bar) Drill bit diameter (d₀): all diameters



Piston plug for overhead or horizontal installation VS

Drill bit diameter (d₀): 18 mm to 40 mm



Steel brush RB

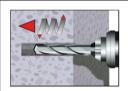
Drill bit diameter (d₀): all diameters

VJ Technology Injection system 420+ Hybrid for concrete	
Intended Use Cleaning and setting tools	Annex B 3



Installation instructions

Drilling of the bore hole



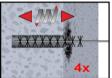
1. Drill with hammer drill a hole into the base material to the size and embedment depth required by the selected anchor (Table B1, B2, or B3). In case of aborted drill hole: the drill hole shall be filled with mortar

Attention! Standing water in the bore hole must be removed before cleaning.

MAC: Cleaning for bore hole diameter $d_0 \le 20$ mm and bore hole depth $h_0 \le 10d_s$ (uncracked concrete only!)



2a. Starting from the bottom or back of the bore hole, blow the hole clean by a hand pump (Annex B 3) a minimum of four times.



2b. Check brush diameter (Table B4). Brush the hole with an appropriate sized wire brush > d_{b,min} (Table B4) a minimum of four times in a twisting motion.
If the bore hole ground is not reached with the brush, a brush extension must be used.

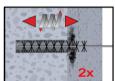


2c. Finally blow the hole clean again with a hand pump (Annex B 3) a minimum of four times.

CAC: Cleaning for all bore hole diameter in uncracked and cracked concrete



2a. Starting from the bottom or back of the bore hole, blow the hole clean with compressed air (min. 6 bar) (Annex B 3) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension must be used.



2b. Check brush diameter (Table B4). Brush the hole with an appropriate sized wire brush > d_{b,min} (Table B4) a minimum of two times.
If the bore hole ground is not reached with the brush, a brush extension must be used.



2c. Finally blow the hole clean again with compressed air (min. 6 bar) (Annex B 3) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension must be used.

After cleaning, the bore hole has to be protected against re-contamination in an appropriate way, until dispensing the mortar in the bore hole. If necessary, the cleaning has to be repeated directly before dispensing the mortar. In-flowing water must not contaminate the bore hole again.

VJ Technology Injection system 420+ Hybrid for concrete

Intended Use

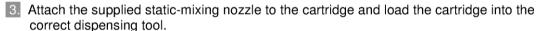
Installation instructions

Annex B 4

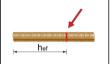


Installation instructions (continuation)





For every working interruption longer than the recommended working time (Table B5) as well as for new cartridges, a new static-mixer shall be used.



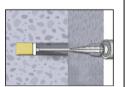
4. Prior to inserting the anchor rod into the filled bore hole, the position of the embedment depth shall be marked on the anchor rods.



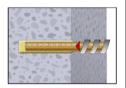
5. Prior to dispensing into the anchor hole, squeeze out separately a minimum of three full strokes and discard non-uniformly mixed adhesive components until the mortar shows a consistent grey colour.



6. Starting from the bottom or back of the cleaned anchor hole, fill the hole up to approximately two-thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid creating air pockets. For embedment larger than 190 mm an extension nozzle shall be used. Observe the gel-/ working times given in Table B5.

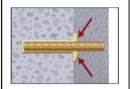


- 7. Piston Plugs and mixer nozzle extensions shall be used according to Table B4 for the following applications:
 - Horizontal assembly (horizontal direction) and ground erection (vertical downwards direction): Drill bit- \emptyset d₀ \ge 18 mm and embedment depth h_{ef} > 250mm
 - Overhead assembly (vertical upwards direction): Drill bit-Ø d₀ ≥ 18 mm

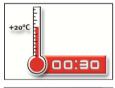


8. Push the threaded rod or reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached.

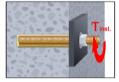
The anchor shall be free of dirt, grease, oil or other foreign material.



9. Be sure that the anchor is fully seated at the bottom of the hole and that excess mortar is visible at the top of the hole. If these requirements are not maintained, the application has to be renewed. For overhead application the anchor rod shall be fixed (e.g. wedges).



10. Allow the adhesive to cure to the specified time prior to applying any load or torque. Do not move or load the anchor until it is fully cured (attend Table B5).



11. After full curing, the add-on part can be installed with up to the max. torque (Table B1 or B3) by using a calibrated torque wrench.

