



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-18/0617 of 12 July 2018

English translation prepared by DIBt - Original version in German language

General Part

Deutsches Institut für Bautechnik Technical Assessment Body issuing the **European Technical Assessment:** Injection system ESSVE ONE or ESSVE ONE ICE Trade name of the construction product Product family Bonded fastener for use in concrete to which the construction product belongs **ESSVE** Produkter AB Manufacturer Esbogatan 14 164 74 KISTA SCHWEDEN ESSVE Plant No. 671 Manufacturing plant This European Technical Assessment 25 pages including 3 annexes which form an integral part contains of this assessment EAD 330499-00-0601 This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

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Page 2 of 25 | 12 July 2018

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Page 3 of 25 | 12 July 2018

European Technical Assessment ETA-18/0617 English translation prepared by DIBt

Specific Part

1 Technical description of the product

The "Injection system ESSVE ONE or ESSVE ONE ICE for concrete" is a bonded anchor consisting of a cartridge with injection mortar ESSVE ONE or ESSVE ONE ICE and a steel element. The steel element consist of a commercial threaded rod with washer and hexagon nut in the range of M8 to M30, reinforcing bar in the range of diameter \emptyset 8 to \emptyset 32 mm or internal threaded rod IG-M6 to IG-M20.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between metal part, injection mortar and concrete.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance to tension load	See Annex
(static and quasi-static loading)	C 1, C 2, C 4 and C 6
Characteristic resistance to shear load	See Annex
(static and quasi-static loading)	C 1, C 3, C 5 and C 7
Displacements	See Annex
(static and quasi-static loading)	C 8 to C 10
Characteristic resistance for seismic performance	See Annex
category C1	C 2, C 3, C 6 and C 7
Characteristic resistance and displacements for seismic performance category C2	No performance assessed

3.2 Hygiene, health and the environment (BWR 3)

Essential characteristic	Performance
Content, emission and/or release of dangerous substances	No performance assessed



European Technical Assessment ETA-18/0617

Page 4 of 25 | 12 July 2018

English translation prepared by DIBt

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with the European Assessment Document EAD 330499-00-0601 the applicable European legal act is: [96/582/EC].

The system to be applied is: 1

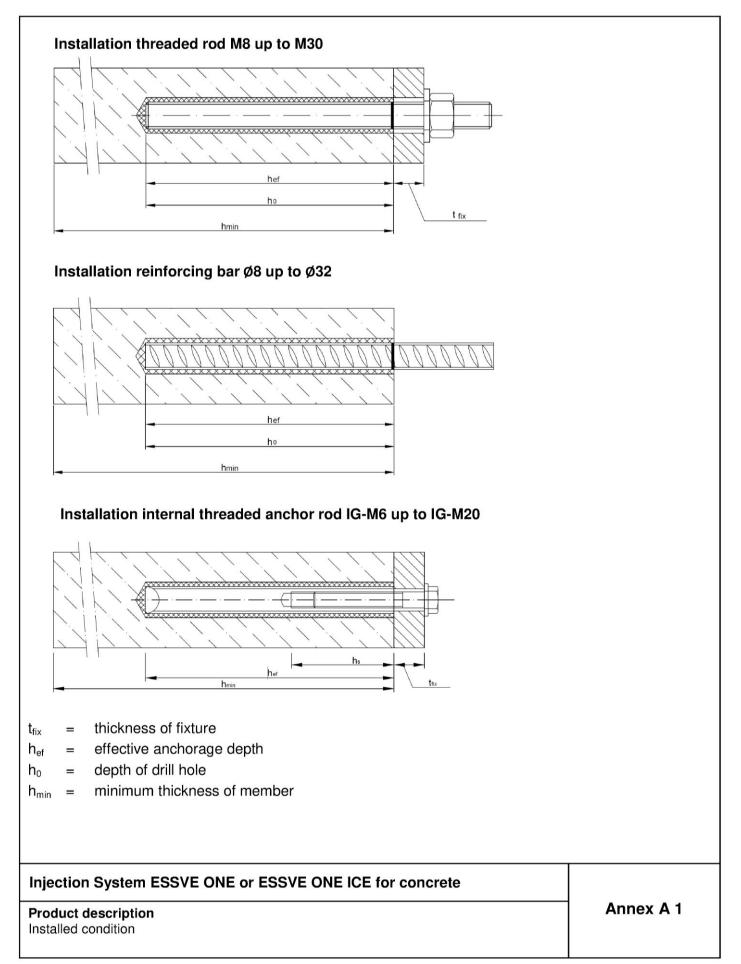
5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at Deutsches Institut für Bautechnik.

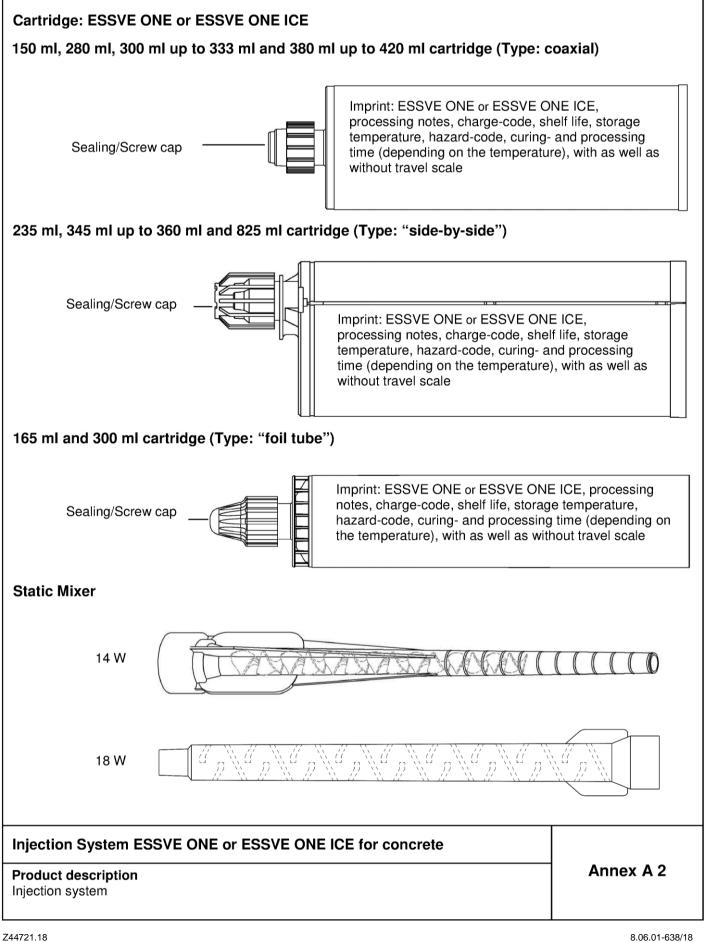
Issued in Berlin on 12 July 2018 by Deutsches Institut für Bautechnik

