



Approval body for construction products and types of construction

**Bautechnisches Prüfamt** 

An institution established by the Federal and Laender Governments



# **European Technical Assessment**

ETA-18/0274 of 12 July 2018

English translation prepared by DIBt - Original version in German language

#### **General Part**

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

Deutsches Institut für Bautechnik

Mapei Injection system Mapefix UM Hybrid for rebar connection

Systems for post-installed rebar connections with mortar

Mapei S.p.A. Via Cafiero, 22 20158 MILANO (Italy) ITALIEN

Mapei S.p.A., Plant1 Germany

21 pages including 3 annexes which form an integral part of this assessment

EAD 330087-00-0601



## **European Technical Assessment ETA-18/0274**

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#### **Specific Part**

#### 1 Technical description of the product

The subject of this European Technical Assessment is the post-installed connection, by anchoring or overlap connection joint, of reinforcing bars (rebars) in existing structures made of normal weight concrete, using the "Mapei Injection system Mapefix UM Hybrid for rebar connection" in accordance with the regulations for reinforced concrete construction.

Reinforcing bars made of steel with a diameter  $\phi$  from 8 to 32 mm or the tension anchor ZA from sizes M12 to M24 according to Annex A and injection mortar Mapefix UM Hybrid are used for rebar connections. The rebar is placed into a drilled hole filled with injection mortar and is anchored via the bond between rebar, injection mortar and concrete.

The product description is given in Annex A.

## 2 Specification of the intended use in accordance with the applicable European assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the rebar connection of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

### 3 Performance of the product and references to the methods used for its assessment

#### 3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance under static and quasi-static loading	See Annex C 1

#### 3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Class A1
Resistance to fire	See Annex C 2 and C 3

## Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with European Assessment Document EAD No. 330087-00-0601, the applicable European legal act is: [96/582/EC].

The system(s) to be applied is (are): 1





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5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

Issued in Berlin on 12 July 2018 by Deutsches Institut für Bautechnik

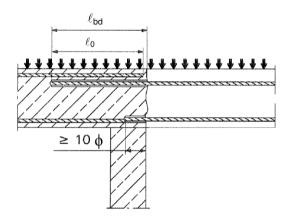
BD Dipl.-Ing. Andreas Kummerow Head of Department

beglaubigt: Baderschneider

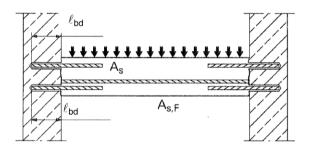


## Installation post installed rebar

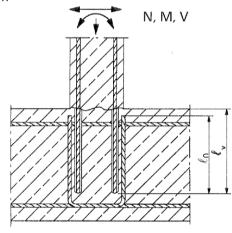
Figure A1: Overlapping joint for rebar connections of slabs and beams



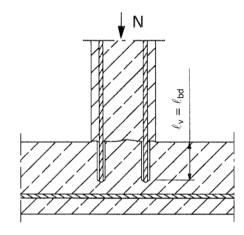
**Figure A3:** End anchoring of slabs or beams (e.g. designed as simply supported)



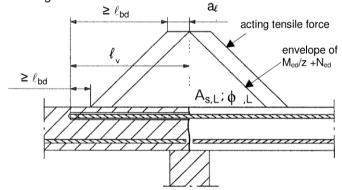
**Figure A2:** Overlapping joint at a foundation of a wall or column where the rebars are stressed in tension



**Figure A4:** Rebar connection for components stressed primarily in compression. The rebars sre stressed in compression



**Figure A5:** Anchoring of reinforcement to cover the line of acting tensile force



#### Note to Figure A1 to A5:

In the Figures no transverse reinforcement is plotted, the transverse reinforcement shall comply with EN 1992-1-1:2004+AC:2010.

Preparing of joints according to Annex B 2

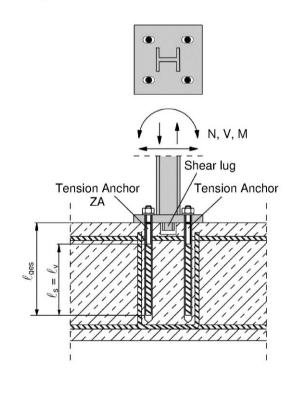
Mapei Injection system Mapefix UM Hybrid for rebar connection	
Product description Installed condition and examples of use for rebars	Annex A 1