VJ Technology Injection system 420+ Hybrid for concrete

Intended Use

Installation instructions (continuation)

Annex B 5

Z36370.17



Table B5: Max	rimum working	time and	minimum	curing time
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Concrete temperature		perature	Gelling working time	Minimum curing time in dry concrete	Minimum curing time in wet concrete		
- 5 °C	to	- 1 °C	50 min	5 h	10 h		
0 °C	to	+ 4 °C	25 min	3,5 h	7 h		
+ 5 °C	to	+ 9 °C	15 min	2 h	4 h		
+ 10 °C	to	+ 14 °C	10 min	1 h	2 h		
+ 15 °C	to	+ 19 °C	6 min	40 min	80 min		
+ 20 °C	to	+ 29 °C	3 min	30 min	60 min		
+ 30 °C	to	+ 40 °C	2 min	30 min	60 min		
Cartridge	temp	oerature	+5°C to +40°C				

VJ Technology Injection system 420+ Hybrid for concrete

Intended Use
Curing time

Annex B 6

Table C1:	Characteristic values for steel tension resistance and steel shear
	resistance of threaded rods

Size				M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
	acteristic tension resistance, Steel failure				111110			111 20	1412-4	27	00
	Property class 4.6 and 4.8	$N_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Steel,	Property class 5.6 and 5.8	N _{Rk,s}	[kN]	18	29	42	78	122	176	230	280
	Property class 8.8	N _{Rk,s}	[kN]	29	46	67	125	196	282	368	449
	ostender Stahl A4 and HCR, Property class 50	N _{Rk,s}	[kN]	18	29	42	79	123	177	230	281
	ostender Stahl A4 and HCR, Property class 70	N _{Rk,s}	[kN]	26	41	59	110	171	247	-	-
	acteristic tension resistance, Partial safety factor	,.									
Steel,	Property class 4.6	γ _{Ms,N} 1)	[-]				2	,0			
Steel,	Property class 4.8	γ _{Ms,N} 1)	[-]				1	,5			
Steel,	Property class 5.6	γ _{Ms,N} 1)	[-]				2	,0			
Steel,	Property class 5.8	γ _{Ms,N} 1)	[-]				1	,5			
Steel,	Property class 8.8	γ _{Ms,N} 1)	[-]				1	,5			
Stainl	ess steel A4 and HCR, Property class 50	γ _{Ms,N} 1)	[-]	2,86							
Stainl	ess steel A4 and HCR, Property class 70	γ _{Ms,N} 1)	[-]				1,	87			
Chara	cteristic shear resistance, Steel failure										
E	Steel, Property class 4.6 and 4.8	$V_{Rk,s}$	[kN]	7	12	17	31	49	71	92	112
er ar	Steel, Property class 5.6 and 5.8	$V_{Rk,s}$	[kN]	9	15	21	39	61	88	115	140
t lev	Steel, Property class 8.8	$V_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Without lever arm	Stainless steel A4 and HCR, Property class 50	$V_{Rk,s}$	[kN]	9	15	21	39	61	88	115	140
\geq	Stainless steel A4 and HCR, Property class 70	$V_{Rk,s}$	[kN]	13	20	30	55	86	124	-	-
_	Steel, Property class 4.6 and 4.8	M _{Rk,s}	[Nm]	15	30	52	133	260	449	666	900
arm	Steel, Property class 5.6 and 5.8	M _{Rk,s}	[Nm]	19	37	65	166	324	560	833	1123
ever	Steel, Property class 8.8	M _{Rk,s}	[Nm]	30	60	105	266	519	896	1333	1797
With lever	Stainless steel A4 and HCR, Property class 50	M _{Rk,s}	[Nm]	19	37	66	167	325	561	832	1125
>	Stainless steel A4 and HCR, Property class 70	M _{Rk,s}	[Nm]	26	52	92	232	454	784	-	-
Chara	cteristic shear resistance, Partial safety factor					•					
Steel,	Property class 4.6	γ _{Ms,V} 1)	[-]				1,	67			
Steel,	Property class 4.8	γ _{Ms,V} 1)	[-]				1,	25			
	Property class 5.6	γ _{Ms,V} 1)	[-]	1;67							
	Property class 5.8	γ _{Ms,V} 1)	[-]					25			
Steel,	Property class 8.8	γ _{Ms,V} 1)	[-]	1,25							
	ess steel A4 and HCR, Property class 50	γMs,V 1)	[-]	2,38							
Stainl	ess steel A4 and HCR, Property class 70	γ _{Ms,V} 1)	[-]	1,56							