BD Dipl.-Ing. Andreas Kummerow Head of Department *beglaubigt:* Baderschneider

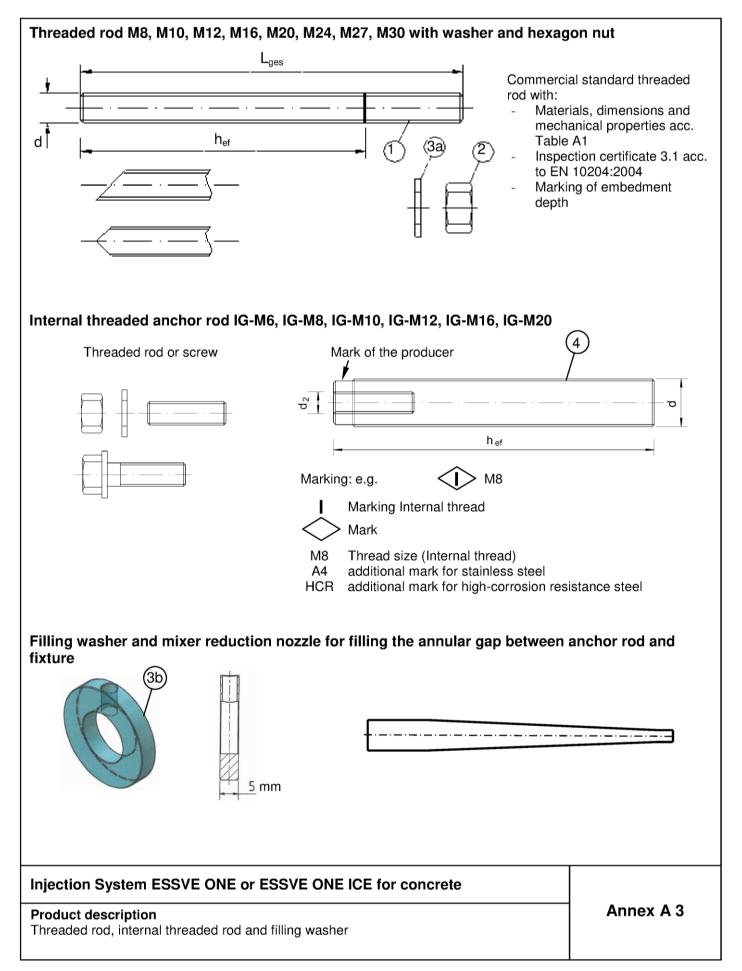














	Designation	Material			
tee	I, zinc plated (Steel acc. to EN 10		:2001)		
	plated \geq 5 µm acc. to EN ISO 4042:1				9 and
	SO 10684:2004+AC:2009 or sherard				
			4.6	f _{uk} =400 N/mm ² ; f _{yk} =240 N/mm ² ; A	$A_5 > 8\%$ fracture elongation
		Property class	4.8	f _{uk} =400 N/mm ² ; f _{yk} =320 N/mm ² ; A	$A_5 > 8\%$ fracture elongation
1	Anchor rod	acc. to	5.6	f _{uk} =500 N/mm ² ; f _{yk} =300 N/mm ² ; A	$A_5 > 8\%$ fracture elongation
		EN ISO 898-1:2013	5.8	f _{uk} =500 N/mm ² ; f _{yk} =400 N/mm ² ; A	
			8.8	f _{uk} =800 N/mm ² ; f _{vk} =640 N/mm ² ; A	
			4	for anchor rod class 4.6 or 4.8	
0		Property class		for anchor rod class 5.6 or 5.8	
2	Hexagon nut	acc. to EN ISO 898-2:2012	5 8		
		LN 100 030-2.2012	8	for anchor rod class 8.8	
3a 3b	Washer, (z.B.: EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 oder EN ISO 7094:2000)	Steel, zinc plated, hot-	dip gal [,]	vanised or sherardized	
30	Filling washer	Property class	5.8	f _{uk} =500 N/mm²; f _{yk} =400 N/mm²	; $A_5 > 8\%$ fracture elongation
4	Internal threaded anchor rod	acc. to		f _{uk} =800 N/mm²; f _{vk} =640 N/mm²	
		EN ISO 898-1:2013			
	nless steel A2 (Material 1.4301 / 1.	4303 / 1.4307 / 1.4567	oder 1	.4541, acc. to EN 10088-1:2014	4)
nd tair	nless steel A4 (Material 1.4401 / 1.	4404 / 1 4571 / 1 4362	or 1 44	578 acc to EN 10088-1.2014)	
lan			50	f _{uk} =500 N/mm ² ; f _{yk} =210 N/mm ² ; A	Ar > 8% fracture elongation
1	Anchor rod ¹⁾³⁾	Property class acc. to	70	f _{uk} =700 N/mm ² ; f _{yk} =450 N/mm ² ; A	
'		EN ISO 3506-1:2009	80	f _{uk} =800 N/mm ² ; f _{yk} =600 N/mm ² ; A	
			50	for anchor rod class 50	
2	Hexagon nut ¹⁾³⁾	Property class acc. to	70	for anchor rod class 70	
2		EN ISO 3506-1:2009	80	for anchor rod class 80	
3a 3b	Washer, (z.B.: EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 oder EN ISO 7094:2000) Filling washer ⁴⁾			/ 1.4307 / 1.4567 or 1.4541, EN / 1.4571 / 1.4362 or 1.4578, EN	
00		Property class	50	f _{uk} =500 N/mm ² ; f _{yk} =210 N/mm ²	$\Lambda_{-} > 0^{\circ}$ fracture elementic
4	Internal threaded anchor rod ¹⁾²⁾	acc. to			
		EN ISO 3506-1:2009	70	f _{uk} =700 N/mm ² ; f _{yk} =450 N/mm ²	; $A_5 > 8\%$ fracture elongation
igh	corrosion resistance steel (Mate	rial 1.4529 or 1.4565, a	acc. to	EN 10088-1: 2014)	
-		Property class	50	f _{uk} =500 N/mm ² ; f _{yk} =210 N/mm ² ; A	$A_5 > 8\%$ fracture elongation
1	Anchor rod ¹⁾	acc. to	70	f _{uk} =700 N/mm ² ; f _{yk} =450 N/mm ² ; A	$A_5 > 8\%$ fracture elongation
		EN ISO 3506-1:2009	80	f _{uk} =800 N/mm ² ; f _{yk} =600 N/mm ² ; A	$A_5 > 8\%$ fracture elongation
		Property class	50	for anchor rod class 50	
2	Hexagon nut ¹⁾	acc. to	70	for anchor rod class 70	
		EN ISO 3506-1:2009	80	for anchor rod class 80	
3a	Washer, (z.B.: EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 oder EN ISO 7094:2000)	Material 1.4529 or 1.4	565, ac	c. to EN 10088-1: 2014	
3b	Filling washer				
4	Internal threaded anchor rod ^{1) 2)}	Property class acc. to	50	f _{uk} =500 N/mm²; f _{yk} =210 N/mm²	-
		EN ISO 3506-1:2009	70	f _{uk} =700 N/mm ² ; f _{yk} =450 N/mm ²	; A ₅ > 8% fracture elongation
2). 3)	Property class 70 for anchor rods up to N for IG-M20 only property class 50 Property class 80 only for stainless steel	A4	anchor	rods up to IG-M16,	
-+)	Filling washer only with stainless steel A	4			
Inj	ection System ESSVE ONE	or ESSVE ONE IC	E for	concrete	