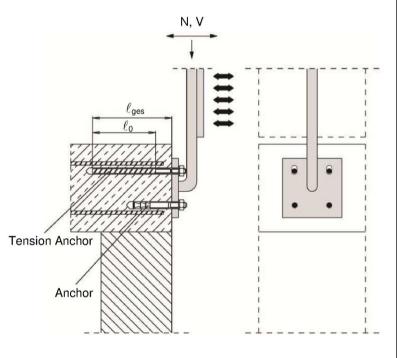


#### Installation tension anchor ZA

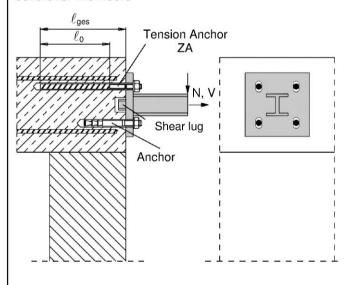
**Figure A6:** Overlapping joint of a column stressed in bending to a foundation

**Figure A7:** Overlap joint for the anchorage of barrier posts





**Figure A8:** Overlap joint for the anchorage to centilever members



## Note to Figure A6 to A8:

In the Figures no transverse reinforcement is plotted, the transverse reinforcement shall comply with EN 1992-1-1:2002+AC:2010

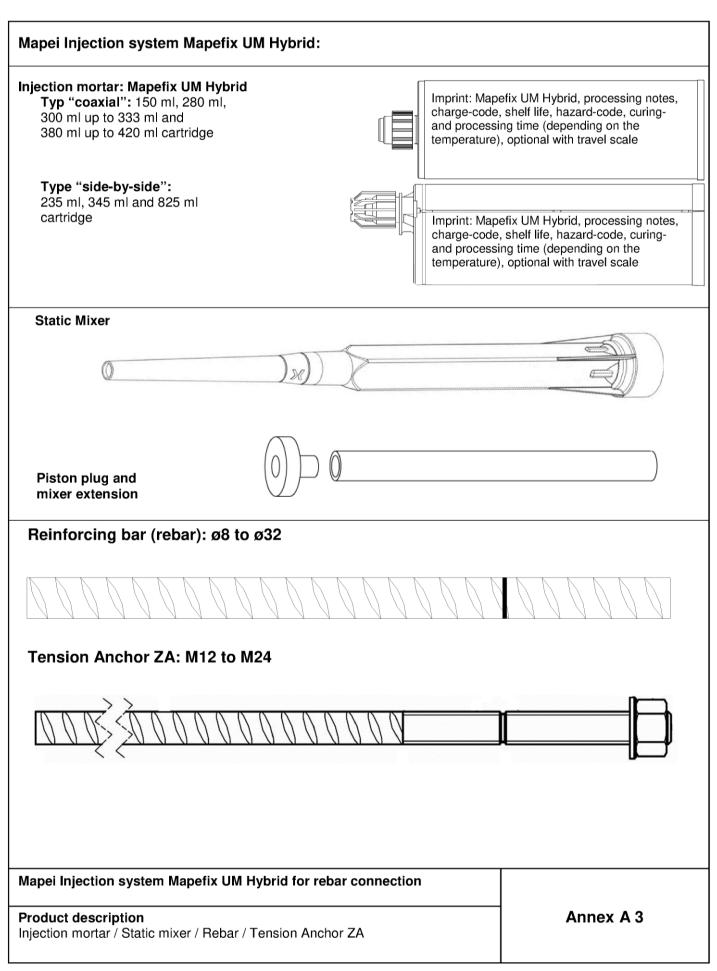
Mapei Injection system Mapefix UM Hybrid for rebar connection
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## **Product description**

Installed condition and examples of use for tension anchors ZA

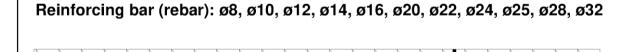
Annex A 2





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- Minimum value of related rip area f<sub>R,min</sub> according to EN 1992-1-1:2004+AC:2010
- Rib height of the bar shall be in the range 0,05φ ≤ h ≤ 0,07φ
   (φ: Nominal diameter of the bar; h: Rip height of the bar)

### Table A1: Materials

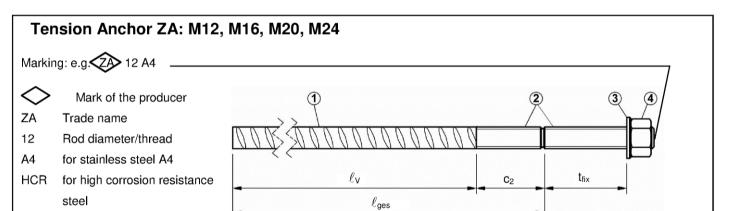
Designation	Material
Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C $f_{yk}$ and k according to NDP or NCL of EN 1992-1-1/NA $f_{uk} = f_{tk} = k \cdot f_{yk}$