¹⁾ in absence of national regulation

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Performances

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Characteristic values for steel tension resistance and steel shear resistance of threaded rods

Annex C 1



Table C2:	Characteristic values of tension loads for threaded rods under static,
	quasi-static action and seismic action (performance category C1+C2)

Anchor size threaded	l rod			М 8	M 10	M 12	M 16	M 20	M24	M 27	M 30	
Steel failure												
		N _{Rk,s}	[kN]				see Ta	able C1				
Characteristic tension	rociotanos	N _{Rk,s,C1}	[kN]				1,0 •	N _{Rks}				
Characteristic terision	resistance	N _{Rk,s,C2}	[kN]	N	PD	1,0 • N _{Rk,s}			nce Dete	rmined (N	NPD)	
Partial safety factor		γ _{Ms,N}	[-]			, -rin,5	see Ta	able C1				
•	nd concrete cone failur											
Characteristic bond res	sistance in non-cracked	concrete C20/2	25									
Temperature range I: 80°C/50°C	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	17	17	16	15	14	13	13	13	
Temperature range II: 120°C/72°C	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	15	14	14	13	12	12	11	11	
Temperature range III: 160°C/100°C	dry and wet concrete	$ au_{ m Rk,ucr}$	[N/mm²]	12	12	11	10	9,5	9,0	9,0	9,0	
Characteristic bond res	sistance in cracked conc	rete C20/25										
Temperature range I:	dry and wet concrete	$\tau_{\text{Rk,cr}} = \tau_{\text{Rk,C1}}$	[N/mm ²]	6,5	7,0	7,5	8,5	8,5	8,5	8,5	8,5	
80°C/50°C	dry and wet concrete	τ _{Rk,C2}	[N/mm ²]	N	PD	3,6			NPD			
Temperature range II:	dry and wet concrete	$\tau_{\text{Rk,cr}} = \tau_{\text{Rk,C1}}$	[N/mm ²]	5,5	6,0	6,5	7,5	7,5	7,5	7,5	7,5	
120°C/72°C	dry and wet concrete	τ _{Rk,C2}	[N/mm ²]	NPD		3,1			NPD			
Temperature range III:	dry and wet concrete	$\tau_{\text{Rk,cr}} = \tau_{\text{Rk,C1}}$	[N/mm ²]	5,0	5,5	6,0	6,5	6,5	6,5	6,5	6,5	
160°C/100°C	dry and wet condicte	τ _{Rk,C2}	[N/mm ²]	N	PD	2,5			NPD			
		C25						02				
		C30/37 1,04										
Increasing factors for c	concrete	C35						07				
Ψο		C40/50		1,08								
		C45		1,09 1,10								
	T	C50.	/60									
Factor according to CEN/TS 1992-4-5	Non-cracked concrete	k ₈	[-]				10),1				
Section 6.2.2.3	Cracked concrete	1/8	[-]	7,2								
Concrete cone failure	•											
Factor according to CEN/TS 1992-4-5	Non-cracked concrete	k _{ucr}	[-]				10),1				
Section 6.2.3.1	Cracked concrete	k _{cr}	[-]				7	,2				
Edge distance		C _{cr,N}	[mm]				1,5	h _{ef}				
Axial distance		S _{cr,N}	[mm]				3,0	h _{ef}				
Splitting failure												
	h/h _{ef} ≥ 2,0						1,0	h _{ef}				
Edge distance 2	2,0> h/h _{ef} > 1,3	C _{cr,sp}	[mm]	$2 \cdot h_{e\!f} \left(2, 5 - \frac{h}{h_{e\!f}} \right)$								
	h/h _{ef} ≤ 1,3							h _{ef}				
Axial distance		S _{cr,sp}	[mm]				2 0	cr,sp				
Installation safety facto (dry and wet concrete)		γ2 = Yinst	[-]		1,0 (1,2)1)			1	,2		
	y and wet concrete) stallation safety factor (MAC) y and wet concrete)		[-]	1,2								

Value in brackets for cracked concrete

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Characteristic values of tension loads for threaded rods under static, quasi-static action and seismic action (performance category C1+C2)