Reir	nforcing bar Ø 8, Ø 10, Ø 12, Ø 14, Ø 10	6, Ø 20, Ø 25, Ø 28, Ø 32	
	h _{ef}	J	
		•	
	 Minimum value of related rip area f_{R,min} ac Rib height of the bar shall be in the range 		
	(d: Nominal diameter of the bar; h: Rip hei		
Tab	le A2: Materials		
Part	Designation	Material	
Reinf	orcing bars	-	
1	Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C f_{yk} and k according to NDP or NCL of EN $f_{uk} = f_{tk} = k \cdot f_{yk}$	1992-1-1/NA
Inje	ction System ESSVE ONE or ESSVE O	NE ICE for concrete	
	luct description		Annex A 5
	rials reinforcing bar		



Specifications of intended use

Anchorages subject to:

- Static and quasi-static loads: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.
- Seismic action for Performance Category C1: M8 to M30 (except hot-dip galvanised rods), Rebar Ø8 to Ø32.

Base materials:

- · Reinforced or unreinforced normal weight concrete without fibres according to EN 206:2013.
- Strength classes C20/25 to C50/60 according to EN 206:2013.
- Non-cracked concrete: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.
- Cracked concrete: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.

Temperature Range:

- I: 40 °C to +40 °C (max long term temperature +24 °C and max short term temperature +40 °C)
- II: 40 °C to +80 °C (max long term temperature +50 °C and max short term temperature +80 °C)
- III: 40 °C to +120 °C (max long term temperature +72 °C and max short term temperature +120 °C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc coated steel, stainless steel A2 resp. A4 or high corrosion resistant steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel A4 or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist

(high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to supports, etc.).
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- The anchorages are designed in accordance to:
 FprEN 1992-4:2017 and Technical Report TR055

Installation:

- Dry or wet concrete: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.
- Flooded holes (not sea water): M8 to M16, Rebar Ø8 to Ø16, IG-M6 to IG-M10.
- Hole drilling by hammer (HD), hollow (HDB) or compressed air drill mode (CD).
- · Overhead installation allowed.
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.

Injection System ESSVE ONE or ESSVE ONE ICE for concrete

Intended Use Specifications Annex B 1

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Deutsches Institut DIBt für Bautechnik