Mapei Injection system Mapefix UM Hybrid for rebar connection

Product description
Specifications Rebar

Annex A 4





### Table A2: Materials

							Mate	erial					
Part	Designation	ZA vz				ZA A4				ZA HCR			
	3	M12	M16	M20	M24	M12	M16	M20	M24	M12	M16	M20	M24
1	1 Reinforcement bar Class B ac				according to NDP or NCL of EN 1992-1-1/NA								
<u> </u>	nemorcement bar	$f_{uk} = f_{tk}$	$f_{uk} = f_{tk} = k \cdot f_{vk}$										
	Threaded		zinc pla		ording	Stainless steel, 1.4362,			High corrosion resistant				
_	rod	to EN 10087:1998 or				1.4401, 1.4404, 1.4571,			steel, 1.4529, 1.4565,				
2		EN 102	263:200	)1		EN 10088-1:2014				EN 10088-1:2014			
	f <sub>yk</sub> [N/mm²]		64	40			640		560	0 640			560
3	Washer	Steel, zinc plated according			Stainless steel, 1.4362,			2,	High corrosion resistant				
		to EN 10087:1998 or			1.4401, 1.4404, 1.4571,			steel, 1.4529, 1.4565,					
4	Nut	EN 10263:2001			EN 10088-1:2014			EN 10088-1:2014					

## Table A3: Dimensions and installation parameter

Size			ZA-M12	ZA-M16	ZA-M20	ZA-M24	
Diameter of thread	led rod		[mm]	12	16	20	24
Diameter of reinfor	cement bar		[mm]	12	16	20	25
Drill hole diameter [mm]		16	20	25	32		
Diameter of cleara	nce hole in fixture		[mm]	14	18	22	26
With across nut fla	ts	sw	[mm]	19	24	30	36
Stress area		As	[mm <sup>2</sup> ]	84	157	245	353
Effective embedme	ent depth	ℓ <sub>v</sub> [mm]		according to static calculation			
Length of bonded	plated		[mm]	≥ 20	≥ 20	≥ 20	≥ 20
thread	A4/HCR	C <sub>2</sub>	[mm]	≥ 100	≥ 100	≥ 100	≥ 100
Minimum thickness	s of fixture	t <sub>fix</sub>	[mm]	5	5	5	5
Maximum thicknes	s of fixture	t <sub>fix</sub>	[mm]	3000	3000	3000	3000
Maximum installati	on torque	T <sub>max</sub>	[Nm]	50	100	150	150

Mapei Injection system Mapefix UM Hybrid for rebar connection	
Product description Specifications Tension Anchor ZA	Annex A 5

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## Specifications of intended use

#### Anchorages subject to:

- · Static and quasi-static loads.
- · Fire exposure

#### Base materials:

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000.
- Strength classes C12/15 to C50/60 according to EN 206-1:2000.
- Maximum chloride concrete of 0,40% (CL 0.40) related to the cement content according to EN 206-1:2000.
- Non-carbonated concrete.

Note: In case of a carbonated surface of the existing concrete structure the carbonated layer shall be removed in the area of the post-installed rebar connection with a diameter of  $\phi$  + 60 mm prior to the installation of the new rebar.

The depth of concrete to be removed shall correspond to at least the minimum concrete cover in accordance with EN 1992-1-1:2004+AC:2010.

The foregoing may be neglected if building components are new and not carbonated and if building components are in dry conditions.

#### **Temperature Range:**

• - 40°C to +80°C (max. short term temperature +80°C and max long term temperature +50°C).

#### Use conditions (Environmental conditions):

- Structures subject to dry internal conditions or subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist
  - (stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist (high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

#### Design:

- · Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Verifiable calculation notes and drawings are prepared taking account of the forces to be transmitted.
- Design according to EN 1992-1-1:2004+AC:2010, EN 1992-1-2:2004+AC:2008 and Annex B 2 and B 3.
- The actual position of the reinforcement in the existing structure shall be determined on the basis of the construction documentation and taken into account when designing.