Annex C 2



Table C3:	Characteristic values of shear static action and seismic action							, quas	i-
Anchor size threa	ded rod	М 8	M 10	M 12	M 16	M 20	M24	M 27	м

Anchor size threaded rod			М 8	M 10	M 12	M 16	M 20	M24	M 27	М 30		
Steel failure without lever arm												
	$V_{Rk,s}$	[kN]				see Ta	able C1					
Characteristic shear resistance	$V_{\text{Rk,s,C1}}$	[kN]				0,70	• V _{Rk,s}					
	$V_{\text{Rk,s,C2}}$	[kN]	(N	PD)	0,80 • V _{Rk,s}	No	Performa	ınce Deter	mined (NI	PD)		
Partial safety factor	γMs,V	[-]				see Ta	able C1					
Steel failure with lever arm												
	M ⁰ _{Rk,s}	[Nm]				see Ta	able C1					
Characteristic bending moment	M ⁰ _{Rk,s,C1}	[Nm]			No Perf	ormance	Determine	ed (NPD)				
	M ⁰ _{Rk,s,C2}	[Nm]			No Perf	ormance	Determine	ed (NPD)				
Partial safety factor	γMs,V	[-]				see Ta	able C1					
Concrete pry-out failure												
Factor k_3 in equation (27) of CEN/TS 1992-4-5 Section 6.3.3 Factor k in equation (5.7) of Technical Report TR 029	k ₍₃₎	[-]				2	,0					
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0									
Concrete edge failure	•											
Effective length of anchor	I _f	[mm]	$I_f = min(h_{ef}; 8 d_{nom})$									
Outside diameter of anchor	d _{nom}	[mm]	8	10	12	16	20	24	27	30		

Outside diameter of anchor	d _{nom}	[mm]	8	10	12	16	20	24	27	30
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]	1,0							

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Characteristic values of shear loads for threaded rods under static, quasi-static action and seismic action (performance category C1+C2)

Annex C 3



Anchor size internally	threaded rods			IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20		
Steel failure1)											
Characteristic tension r Steel, strength class 5.8		N _{Rk,s}	[kN]	10	17	29	42	76	123		
Partial safety factor		γMs,N	[-]			1	,5				
Characteristic tension r	,	N _{Rk.s}	[kN]	16	27	46	67	121	196		
Steel, strength class 8.8	8										
Partial safety factor Characteristic tension r	esistance	γMs,N	[-]				,5				
Stainless Steel A4, Stre		$N_{Rk,s}$	[kN]	14	26	41	59	110	172		
Partial safety factor		γMs,N	[-]			1,	87				
Combined pull-out an	d concrete cone failure										
	istance in non-cracked co	ncrete C20/25									
Temperature range I: 80°C/50°C	dry and wet concrete	$ au_{Rk,ucr}$	[N/mm²]	17	16	15	14	13	13		
Temperature range II: 120°C/72°C	dry and wet concrete	$ au_{Rk,ucr}$	[N/mm²]	14	14	13	12	12	11		
Temperature range III: 160°C/100°C	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm²]	12	11	10	9,5	9,0	9,0		
	istance in cracked concre	te C20/25									
Temperature range I: 80°C/50°C	dry and wet concrete	$ au_{Rk,cr}$	[N/mm²]	7,0	7,5	8,5	8,5	8,5	8,5		
Temperature range II: 120°C/72°C	dry and wet concrete	$ au_{Rk,cr}$	[N/mm²]	6,0	6,5	7,5	7,5	7,5	7,5		
Temperature range III: 160°C/100°C	dry and wet concrete	$ au_{Rk,cr}$	[N/mm²]	5,5	6,0	6,5	6,5	6,5	6,5		
		C25/30				-,	02				
		C30/37					04				
Increasing factors for co	oncrete		5/45	1,07							
Ψο			0/50	1,08							
			5/55	1,09 1,10							
	1	C50	0/60								
Factor according to CEN/TS 1992-4-5	Non-cracked concrete	- k ₈	[-]	10,1							
Section 6.2.2.3	Cracked concrete		.,			7	,2				
Concrete cone failure											
Factor according to CEN/TS 1992-4-5	Non-cracked concrete	k _{ucr}	[-]			10),1				
Section 6.2.3.1	Cracked concrete	k _{cr}	[-]			7	,2				
Edge distance		C _{cr,N}	[mm]			1,5	h _{ef}				
Axial distance		S _{cr,N}	[mm]			3,0	h _{ef}				
Splitting failure		•									
	h/h _{ef} ≥ 2,0					1,0	h _{ef}				
Edge distance 2,0> h/h _{cf}	2,0> h/h _{ef} > 1,3	C _{cr,sp}	[mm]	$2 \cdot h_{e\!f} \left(2,5 - rac{h}{h_{e\!f}} ight)$							
	h/h _{ef} ≤ 1,3						h _{ef}				
Axial distance		S _{cr,sp}	[mm]			2 0	cr,sp				
Installation safety facto (dry and wet concrete)	r (CAC)	γ2 = γinst	[-]		1,0 (1,2)2)			1,2			
Installation safety facto	r (MAC)	γ2 = γinst	[-]		1,2			-			
(dry and wet concrete)		, - , - , - , - , - , - , - , - , - , -	1 ''		,		1				