Anchor size				М	8 M	10	м	12	M 16		M 20	M 2	24	M 27	M 30
Outer diameter of anchor		d _{nom} [mm] =	8	1	0		12	16	+	20	24		27	30
Nominal drill hole diameter		d ₀ [mm		1(D 1	2		14	18	\top	24	28	3	32	35
		h _{ef,min} [mm] =	60	<u>ς</u> σ	60	7	70	80	+	90	96	;	108	120
Effective embedment depth		h _{ef,max} [mm		16	0 2	00	2	40	320		400	48	0	540	600
Diameter of clearance hole in the fixture		d _f [mm		9	1	2		14	18		22	26	5	30	33
Diameter of steel brush		d _b [mm]≥	12	2 1	4		16	20		26	30)	34	37
Maximum torque moment		T _{inst} [Nm]≤	1() 2	20	40		80		120	16	0	180	200
Minimum thickness of memb	er	h _{min} [m	m]	h _{ef}	+ 30 mm ≥ 100 mm					ł	η _{ef} +	$2d_0$			
Minimum spacing		s _{min} [mm] 4		4(D 5	50	(60	80		100	12	0	135	150
Minimum edge distance		c _{min} [m	m]	4(D 5	50	(60	80		100	12	0	135	150
Rebar size Outer diameter of anchor	d	nom [mm] =	Ø 8		Ø 10 10	Ø 1 12		Ø 1 14			Ø 20 20	_	25	Ø 28 28	Ø 32
Table B2: Installation	<u>, </u>					a	10	<i>α</i> 1	1 0	16	<i>a</i> 00	a	05	<i>a</i> 00	an
Outer diameter of anchor	d	nom [mm] =	8	3	10	12	2	14	16	6	20	2	25	28	32
Nominal drill hole diameter		$d_0 [mm] =$	12	2	14	16	6	18	20)	24	3	32	35	40
Effective embedment depth		_{min} [mm] =	6		60	70		75			90	-	00	112	128
· .	h _{ef,}	_{max} [mm] =	16		200	24		280			400	-	00	580	640
Diameter of steel brush		d _b [mm] ≥	1.		16	18	3	20	22	2	26	3	34	37	41,5
Minimum thickness of member		h _{min} [mm]) mm						h _{ef} + 2c			I	
Minimum spacing		s _{min} [mm]	4		50	60		70			100	-	25	140	160
Minimum edge distance		c _{min} [mm]	4	0	50	60)	70	8)	100	1	25	140	160
Table B3: Installation	on pa	arameters	s fo	r in	ternal	thr	ead	ded	ancho	or r	od				
Size internal threaded anchor	rod				IG-M 6	; I	G-N	/ 8	IG-M ⁻¹	0	IG-M	12	IG-N	/ 16	IG-M 20
Internal diameter of anchor			mm		6		8		10		12		1	6	20
Outer diameter of anchor ¹⁾		d _{nom} [10		12		16		20		2		30
Nominal drill hole diameter			mm	· - +	12		14		18		22		2		35
Effective embedment depth		h _{ef,min} [h _{ef,max} [60 200		70 24		80 320		90 400		9 48		120 600
Diameter of clearance hole in the fixture			mm		200		24 9		12		14			8	22
Maximum torque moment		T _{inst}	[Nm ⁻	1 ≤	10		10)	20		40		6	0	100
Thread engagement length			mm		8/20		8/2		10/25	5	12/30	C		/32	20/40
min/max				-+	h _{ef} + 30 mm		h _{ef} + 2c			0.1					
	er	h _{mi}	_n [m	m]		00 m					r	l _{ef} +	$2a_0$		
min/max Minimum thickness of memb Minimum spacing Minimum edge distance	ber	S _{mi}	n [m] <u>n [</u> m] n [m]	m])	80 80		r 100 100		12	20	150 150

Injection System ESSVE ONE or ESSVE ONE ICE for concrete

Intended Use Installation parameters Annex B 2

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2			8		****					
Threaded Rod	Rebar	Internal threaded Anchor rod	d₀ Drill bit - Ø HD, HDB, CA	d Brusl		d _{b,min} min. Brush - Ø	Piston plug		n direction piston plu	
[mm]	[mm]	[mm]	[mm]		[mm]	[mm]		↓	\rightarrow	Î
M8			10	RBT10	12	10,5				
M10	8	IG-M6	12	RBT12	14	12,5	1	N and at	han an t	-1
M12	10	IG-M8	14	RBT14	16	14,5	1	No piston p	olug require	d
	12		16	RBT16		16,5	1			
M16	14	IG-M10	18	RBT18	20	18,5	VS18			
	16		20	RBT20	22	20,5	VS20	1		
M20	20	IG-M12	24	RBT24	26	24,5	V820	1		
M24	20	IG-M12	24	RBT28	30	24,5	VS24 VS28	h _{ef} >	h _{ef} >	all
M27	25	10-10110	32	RBT32	34	32,5	VS28 VS32	250 mm	250 mm	all
								4		
M30	28 32	IG-M20	35 40	RBT35 RBT40		35,5 40,5	VS35 VS40	-		
Drill bit dia Drill hole c						- Rec. com bit diameter (d			min 6 ba	r)
	G	0			⊐⊵ Sti	⊴~~∰		<i>.</i>	WW	∃ ↓ d⊾

Intended Use Cleaning and setting tools Annex B 3



Drilling of the bore	hole	
	1. Drill with hammer drill a hole into the base material to the size and required by the selected anchor (Table B1, B2, or B3), with hammor compressed air (CD) drilling. The use of a hollow drill bit is only sufficient vacuum permitted. In case of aborted drill hole: The drill hole shall be filled with mort	her (HD), hollow (HDB) in combination with a
	Attention! Standing water in the bore hole must be removed before	ore cleaning.
MAC: Cleaning for	bore hole diameter $d_0 \le 20$ mm and bore hole depth $h_0 \le 10d_{nom}$ (und	racked concrete only!
4x	 2a. Starting from the bottom or back of the bore hole, blow the hole cl (Annex B 3) a minimum of four times. 	ean by a hand pump ¹⁾
<u>***********</u> ***	 Check brush diameter (Table B4). Brush the hole with an appropr > d_{b,min} (Table B4) a minimum of four times in a twisting motion. If the bore hole ground is not reached with the brush, a brush exit 	
	2c. Finally blow the hole clean again with a hand pump (Annex B 3) a	minimum of four times.
4x	¹⁾ It is permitted to blow bore holes with diameter between 14 mm and 20 mm up to 10d _{nom} also in cracked concrete with hand-pump.	and an embedment depth
CAC: Cleaning for a	all bore hole diameter in uncracked and cracked concrete	
4x)	2a. Starting from the bottom or back of the bore hole, blow the hole c compressed air (min. 6 bar) (Annex B 3) a minimum of four times stream is free of noticeable dust. If the bore hole ground is not reaextension must be used.	until return air
<u>********</u> *** 4x	2b. Check brush diameter (Table B4). Brush the hole with an appropriate $d_{b,min}$ (Table B4) a minimum of four times in a twisting motion. If the bore hole ground is not reached with the brush, a brush extended with the brush and the brush extended with the brush and the brush a	
4x	2c. Finally blow the hole clean again with compressed air (min. 6 bar minimum of four times until return air stream is free of noticeable ground is not reached an extension must be used.	
	After cleaning, the bore hole has to be protected against re-co an appropriate way, until dispensing the mortar in the bore ho the cleaning has to be repeated directly before dispensing the In-flowing water must not contaminate the bore hole again.	ole. If necessary,
Injection System	ESSVE ONE or ESSVE ONE ICE for concrete	