#### Installation:

- · Dry or wet concrete.
- · It must not be installed in flooded holes.
- · Hole drilling by hammer drill (HD) or compressed air drill mode (CD).
- The installation of post-installed rebar resp. tension anchors shall be done only by suitable trained installer and under supervision on site; the conditions under which an installer may be considered as suitable trained and the conditions for supervision on site are up to the Member States in which the installation is done.
- Check the position of the existing rebars (if the position of existing rebars is not known, it shall be determined using a rebar detector suitable for this purpose as well as on the basis of the construction documentation and then marked on the building component for the overlap joint).

Mapei Injection system Mapefix UM Hybrid for rebar connection	
Intended use Specifications	Annex B 1

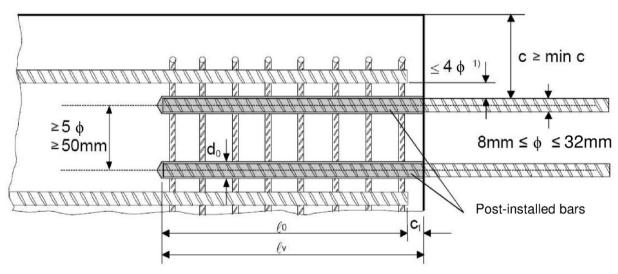
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### Figure B1: General construction rules for post-installed rebars

- · Only tension forces in the axis of the rebar may be transmitted
- The transfer of shear forces between new concrete and existing structure shall be designed additionally according to EN 1992-1-1:2004+AC:2010.
- The joints for concreting must be roughened to at least such an extent that aggregate protrude.



If the clear distance between lapped bars exceeds 4φ, then the lap length shall be increased by the difference between the clear bar distance and 4φ.

#### The following applies to Figure B1:

c concrete cover of post-installed rebar concrete cover at end-face of existing rebar

min c minimum concrete cover according to Table B1 and to EN 1992-1-1:2004+AC:2010, Section 4.4.1.2

diameter of post-installed rebar

 $\ell_0$  lap length, according to EN 1992-1-1:2004+AC:2010, Section 8.7.3

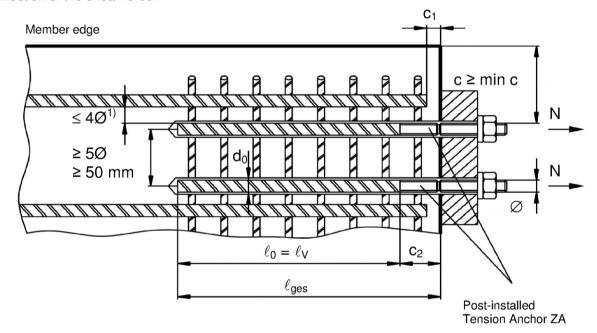
 $\begin{array}{ll} \ell_v & \text{effective embedment depth,} \geq \ell_0 + c_1 \\ d_0 & \text{nominal drill bit diameter, see Annex B 6} \end{array}$ 

Mapei Injection system Mapefix UM Hybrid for rebar connection	
Intended use General construction rules for post-installed rebars	Annex B 2



## Figure B2: General construction rules for tension anchors ZA

- · The length of the bonded-in thread may be not be accounted as anchorage
- · Only tension forces in the direction of the bar axis may be transmitted by the tension anchor ZA
- The tension force must be transferred via an overlap joint to the reinforcement in the building part.
- The transfer of shear forces shall be ensured by appropriate additional measures, e.g shear lugs or by anchors with an European technical assessment.
- In the anchor plate, the holes for the tension anchors shall be executed as elongated holes with axis in the direction of the shear force.



1) If the clear distance between lapped bars exceeds 4φ, then the lap length shall be increased by the difference between the clear bar distance and 4φ.

The following applies to Figure B2:

c concrete cover of tension anchor ZA

c<sub>1</sub> concrete cover at end-face of existing rebar

c<sub>2</sub> Length of bonded thread

min c minimum concrete cover according to Table B1 and to EN 1992-1-1:2004+AC:2010, Section 4.4.1.2

φ diameter of tension anchor

 $\ell_0$  lap length, according to EN 1992-1-1:2004+AC:2010, Section 8.7.3

 $\begin{array}{ll} \ell_v & \text{effective embedment depth,} \geq \ell_0 + c_1 \\ \ell_{ges} & \text{overall embedment depth,} \geq \ell_0 + c_2 \end{array}$ 

d<sub>0</sub> nominal drill bit diameter, see Annex B 6

Mapei Injection system Mapefix UM Hybrid for rebar connection	
Intended use General construction rules for tension anchors	Annex B 3



Table B1: Minimum concrete cover min c<sup>1)</sup> of post-installed rebar depending of drilling method