Fastening screws or threaded rods (incl. nut and washer) must comply with the appropriate material and property class of the internal threaded rod. The characteristic tension resistance for steel failure of the given strength class are valid for the internal threaded rod and the fastening element.

Value in brackets for cracked concrete.

VJ Technology Injection system 420+ Hybrid for concrete	
Performances Characteristic values of tension loads for internal threaded rods under static and quasi-static action	Annex C 4

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Table C5: Characteristic values of shear loads for internal threaded rods under static and quasi-static action

Anchor size for internally threaded ro-	ds		IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20
Steel failure without lever arm ¹⁾								
Characteristic shear resistance, Steel, strength class 5.8	$V_{Rk,s}$	[kN]	5	9	15	21	38	61
Partial safety factor	γMs,V	[-]			1,2	25		
Characteristic shear resistance, Steel, strength class 8.8	$V_{Rk,s}$	[kN]	8	14	23	34	60	98
Partial safety factor	γMs,V	[-]			1,2	25		
Characteristic shear resistance, Stainless Steel A4, Strength class 70	$V_{Rk,s}$	[kN]	7	13	20	30	55	86
Partial safety factor	γMs,V	[-]			1,5	66		
Steel failure with lever arm1)								
Characteristic bending moment, Steel, strength class 5.8	M ⁰ _{Rk,s}	[Nm]	8	19	37	66	167	325
Partial safety factor	γMs,V	[-]			1,2	25		
Characteristic bending moment, Steel, strength class 8.8	M ⁰ _{Rk,s}	[Nm]	12	30	60	105	267	519
Partial safety factor	γMs,V	[-]			1,2	25		
Characteristic bending moment, Stainless Steel A4, Strength class 70	M ⁰ _{Rk,s}	[Nm]	11	26	52	92	233	454
Partial safety factor	γMs,V	[-]			1,5	66		
Concrete pry-out failure								
Factor k₃ in equation (27) of CEN/TS 1992-4-5 Section 6.3.3 Factor k in equation (5.7) of Technical Report TR 029	k ₍₃₎	[-]			2,	0		
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]			1,	0		
Concrete edge failure								
Effective length of anchor	I _f	[mm]			$I_f = min(h_e)$	_{if} ; 8 d _{nom})		
Outside diameter of anchor	d _{nom}	[mm]	10	12	16	20	24	30
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]			1,	0		

Fastening screws or threaded rods (incl. nut and washer) must comply with the appropriate material and property class of the internal threaded rod. The characteristic tension resistance for steel failure of the given strength class are valid for the internal threaded rod and the fastening element.

VJ Technology Injection system 420+ Hybrid for concrete	
Performances Characteristic values of shear loads for internal threaded rods under static and quasi-static action	Annex C 5