Installation inst	ructions (continuation)	
	3. Attach the supplied static-mixing nozzle to the cartridge and load th correct dispensing tool. Cut off the foil tube clip before use. For every working interruption longer than the recommended work well as for new cartridges, a new static-mixer shall be used.	-
ter ere order order order porter order order ← hef →	Prior to inserting the anchor rod into the filled bore hole, the positio depth shall be marked on the anchor rods.	n of the embedment
min. 3 full stroke	5. Prior to dispensing into the anchor hole, squeeze out separately a r strokes and discard non-uniformly mixed adhesive components unt consistent grey colour. For foil tube cartridges it must be discarded strokes.	il the mortar shows a
	6. Starting from the bottom or back of the cleaned anchor hole, fill the approximately two-thirds with adhesive. Slowly withdraw the static r hole fills to avoid creating air pockets. If the bottom or back of the a reached, an appropriate extension nozzle must be used. Observe the given in Annex B 6.	nixing nozzle as the nchor hole is not
	 Piston plugs and mixer nozzle extensions shall be used according t following applications: Horizontal assembly (horizontal direction) and ground erection direction): Drill bit-Ø d₀ ≥ 18 mm and embedment depth h_{ef} > 2 Overhead assembly (vertical upwards direction): Drill bit-Ø d₀ ≥ 	(vertical downwards 50mm
	8. Push the threaded rod or reinforcing bar into the anchor hole while ensure positive distribution of the adhesive until the embedment de	
	The anchor shall be free of dirt, grease, oil or other foreign material	
	9. Be sure that the anchor is fully seated at the bottom of the hole and visible at the top of the hole. If these requirements are not maintain to be renewed. For overhead application the anchor rod shall be fixed application the anchor rod shall be fixed application.	ned, the application has
+20°C	10. Allow the adhesive to cure to the specified time prior to applying ar not move or load the anchor until it is fully cured (attend Annex B 6	
	11. After full curing, the add-on part can be installed with up to the max (Table B1 or B3) by using a calibrated torque wrench. It can be opt gap between anchor and fixture with mortar. Therefor substitute the washer and connect the mixer reduction nozzle to the tip of the mix filled with mortar, when mortar oozes out of the washer.	ional filled the annular e washer by the filling
Injection System	ESSVE ONE or ESSVE ONE ICE for concrete	
Intended Use		Annex B 5

Installation instructions (continuation)