6/19/19/19/19/19/19/19/19/19/19/19/19/19/	Drilling aid
	<b>□</b>
11/1/1/11/11	1

Drilling method	Rebar diameter	Without drilling aid	With drilling aid
Hammer drilling (HD)	< 25 mm	30 mm + 0,06 · $\ell_{\rm v}$ ≥ 2 $\phi$	30 mm + 0,02 · $\ell_{v}$ ≥ 2 $\phi$
Hammer drilling (HD)	≥ 25 mm	40 mm + 0,06 · $\ell_{\rm v}$ ≥ 2 $\phi$	40 mm + 0,02 · $\ell_{v}$ ≥ 2 $\phi$
Compressed air drilling (CD)	< 25 mm	50 mm + 0,08 · ℓ <sub>v</sub>	50 mm + 0,02 · ℓ <sub>v</sub>
Compressed air drilling (CD)	≥ 25 mm	60 mm + 0,08 · ℓ <sub>v</sub>	60 mm + 0,02 · ℓ <sub>v</sub>

see Annex B2, Figures B1 and Annex B3, Figure B2 Comments: The minimum concrete cover acc. EN 1992-1-1:2004+AC:2010 must be observed

Table B2: maximum embedment depth  $\ell_{v,max}$ 

Rebar	Tension anchor	0
ф	ф	$\ell_{v,max}$ [mm]
8 mm		1000
10 mm		1000
12 mm	M12	1200
14 mm		1400
16 mm	M16	1600
20 mm	M20	2000
22 mm		2000
24 mm		2000
25 mm	M24	2000
28 mm		2000
32 mm		2000

Table B3: Base material temperature, gelling time and curing time

Concrete	tem	perature	Gelling working time <sup>1)</sup>	Minimum curing time in dry concrete	Minimum curing time in wet concrete
- 5 °C	to	- 1 °C	50 min	5 h	10 h
0 °C	to	+ 4 °C	25 min	3,5 h	7 h
+ 5 °C	to	+ 9 °C	15 min	2 h	4 h
+ 10 °C	to	+ 14 °C	10 min	1 h	2 h
+ 15 °C	to	+ 19 °C	6 min	40 min	60 min
+ 20 °C	to	+ 29 °C	3 min	30 min	60 min
+ 30 °C	to	+ 40 °C	2 min	30 min	60 min
Cartridge	temp	oerature	+5°C to +40°C		

<sup>1)</sup> t<sub>gel</sub>: maximum time from starting of mortar injection to completing of rebar setting.

Mapei Injection system Mapefix UM Hybrid for rebar connection	
Intended use Minimum concrete cover Maximum embedment depth / working time and curing times	Annex B 4





Table B4: Dispensing tools

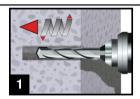
Cartridge type/size	Har	Pneumatic tool	
Coaxial cartridges 150, 280, 300 up to 333 ml			
	e.g. Type ⊦	297 or H244C	e.g. Type TS 492 X
Coaxial cartridges 380 up to 420 ml			
	e.g. Type CCM 380/10	e.g. Type H 285 or H244C	e.g. Type TS 485 LX
Side-by-side cartridges 235, 345 ml	a a Tura CDM 2004		
	e.g. Type CBM 330A	e.g. Type H 260	e.g. Type TS 477 LX
Side-by-side cartridge 825 ml	-	-	e.g. Type TS 498X

All cartridges could also be extruded by a battery tool.

Mapei Injection system Mapefix UM Hybrid for rebar connection	
Intended Use Dispensing tools	Annex B 5

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## A) Bore hole drilling



11 Drill a hole into the base material to the size and embedment depth required by the selected reinforcing bar with carbide hammer drill (HD) or a compressed air drill (CD). In case of aborted drill hole: the drill hole shall be filled with mortar.





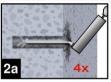
Hammer drill (HD)

Compressed air drill (CD)

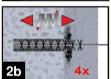
Rebar - ф	ZA- φ	Drill - Ø [mm]
8 mm		12
10 mm		14
12 mm	M12	16
14 mm		18
16 mm	M16	20
20 mm	M20	25
22 mm		28
24 mm		32
25 mm	M24	32
28 mm		35
32 mm		40

### B) Bore hole cleaning

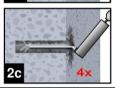
### MAC: Cleaning for bore hole diameter $d_0 \le 20$ mm and bore hole depth $h_0 \le 10d_s$



2a. Starting from the bottom or back of the bore hole, blow the hole clean a hand pump (Annex B 7) a minimum of four times.