	Characteristic va								ic, qu	ıasi-s	tatic	
Anchor size reinforci	ng bar			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Steel failure												
Characteristic tension	resistance	N _{Rk,s} = N _{Rk,s,C1}	[kN]					A _s • f _{uk} ²⁾				
Cross section area		As	[mm²]	50	79	113	154	201	214	491	616	804
Partial safety factor		γMs,N	[-]					1,43)				
Combined pull-out ar	nd concrete cone failur	е										
Characteristic bond res	sistance in non-cracked	concrete C20/	25									
Temperature range I: 80°C/50°C	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm²]	14	14	14	14	13	13	13	13	13
Temperature range II: 120°C/72°C	dry and wet concrete	$ au_{Rk,ucr}$	[N/mm²]	13	12	12	12	12	11	11	11	11
Temperature range III: 160°C/100°C	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm²]	10	10	9,5	9,5	9,5	9,0	9,0	9,0	9,0
	sistance in cracked conc	rete C20/25										
Temperature range I: 80°C/50°C	dry and wet concrete	$\tau_{\text{Rk,cr}} = \tau_{\text{Rk,C1}}$	[N/mm²]	5,0	5,5	6,0	6,0	7,5	7,5	7,5	7,5	8,0
Temperature range II: 120°C/72°C	dry and wet concrete	$\tau_{\text{Rk,cr}} = \tau_{\text{Rk,C1}}$	[N/mm²]	4,5	5,0	5,0	5,5	6,5	6,5	6,5	6,5	7,0
Temperature range III: 160°C/100°C	dry and wet concrete	$\tau_{\text{Rk,cr}} = \tau_{\text{Rk,C1}}$	[N/mm²]	4,0	4,5	4,5	5,0	5,5	6,0	6,0	5,5	6,5
	C25/						1,02					
	C30/						1,04					
Increasing factors for c	concrete	C35/						1,07				
ψ_{c}		C40/		1,08								
		C45/		1,09								
Factor according to	I	C50/	60					1,10				
Factor according to CEN/TS 1992-4-5	Non-cracked concrete	k ₈	[-]					10,1				
Section 6.2.2.3	Cracked concrete							7,2				
Concrete cone failure)											
Factor according to CEN/TS 1992-4-5	Non-cracked concrete	k _{ucr}	[-]					10,1				
Section 6.2.3.1	Cracked concrete	k _{cr}	[-]					7,2				
Edge distance		C _{cr,N}	[mm]					1,5 h _{ef}				
Axial distance		S _{cr,N}	[mm]					3,0 h _{ef}				
Splitting failure												
	h/h _{ef} ≥ 2,0							1,0 h _{ef}				
Edge distance	2,0> h/h _{ef} > 1,3	C _{cr,sp}	[mm]				$2 \cdot h_{o}$	_{ef} (2,5 -	$\left(\frac{h}{h_{ef}}\right)$			
	h/h _{ef} ≤ 1,3							2,4 h _{ef}				
Axial distance		S _{cr,sp}	[mm]					2 c _{cr,sp}				
Installation safety facto (dry and wet concrete)	` '	$\gamma_2 = \gamma_{inst}$	[-]			1,0 (1,2))			1	,2	
Installation safety facto (dry and wet concrete)		$\gamma_2 = \gamma_{inst}$	[-]			1,2					-	

VJ Technology Injection system 420+ Hybrid for concrete	
Performances Characteristic values of tension loads for rebar under static, quasi-static action and seismic action (performace category C1)	Annex C 6

Value in brackets for cracked concrete
 f_{uk} shall be taken from the specifications of reinforcing bars
 in absence of national regulation



Table C7: Characteristic values of shear loads for rebar under static, quasi-static action and seismic action (performance category C1)												
Anchor size reinforcing bar			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
Steel failure without lever arm												
Characteristic shear resistance	$V_{Rk,s}$	[kN]	0,50 • N _{Rk,s}									
Characteristic shear resistance	V _{Rk,s,C1}	[kN]	0,37 • N _{Rk,s}									
Partial safety factor	γMs,V	[-]	1,5 ²⁾									
Ductility factor according to CEN/TS 1992-4-5 Section 6.3.2.1	k ₂		0,8									
Steel failure with lever arm												
Characteristic bending moment	M ⁰ _{Rk,s}	[Nm]	1.2 • W _{el} • f _{uk} ¹⁾									
Characteristic bending moment	M ⁰ _{Rk,s,C1}	[Nm]	Im] No Performance Determined (NPD)									
Elastic section modulus	Wel	[mm³]	50	98	170	269	402	785	1534	2155	3217	
Partial safety factor	γMs,V	[-]					1,52)					
Concrete pry-out failure		•										
Factor k_3 in equation (27) of CEN/TS 1992-4-5 Section 6.3.3 Factor k in equation (5.7) of Technical Report TR 029	k ₍₃₎	[-]					2,0					
Installation safety factor	$\gamma_2=\gamma_{inst}$	[-]					1,0					
Concrete edge failure												
Effective length of anchor	l _f	[mm]				$l_f = r$	nin(h _{ef} ; 8	d _{nom})				
Outside diameter of anchor	d _{nom}	[mm]	8	10	12	14	16	20	25	28	32	
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]					1,0					