- 20 °C to +29°C 6 min 45 min - 30 °C to +34°C 4 min 25 min - 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C -40°C	5 °C to +9°C 25 min 2 h 0 °C to +19°C 15 min 80 min 0 °C to +29°C 6 min 45 min 0 °C to +29°C 6 min 45 min 0 °C to +29°C 6 min 25 min 10 °C to +34°C 4 min 25 min 15 °C to +39°C 2 min 20 min +40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C +40°C wet concrete the curing time must be doubled. wet concrete the curing time must be doubled. ble B6: Maximum working time and minimum curing time ESSVE ONE ICE soncrete temperature Gelling- / working time Minimum curing time in dry concrete ') 0 °C to +4°C 10 min 2,5 h 5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min	+5 °C to +9°C 25 min 2 h 10 °C to +19°C 15 min 80 min 20 °C to +29°C 6 min 45 min 30 °C to +34°C 4 min 25 min 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C 15 min vet concrete the curing time must be doubled. wet concrete the curing time must be doubled. ble B6: Maximum working time and minimum curing time ESSVE ONE ICE Minimum curing time in dry concrete '' 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min +10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 60 Min 60 Min	$+5 \ ^{\circ}C$ to $+9 \ ^{\circ}C$ $25 \ ^{\circ}min$ $2 \ ^{\circ}h$ $+10 \ ^{\circ}C$ to $+19 \ ^{\circ}C$ $15 \ ^{\circ}min$ $80 \ ^{\circ}min$ $+20 \ ^{\circ}C$ to $+29 \ ^{\circ}C$ $6 \ ^{\circ}min$ $45 \ ^{\circ}min$ $+20 \ ^{\circ}C$ to $+29 \ ^{\circ}C$ $6 \ ^{\circ}min$ $45 \ ^{\circ}min$ $+30 \ ^{\circ}C$ to $+34 \ ^{\circ}C$ $4 \ ^{\circ}min$ $25 \ ^{\circ}min$ $+35 \ ^{\circ}C$ to $+39 \ ^{\circ}C$ $2 \ ^{\circ}min$ $20 \ ^{\circ}min$ $+40 \ ^{\circ}C$ $1,5 \ ^{\circ}min$ $15 \ ^{\circ}min$ $15 \ ^{\circ}min$ Cartridge temperature $+5 \ ^{\circ}C \ ^{\circ} +40 \ ^{\circ}C$ $10 \ ^{\circ}min$ In wet concrete the curing time must be doubled. $40 \ ^{\circ}C$ $10 \ ^{\circ}min$ Table B6:Maximum working time and minimum curing time $Minimum \ ^{\circ}min$ $ESSVE \ ONE \ ICE$ $Concrete \ temperature$ $Concrete \ ^{\circ}min$ $2,5 \ ^{\circ}h$ $0 \ ^{\circ}C$ $to \ +4^{\circ}C$ $10 \ ^{\circ}min$ $2,5 \ ^{\circ}h$ $+5 \ ^{\circ}C$ $to \ +9^{\circ}C$ $6 \ ^{\circ}min$ $80 \ ^{\circ}Min$ $+10 \ ^{\circ}C$ $6 \ ^{\circ}min$ $60 \ ^{\circ}Min$	Concrete temp	erature	Gelling- / working time	Minimum curing time in dry concrete ¹⁾
- 10 °C to +19°C 15 min 80 min - 20 °C to +29°C 6 min 45 min - 30 °C to +34°C 4 min 25 min - 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C -40°C	0 °C to +19°C 15 min 80 min 10 °C to +29°C 6 min 45 min 10 °C to +34°C 4 min 25 min 15 °C to +39°C 2 min 20 min 15 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C wet concrete the curing time must be doubled. +5°C to +40°C ble B6: Maximum working time and minimum curing time ESSVE ONE ICE Gelling- / working time Minimum curing time in dry concrete '' 0 °C to +4°C 10 min 2,5 h 5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 10°C	10 °C to +19°C 15 min 80 min 20 °C to +29°C 6 min 45 min 30 °C to +34°C 4 min 25 min 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 20 min - + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C	+ 10 °C to +19°C 15 min 80 min + 20 °C to +29°C 6 min 45 min + 30 °C to +34°C 4 min 25 min + 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C 15 min In wet concrete the curing time must be doubled. +5°C to +40°C In wet concrete the curing time must be doubled. Minimum curing time Concrete temperature Gelling- / working time Minimum curing time findry concrete 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C 10°C	0 °C to	+4°C	45 min	7 h
- 20 °C to +29°C 6 min 45 min - 30 °C to +34°C 4 min 25 min - 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C -40°C	$00 \circ C$ to $+29 \circ C$ 6 min 45 min $10 \circ C$ to $+34 \circ C$ 4 min 25 min $10 \circ C$ to $+34 \circ C$ 4 min 25 min $15 \circ C$ to $+39 \circ C$ 2 min 20 min $+40 \circ C$ 1,5 min 15 min 15 min Cartridge temperature $+5 \circ C$ to $+40 \circ C$ wet concrete the curing time must be doubled. ble B6: Maximum working time and minimum curing time ESSVE ONE ICE Minimum curing time in dry concrete " concrete temperature Gelling- / working time Minimum curing time in dry concrete " $0 \circ C$ to $+4^{\circ}C$ 10 min 2,5 h $5 \circ C$ to $+9^{\circ}C$ 6 min 80 Min $+10 \circ C$ 6 min 60 Min 60 Min Cartridge temperature $0^{\circ}C$ to $+10^{\circ}C$ $0^{\circ}C$ to $+10^{\circ}C$ $0^{\circ}C$ to $+10^{\circ}C$	20 °C to +29°C 6 min 45 min 30 °C to +34°C 4 min 25 min 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C +5°C to +40°C wet concrete the curing time must be doubled.	+ 20 °C to +29°C 6 min 45 min + 30 °C to +34°C 4 min 25 min + 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C 10 min 15 min In wet concrete the curing time must be doubled. +5°C to +40°C 10 min 10 min Concrete temperature Gelling- / working time Minimum curing time find ray concrete 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 6 min 60 Min	+5 °C to	+9°C	25 min	2 h
- 30 °C to +34°C 4 min 25 min - 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C -40°C	$00 \circ C$ to $+34 \circ C$ 4 min 25 min $15 \circ C$ to $+39 \circ C$ 2 min 20 min $+ 40 \circ C$ 1,5 min 15 min 15 min Cartridge temperature $+5^\circ C$ to $+40^\circ C$ wet concrete the curing time must be doubled. oble B6: Maximum working time and minimum curing time ESSVE ONE ICE Minimum curing time in dry concrete ") $0 \circ C$ to $+4^\circ C$ 10 min 2,5 h $5 \circ C$ to $+9^\circ C$ 6 min 80 Min $+10 \circ C$ 6 min 60 Min 60 Min Cartridge temperature $0^\circ C$ to $+10^\circ C$ 6 min 60 Min	30 °C to $+34^{\circ}$ C 4 min 25 min 35 °C to $+39^{\circ}$ C 2 min 20 min $+40 ^{\circ}$ C 1,5 min 15 min Cartridge temperature $+5^{\circ}$ C to $+40^{\circ}$ C wet concrete the curing time must be doubled. ble B6: Maximum working time and minimum curing time ESSVE ONE ICE Concrete temperature Gelling- / working time Minimum curing time in dry concrete 1) 0 °C to $+4^{\circ}$ C 10 min 2,5 h $+5 ^{\circ}$ C to $+9^{\circ}$ C 6 min 80 Min $+10 ^{\circ}$ C 6 min 60 Min 60 Min Cartridge temperature 0°C to $+10^{\circ}$ C 6 min 60 Min	+ 30 °C to +34°C 4 min 25 min + 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C 10 min 15 min In wet concrete the curing time must be doubled.	10 °C to	+19°C	15 min	80 min
- 35 °C to + 39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C 15 min	15 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C wet concrete the curing time must be doubled. ble B6: Maximum working time and minimum curing time ESSVE ONE ICE concrete temperature Gelling- / working time Minimum curing time in dry concrete ¹⁾ 0 °C to +4°C 10 min 2,5 h 5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C 0°C to +10°C	35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C +5°C to +40°C wet concrete the curing time must be doubled. wet concrete the curing time must be doubled. ble B6: Maximum working time and minimum curing time ESSVE ONE ICE Minimum curing time in dry concrete ¹) 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C	+ 35 °C to +39°C 2 min 20 min + 40 °C 1,5 min 15 min 15 min Cartridge temperature +5°C to +40°C +5°C to +40°C In wet concrete the curing time must be doubled. Fable B6: Maximum working time and minimum curing time ESSVE ONE ICE Concrete temperature Gelling- / working time Minimum curing time diverses 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C 0°C to +10°C	20 °C to	+29°C	6 min	45 min