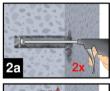


2b. Check brush diameter (Table B5). Brush the hole with an appropriate sized wire brush > d<sub>b,min</sub> (Table B5) a minimum of four times in a twisting motion.
If the bore hole ground is not reached with the brush, a brush extension shall be used.

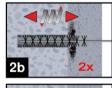


2c. Finally blow the hole clean again with a hand pump (Annex B 7) a minimum of four times.

#### CAC: Cleaning for all bore hole diameter and bore hole depth



Starting from the bottom or back of the bore hole, blow the hole clean with compressed air (min. 6 bar) (Annex B 7) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension shall be used.



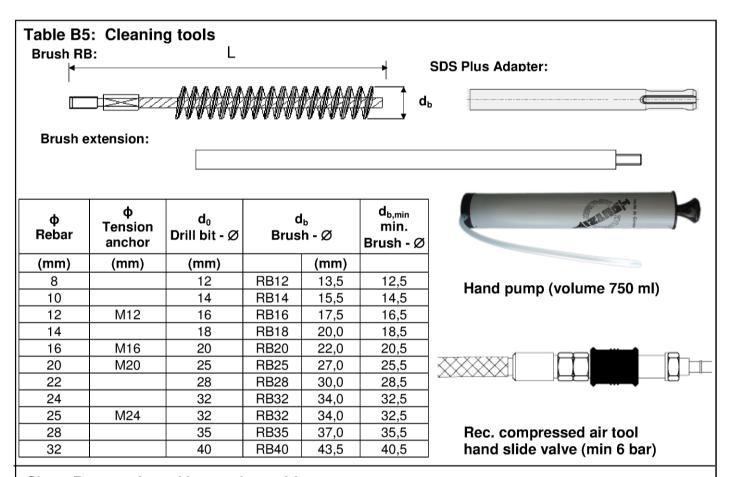
2b. Check brush diameter (Table B5). Brush the hole with an appropriate sized wire brush > d<sub>b,min</sub> (Table B5) a minimum of two times. If the bore hole ground is not reached with the brush, a brush extension shall be used (Table B5).



Finally blow the hole clean again with compressed air (min. 6 bar) (Annex B 7) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension shall be used.

Mapei Injection system Mapefix UM Hybrid for rebar connection	
Intended Use Installation instruction: Bore hole drilling and Bore hole cleaning	Annex B 6



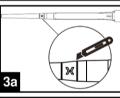


## C) Preparation of bar and cartridge

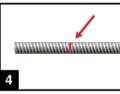


3. Attach the supplied static-mixing nozzle to the cartridge and load the cartridge into the correct dispensing tool.

For every working interruption longer than the recommended working time (Table B3) as well as for every new cartridges, a new static-mixer shall be used.

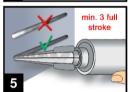


3a. In case of using the mixer extension VL16/1,8, the tip of the mixer nozzle has to be cut off at position "X".



4. Prior to inserting the reinforcing bar into the filled bore hole, the position of the embedment depth shall be marked (e.g. with tape) on the reinforcing bar and insert bar in empty hole to verify hole and depth  $\ell_{\rm v}$ .

The reinforcing bar should be free of dirt, grease, oil or other foreign material.

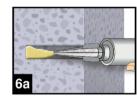


5. Prior to dispensing into the anchor hole, squeeze out separately the mortar until it shows a consistent grey colour, but a minimum of three full strokes, and discard non-uniformly mixed adhesive components.

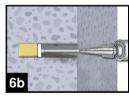
Mapei Injection system Mapefix UM Hybrid for rebar connection	
Intended Use Installation instruction: Cleaning tools and Preparation of bar and cartridge	Annex B 7



## D) Filling the bore hole



6. Starting from the bottom or back of the cleaned anchor hole fill the hole up to approximately two-thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid creating air pockets. For embedment larger than 190 mm an extension nozzle shall be used.

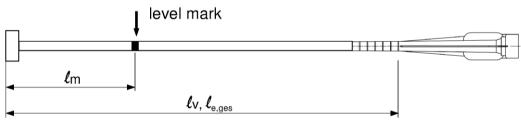


For overhead and horizontal installation and bore holes deeper than 240 mm a piston plug and the appropriate mixer extension must be used.

Observe the gel-/ working times given in Table B3.