 $^{^{1)}}_{\rm 2}\,\rm f_{uk}$ shall be taken from the specifications of reinforcing bars in absence of national regulation

VJ Technology Injection system 420+ Hybrid for concrete	
Performances Characteristic values of shear loads for rebar under static, quasi-static action and seismic action (performance category C1)	Annex C 7



				1						
Anchor size thread	led rod		M 8	M 10	M 12	M 16	M 20	M24	M 27	М 30
Non-cracked conc	rete C20/25 un	der static and qua	si-statio	action						
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,031	0,032	0,034	0,037	0,039	0,042	0,044	0,046
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,040	0,042	0,044	0,047	0,051	0,054	0,057	0,060
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]	0,032	0,034	0,035	0,038	0,041	0,044	0,046	0,048
120°C/72°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,042	0,044	0,045	0,049	0,053	0,056	0,059	0,062
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,121	0,126	0,131	0,142	0,153	0,163	0,171	0,179
160°C/100°C	δ _{N∞} -factor	[mm/(N/mm²)]	0,124	0,129	0,135	0,146	0,157	0,168	0,176	0,184
Cracked concrete	C20/25 under s	tatic, quasi-static	and sei	smic C	1 action	1				
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]	0,081	0,083	0,085	0,090	0,095	0,099	0,103	0,106
80°C/50°C	δ _{N∞} -factor	[mm/(N/mm²)]	0,104	0,107	0,110	0,116	0,122	0,128	0,133	0,137
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]	0,084	0,086	0,088	0,093	0,098	0,103	0,107	0,110
120°C/72°C	δ _{N∞} -factor	[mm/(N/mm²)]	0,108	0,111	0,114	0,121	0,127	0,133	0,138	0,143
120 0/12 0			0.040	0.001	0,330	0,349	0,367	0,385	0,399	0,412
	δ_{N0} -factor	[mm/(N/mm ²)]	0,312	0,321	0,550	, 0,010				
Temperature range III: 160°C/100°C	δ_{N0} -factor $\delta_{\text{N}_{\infty}}$ -factor	[mm/(N/mm²)] [mm/(N/mm²)]	0,312	0,321	0,340	0,358	0,377	0,396	0,410	0,424
Temperature range III: 160°C/100°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]						0,396	0,410	0,424
Temperature range III:	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,321			0,358	0,377		0,410	,

¹⁾ Calculation of the displacement

 $\delta_{N0} = \delta_{N0}$ -factor $\cdot \tau$; τ: action bond stress for tension

 $\delta_{N_{\infty}} = \delta_{N_{\infty}} \text{-factor } \cdot \tau;$

Displacements under shear load¹⁾ (threaded rod) Table C9:

Anchor size threaded rod				M 10	M 12	M 16	M 20	M24	M 27	М 30
Non-cracked and cracked concrete C20/25 under static, quasi-static and seismic C1 action										
All temperature	δ_{V0} -factor	[mm/(kN)]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V\infty}\text{-factor}$	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05
Cracked concrete	e C20/25 under :	seismic C2 action								
All temperature	$\delta_{\text{V,seis}(\text{DLS})}$	[mm/(kN)]		rameter mined	0,27	No Parameter Determined (NPD)				
ranges	$\delta_{\text{V,seis}(\text{ULS})}$	[mm/(kN)]		PD)	0,27					

 $^{1)}$ Calculation of the displacement $\delta_{V0}=\delta_{V0}\text{-factor}\ \cdot \text{V}; \qquad \text{V: action shear load} \\ \delta_{V\infty}=\delta_{V\infty}\text{-factor}\ \cdot \text{V};$

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Displacements (threaded rods)	