+ 40 °C1,5 min15 minCartridge temperature+5°C to +40°C	+ 40 °C 1,5 min 15 min Cartridge temperature +5°C to +40°C wet concrete the curing time must be doubled. ole B6: Maximum working time and minimum curing time ESSVE ONE ICE concrete temperature Gelling- / working time 0 °C to +4°C 10 min 2,5 h 5 °C to +9°C 6 min 80 Min + 10 °C 6 min Cartridge temperature 0°C to +10°C	+ 40 °C 1,5 min 15 min Cartridge temperature +5°C to +40°C wet concrete the curing time must be doubled. ble B6: Maximum working time and minimum curing time ESSVE ONE ICE Concrete temperature Gelling- / working time Minimum curing time in dry concrete ¹⁾ 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min +10 °C 6 min 60 Min Cartridge temperature 0°C to +10°C	+ 40 °C 1,5 min 15 min Cartridge temperature +5°C to +40°C In wet concrete the curing time must be doubled. Fable B6: Maximum working time and minimum curing time ESSVE ONE ICE Concrete temperature Gelling- / working time 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min Cartridge temperature 0°C to +10°C	· 30 °C to	+34°C	4 min	25 min
Cartridge temperature +5°C to +40°C	Cartridge temperature +5°C to +40°C wet concrete the curing time must be doubled. ole B6: Maximum working time and minimum curing time ESSVE ONE ICE concrete temperature Gelling- / working time Minimum curing time in dry concrete 10 in dr	Cartridge temperature +5°C to +40°C wet concrete the curing time must be doubled. Ible B6: Maximum working time and minimum curing time ESSVE ONE ICE Concrete temperature Gelling- / working time in dry concrete 10 i	Cartridge temperature +5°C to +40°C In wet concrete the curing time must be doubled. Fable B6: Maximum working time and minimum curing time ESSVE ONE ICE Concrete temperature Gelling- / working time Minimum curing time time findry concrete 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C 0°C to +10°C	35 °C to	+39°C	2 min	20 min
	wet concrete the curing time must be doubled. ole B6: Maximum working time and minimum curing time ESSVE ONE ICE concrete temperature Gelling- / working time in dry concrete ¹⁾ 0 °C to +4°C 10 min 2,5 h 5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C	Wet concrete the curing time must be doubled. Ible B6: Maximum working time and minimum curing time ESSVE ONE ICE Concrete temperature Gelling- / working time in dry concrete 1) 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C	In wet concrete the curing time must be doubled. Table B6: Maximum working time and minimum curing time ESSVE ONE ICE Concrete temperature Gelling- / working time Minimum curing time in dry concrete 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 0°C to +10°C	+ 40 °C	;	1,5 min	15 min
In wet concrete the curing time must be doubled.	Dele B6: Maximum working time and minimum curing time ESSVE ONE ICE Moncrete temperature Gelling- / working time Minimum curing time in dry concrete 1) 0 °C to +4°C 10 min 2,5 h 5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C	Able B6: Maximum working time and minimum curing time ESSVE ONE ICE Concrete temperature Gelling- / working time Minimum curing time in dry concrete 1) 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C	Concrete temperature Gelling- / working time Minimum curing time 0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C	Cartridge temp	erature	+5°C to	9 +40°C
Concrete temperature Gelling- / working time Minimum curing in dry concrete	0 °C to +4°C 10 min 2,5 h .5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min Cartridge temperature 0°C to +10°C	0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min Cartridge temperature 0°C to +10°C	0 °C to +4°C 10 min 2,5 h +5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min Cartridge temperature 0°C to +10°C	able B6: M	aximum wor	king time and minimum curing	
	+ 10 °C6 min60 MinCartridge temperature0°C to +10°C	+ 10 °C6 min60 MinCartridge temperature0°C to +10°C	+ 10 °C6 min60 MinCartridge temperature0°C to +10°C	able B6: M E	aximum wor SSVE ONE I	king time and minimum curing CE	Minimum curing time
0 °C to +4°C 10 min 2,5 h	Cartridge temperature 0°C to +10°C	Cartridge temperature 0°C to +10°C	Cartridge temperature 0°C to +10°C	able B6: M E Concrete temp	aximum wor SSVE ONE IG erature	king time and minimum curing CE Gelling- / working time	Minimum curing time in dry concrete ¹⁾
				able B6: M Es Concrete temp 0 °C to	aximum wor SSVE ONE IG erature +4°C	king time and minimum curing CE Gelling- / working time 10 min	Minimum curing time in dry concrete ¹⁾ 2,5 h
+5 °C to +9°C 6 min 80 Min	wet concrete the curing time must be doubled.	wet concrete the curing time must be doubled.	In wet concrete the curing time must be doubled.	able B6: M Concrete temp 0 °C to +5 °C to	aximum wor SSVE ONE IG erature +4°C	king time and minimum curing CE Gelling- / working time 10 min 6 min	Minimum curing time in dry concrete ¹⁾ 2,5 h 80 Min
	+ 10 °C 6 min 60 Min dge temperature 0°C to +10°C	+ 10 °C 6 min 60 Min dge temperature 0°C to +10°C	+ 10 °C 6 min 60 Min dge temperature 0°C to +10°C	36: M E ete temp	aximum wor SSVE ONE IG erature	king time and minimum curing CE Gelling- / working time	Minimum curing time in dry concrete ¹⁾
				ole B6: M Es oncrete temp	aximum wor SSVE ONE IG erature +4°C	king time and minimum curing CE Gelling- / working time 10 min	Minimum curing time in dry concrete ¹⁾ 2,5 h
5 °C to +9°C 6 min 80 Min				De B6: M Es Dencrete temp	aximum wor SSVE ONE IG erature +4°C	king time and minimum curing CE Gelling- / working time 10 min 6 min	Minimum curing time in dry concrete ¹⁾ 2,5 h 80 Min
5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min				le B6: M E3 oncrete temp 0 °C to 5 °C to + 10 °C	aximum wor SSVE ONE IG erature +4°C +9°C	king time and minimum curing CE Gelling- / working time 10 min 6 min 6 min	Minimum curing time in dry concrete ¹⁾ 2,5 h 80 Min 60 Min
5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min artridge temperature 0°C to +10°C				ole B6: M E oncrete temp 0 °C to 5 °C to + 10 °C artridge temp	erature +4°C +9°C erature	king time and minimum curing CE Gelling- / working time 10 min 6 min 6 min 0°C to	Minimum curing time in dry concrete ¹⁾ 2,5 h 80 Min 60 Min
+5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min 60 Min Cartridge temperature 0°C to +10°C 0°C to +10°C				ble B6: M E Concrete temp 0 °C to +5 °C to + 10 °C Cartridge temp	erature +4°C +9°C erature	king time and minimum curing CE Gelling- / working time 10 min 6 min 6 min 0°C to	Minimum curing time in dry concrete ¹⁾ 2,5 h 80 Min 60 Min
5 °C to +9°C 6 min 80 Min + 10 °C 6 min 60 Min Cartridge temperature 0°C to +10°C				ole B6: M E oncrete temp 0 °C to 5 °C to + 10 °C Cartridge temp	erature +4°C +9°C erature	king time and minimum curing CE Gelling- / working time 10 min 6 min 6 min 0°C to	Minimum curing time in dry concrete ¹⁾ 2,5 h 80 Min 60 Min