Table B6: Piston plugs, max anchorage depth and mixer extension

	Tension		rill			Cartridge: All sizes		Cartridge: side-by-side (825 ml)				
Bar size	anchor	bit	- Ø	Piston   plug	Piston Hand or battery tool Pneumatic tool Pneumati		atic tool					
ф	ф	HD	CD	piag	$I_{v,max}$	Mixer extension	I <sub>v,max</sub>	Mixer extension	$I_{v,max}$	Mixer extension		
[mm]	[mm]	[m	m]		[cm]		[cm]		[cm]			
8		12	-	-			80		80	\/I 10/0.75		
10		14	-	VS14		100			]	100	VL 10/0,75	
12	M12	1	6	VS16	70		100		120			
14		1	8	VS18				140				
16	M16	2	0	VS20					160			
20	M20	25	26	VS25		VL 10/0,75	70,75 70	70	70	VL 10/0,75		
22		2	8	VS28				70	300 V	VL 16/1,8		
24		3	2	VS32	<b>5</b> 0			]	200			
25	M24	3	2	VS32	50			50				
28		3	5	VS35					200			
32		4	0	VS40				200				



Injection tool must be marked by mortar level mark  $\ell_{\rm m}$  and anchorage depth  $\ell_{\rm v}$  resp.  $\ell_{\rm e.ges}$  with tape or marker.

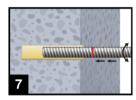
Quick estimation:  $\ell_m = 1/3 \cdot \ell_v$ 

Continue injection until the mortar level mark  $\ell_{\rm m}$  becomes visible.

Optimum mortar volume:  $\ell_{\rm m} = \ell_{\rm v} \text{ resp. } \ell_{\rm e,ges} \cdot \left(1,2 \cdot \frac{\phi^2}{d_0^2} - 0,2\right) \text{ [mm]}$ 

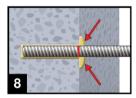
Mapei Injection system Mapefix UM Hybrid for rebar connection	
Intended Use Installation instruction: Filling the bore hole	Annex B 8

## E) Inserting the rebar

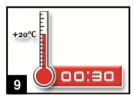


7. Push the reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached.

The bar should be free of dirt, grease, oil or other foreign material.



8. Be sure that the bar is inserted in the bore hole until the embedment mark is at the concrete surface and that excess mortar is visible at the top of the hole. If these requirements are not maintained, the application has to be renewed. For overhead installation fix embedded part (e.g. wedges).



9. Observe gelling time  $t_{\rm gel}$ . Attend that the gelling time can vary according to the base material temperature (see Table B3). It is not allowed to move the bar after geling time  $t_{\rm gel}$  has elapsed.

Allow the adhesive to cure to the specified time prior to applying any load. Do not move or load the bar until it is fully cured (attend Table B3). After full curing time  $t_{\text{cure}}$  has elapsed, the add-on part can be installed.

Mapei Injection system Mapefix UM Hybrid for rebar connection

Intended Use

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Installation instruction: Inserting rebar

Annex B 9



## Minimum anchorage length and minimum lap length

The minimum anchorage length  $\ell_{b,min}$  and the minimum lap length  $\ell_{0,min}$  according to EN 1992-1-1:2004+AC:2010 ( $\ell_{b,min}$  acc. to Eq. 8.6 and Eq. 8.7 and  $\ell_{0,min}$  acc. to Eq. 8.11) shall be multiply by the amplification factor  $\alpha_{lb}$  according to Table C1.

## Table C1: Amplification factor $\alpha_{lb}$ related to concrete class and drilling method

Concrete class	Drilling method	Bar size	Amplification factor α <sub>lb</sub>
C12/15 to C50/60	Hammer drilling and compressed air drilling	8 mm to 32 mm ZA-M12 to ZA-M24	1,0

### Table C2: Reduction factor kb for all drilling methods

Rebar - Ø	Concrete class								
ф	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
8 to 32 mm ZA-M12 to ZA-M24					1,0				

## Table C3: Design values of the ultimate bond stress f<sub>bd,PIR</sub> in N/mm<sup>2</sup> for all drilling methods and for good conditions

 $f_{bd,PIR} = k_b \cdot f_{bd}$ 

with

 $f_{bd}$ : Design value of the ultimate bond stress in N/mm² considering the concrete classes and the rebar diameter according to EN 1992-1-1:2004+AC:2010.