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Table C10: Displacements under tension load ¹⁾ (rebar)												
Anchor size reinfo	Anchor size reinforcing bar Ø 8 Ø 10 Ø 12 Ø 14 Ø 16 Ø 20 Ø 25 Ø 28 Ø 32											
Non-cracked concrete C20/25 under static and quasi-static action												
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,031	0,032	0,034	0,035	0,037	0,039	0,043	0,045	0,048	
80°C/50°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,040	0,042	0,044	0,045	0,047	0,051	0,055	0,058	0,063	
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]	0,032	0,034	0,035	0,036	0,038	0,041	0,045	0,047	0,050	
120°C/72°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,042	0,044	0,045	0,047	0,049	0,053	0,057	0,060	0,065	
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,121	0,126	0,131	0,137	0,142	0,153	0,164	0,172	0,186	
160°C/100°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,124	0,129	0,135	0,141	0,146	0,157	0,169	0,177	0,192	
Cracked concrete	C20/25 ui	nder static, qua	si-statio	and se	ismic C	1 actio	n					
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,081	0,083	0,085	0,087	0,090	0,095	0,099	0,103	0,108	
80°C/50°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,104	0,107	0,110	0,113	0,116	0,122	0,128	0,133	0,141	
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]	0,084	0,086	0,088	0,090	0,093	0,098	0,103	0,107	0,113	
120°C/72°C	$\delta_{N\infty}\text{-factor}$	[mm/(N/mm²)]	0,108	0,111	0,114	0,118	0,121	0,127	0,133	0,138	0,148	
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,312	0,321	0,330	0,340	0,349	0,367	0,385	0,399	0,425	
160°C/100°C	$\delta_{N_{\infty}}\text{-factor}$	[mm/(N/mm²)]	0,321	0,330	0,340	0,349	0,358	0,377	0,396	0,410	0,449	

¹⁾ Calculation of the displacement

 $\delta_{N0} = \delta_{N0}$ -factor \cdot τ ; τ : action bond stress for tension

 $\delta_{N_{\infty}} = \delta_{N_{\infty}} \text{-factor } \cdot \tau;$

Table C11: Displacement under shear load 1) (rebar)

Anchor size reinforcing bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
For concrete C20/25 under static, quasi-static and seismic C1 action											
All temperature	δ_{V0} -factor	[mm/(kN)]	0,06	0,05	0,05	0,04	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V\infty}$ -factor	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,04	0,04

¹⁾ Calculation of the displacement

$$\begin{split} \delta_{V0} &= \delta_{V0}\text{-factor} \cdot V; \\ \delta_{V\infty} &= \delta_{V\infty}\text{-factor} \cdot V; \end{split}$$
V: action shear load

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Table C12: Displacements under tension load ¹⁾ (Internal threaded rod)									
Anchor size Interna	al threaded ro	d	IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20	
Non-cracked conc	rete C20/25 un	der static and quas	si-static ac	tion	•				
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,032	0,034	0,037	0,039	0,042	0,046	
80°C/50°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,042	0,044	0,047	0,051	0,054	0,060	
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]	0,034	0,035	0,038	0,041	0,044	0,048	
120°C/72°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,044	0,045	0,049	0,053	0,056	0,062	
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,126	0,131	0,142	0,153	0,163	0,179	
160°C/100°C	δ _{N∞} -factor	[mm/(N/mm²)]	0,129	0,135	0,146	0,157	0,168	0,184	
Cracked concrete	C20/25 under s	static and quasi-sta	tic action						
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,083	0,085	0,090	0,095	0,099	0,106	
80°C/50°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,170	0,110	0,116	0,122	0,128	0,137	
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]	0,086	0,088	0,093	0,098	0,103	0,110	
120°C/72°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,111	0,114	0,121	0,127	0,133	0,143	
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,321	0,330	0,349	0,367	0,385	0,412	
160°C/100°Č	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,330	0,340	0,358	0,377	0,396	0,424	

¹⁾ Calculation of the displacement

 $\delta_{N0} = \delta_{N0}\text{-factor} \ \cdot \ \tau;$

τ: action bond stress for tension

 $\delta_{N\infty} = \delta_{N\infty} \text{-factor} \quad \tau;$

Table C13: Displacements under shear load¹⁾ (Internal threaded rod)

Anchor size Inte	IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20				
Non-cracked and cracked concrete C20/25 under static and quasi-static action										
All temperature	δ _{v0} -factor	[mm/(kN)]	0,07	0,06	0,06	0,05	0,04	0,04		
ranges	$\delta_{V_{\infty}}$ -factor	[mm/(kN)]	0,10	0,09	0,08	0,08	0,06	0,06		

¹⁾ Calculation of the displacement

 $\delta_{V0} = \delta_{V0}\text{-factor} \cdot V;$ V: action shear load

 $\delta_{V_{\infty}} = \delta_{V_{\infty}}\text{-factor} \quad V;$

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