(for all other bond conditions multiply the values by 0.7)

k<sub>b</sub>: Reduction factor according to Table C2

Rebar - Ø	Concrete class								
ф	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
8 to 32 mm ZA-M12 to ZA-M24	1,6	2,0	2,3	2,7	3,0	3,4	3,7	4,0	4,3

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Performances	Annex C 1
Amplification factor $\alpha_{lb}$ , Reduction factor	
Design values of ultimate bond resistance f <sub>bd,PIR</sub>	

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## Design value of the ultimate bond stress $f_{bd,fi}$ under fire exposure for concrete classes C12/15 to C50/60, (all drilling methods):

The design value of the bond stress f<sub>bd,fi</sub> under fire exposure has to be calculated by the following equation:

$$f_{bd,fi} = k_{fi}(\theta) \cdot f_{bd,PIR} \cdot \gamma_c / \gamma_{M,fi}$$

with:  $\theta \le 364^{\circ}C$ :  $k_{fi}(\theta) = 30,34 \cdot e^{(\theta \cdot \cdot \cdot 0,011)} / (f_{bd,PIR} \cdot 4,3) \le 1,0$ 

 $\theta > 364$ °C:  $k_{fi}(\theta) = 0$ 

f<sub>bd,fi</sub> Design value of the ultimate bond stress in case of fire in N/mm<sup>2</sup>

θ Temperature in °C in the mortar layer.

 $k_{fi}(\theta)$  Reduction factor under fire exposure.

f<sub>bd,PIR</sub> Design value of the ultimate bond stress in N/mm² in cold condition according to Table C3

considering the concrete classes, the rebar diameter and the bond conditions according to

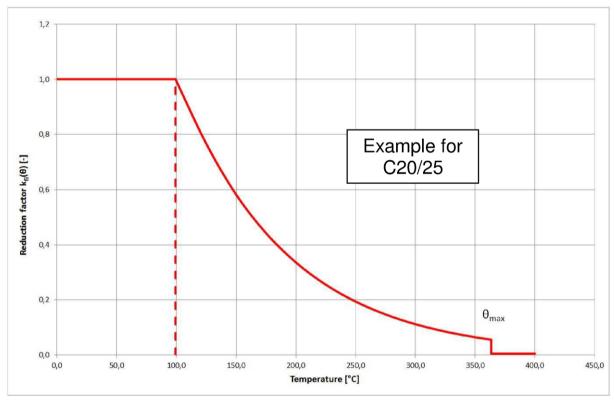
EN 1992-1-1:2004+AC:2010.

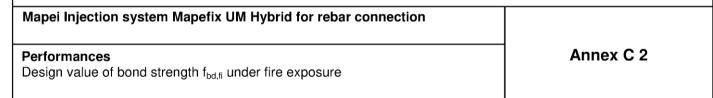
 $\gamma_c$  partially safety factor according to EN 1992-1-1:2004+AC:2010

 $\gamma_{M,fi}$  partially safety factor according to EN 1992-1-2:2004+AC:2008

For evidence under fire exposure the anchorage length shall be calculated according to EN 1992-1-1:2004+AC:2010 Equation 8.3 using the temperature-dependent ultimate bond stress f<sub>bd.fi</sub>.

## Example graph of Reduction factor $k_{fi}(\theta)$ for concrete classes C20/25 for good bond conditions:





English translation prepared by DIBt



## Table C4: Characteristic tension strength for tension anchor ZA under fire exposure,

concrete classes C12/15 to C50/60, according to Technical Report TR 020

Tension Anchor				M12	M16	M20	M24		
Steel, zinc plated	(ZA vz)								
	R30	$oldsymbol{\sigma}_{Rk,s,fi}$	[N/mm²]	20					
Characteristic steel strength	R60			15					
	R90			13					
	R120				1	0			
Stainless Steel (Z.	A A4 or Z	A HCR)							
Characteristic steel strength	R30		[N1/mm 2]	30					
	R60			25					
	R90	$oldsymbol{\sigma}_{Rk,s,fi}$	[N/mm²]	20					
	R120				1	6			

## Design value of the steel strength $\sigma_{\text{Rd,s,fi}}$ under fire exposure

The design value of the steel strength  $\sigma_{\text{Rd,s,fi}}$  under fire exposure has to be calculated by the following equation:

$$\sigma_{\text{Rd,s,fi}} = \sigma_{\text{Rk,s,fi}} \, / \, \gamma_{\text{M,fi}}$$

with:

 $\sigma_{Rk,s,fi}$  characteristic steel strength according to Table C4

 $\gamma_{M,fi}$  partially safety factor according to EN 1992-1-2:2004+AC:2008

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Performances	Annex C 3
Design value of the steel strength $\sigma_{\scriptscriptstyle{Rd,s,fi}}$ for tension anchor ZA under fire	
exposure	