Injection System ESSVE ONE or ESSVE ONE ICE for concrete

Intended Use

Annex B 6

Curing time



Table C1: Characteristic values for steel tension resistance and steel shear resistance of threaded rods Size M 8 M 10 M 12 M 16 M 20 M24 M 27 M 30 Cross section area [mm²] 36,6 58 84,3 157 245 353 459 561 A_s Characteristic tension resistance, Steel failure 1) Steel, Property class 4.6 and 4.8 N_{Rk,s} [kN] 15 (13) 23 (21) 34 63 98 141 184 224 122 Steel, Property class 5.6 and 5.8 N_{Rk,s} [kN] 18 (17) 29 (27) 42 78 176 230 280 Steel, Property class 8.8 N_{Rk,s} [kN] 29 (27) 46 (43) 67 125 196 282 368 449 Stainless steel A2, A4 and HCR, Property class 50 29 79 123 177 230 281 N_{Rk.s} [kN] 18 42 N_{Rk,s} [kN] Stainless steel A2, A4 and HCR, Property class 70 26 41 59 110 171 247 _ _ Stainless steel A4 and HCR, Property class 80 [kN] 29 46 67 126 196 282 $N_{Rk,s}$ _ Characteristic tension resistance, Partial factor²⁾ Steel, Property class 4.6 [-] 2.0 γMs.V Steel, Property class 4.8 1,5 [-] γMs,V Steel, Property class 5.6 [-] 2,0 γMs,V Steel, Property class 5.8 [-] 1,5 γMs,V Steel, Property class 8.8 [-] 1,5 γMs.V Stainless steel A2, A4 and HCR, Property class 50 2.86 [-] γMs.V Stainless steel A2, A4 and HCR, Property class 70 1.87 [-] γMs.V Stainless steel A4 and HCR, Property class 80 γMs,V [-] 1,6 Characteristic shear resistance, Steel failure 1) Steel, Property class 4.6 and 4.8 V⁰_{Rk.s} [kN] 9 (8) 14 (13) 20 38 59 85 110 135 $\overline{V^0}_{Rk,s}$ arm Steel, Property class 5.6 and 5.8 [kN] 9 (8) 15 (13) 39 61 88 115 140 21 lever Steel, Property class 8.8 V⁰_{Rk,s} 15 (13) 23 (21) 63 98 141 184 224 [kN] 34 Stainless steel A2, A4 and HCR, Property class 50 V⁰_{Rk,s} [kN] 9 15 21 39 61 88 115 140 Without Stainless steel A2, A4 and HCR, Property class 70 $V^0_{Rk,s}$ 124 [kN] 13 20 30 55 86 _ 141 Stainless steel A4 and HCR, Property class 80 V⁰_{Rk,s} 15 23 34 63 98 [kN] -Steel, Property class 4.6 and 4.8 M⁰_{Rk,s} [Nm] 15 (13) 30 (27) 52 133 260 449 666 900 Steel, Property class 5.6 and 5.8 M⁰_{Rk,s} [Nm] 19 (16) 37 (33) 65 166 324 560 833 1123 arm M⁰_{Rk,s} 30 (26) 896 Steel, Property class 8.8 [Nm] 60 (53) 105 266 519 1333 1797 lever Stainless steel A2, A4 and HCR, Property class 50 M⁰_{Rk,s} 19 37 167 325 561 832 1125 [Nm] 66 Nith Stainless steel A2, A4 and HCR, Property class 70 M⁰_{Rk.s} [Nm] 26 52 92 232 454 784 --Stainless steel A4 and HCR, Property class 80 M⁰_{Rk,s} [Nm] 30 59 105 266 519 896 --Characteristic shear resistance, Partial factor²⁾ Steel, Property class 4.6 [-] 1,67 γMs.V Steel, Property class 4.8 [-] 1,25 γMs,V Steel, Property class 5.6 [-] 1.67 γMs.V Steel, Property class 5.8 1,25 [-] γMs.V Steel, Property class 8.8 1,25 [-] γMs,V Stainless steel A2, A4 and HCR, Property class 50 2,38 [-] γMs.V Stainless steel A2, A4 and HCR, Property class 70 [-] 1,56 γMs,V Stainless steel A4 and HCR, Property class 80 1,33 γMs,V [-]

¹⁾ Values are only valid for the given stress area A_s. Values in brackets are valid for undersized threaded rods with smaller stress area A_s for hotdip galvanised threaded rods according to EN ISO 10684:2004+AC:2009.

²⁾ in absence of national regulation

Injection System ESSVE ONE or ESSVE ONE ICE for concrete

Performances

Characteristic values for steel tension resistance and steel shear resistance of threaded rods

Annex C 1



Anchor size threaded	rod			M 8	M 10	M 12	M 16	M 20	M24	M27	M3
Steel failure											
Characteristic tension re	esistance	N _{Rk,s}	[kN]			As	• f _{uk} (or se		C1)		
Deutlel fester		N _{Rk,s, eq}	[kN]				1,0 •				
Partial factor	d a service de lla ser	γMs,N	[-]				see Ta	ole C1			
Combined pull-out and		000/05									
	stance in non-cracked co		IN 1/ 07		10	10		10		10	
Femperature range I: 40°C/24°C	dry and wet concrete	$ au_{Rk,ucr}$	[N/mm ²]	10	12	12	12	12	11	10	9
	flooded bore hole dry and wet concrete	τ _{Rk,ucr}	[N/mm ²]	7,5 7,5	8,5 9	8,5 9	8,5 9	9	8,5	Assessec 7,5	6.5
Temperature range II: 80°C/50°C	flooded bore hole	$ au_{Rk,ucr}$	[N/mm ²]	5,5	6,5	6,5	6,5	•	-	Assessed	- /
Temperature range III:	dry and wet concrete	τ _{Rk,ucr}	[N/mm ²]	5,5	6,5	6,5	6,5	6.5	6,5	5,5	5.0
120°C/72°C	flooded bore hole	τ _{Rk.ucr}	[N/mm ²]	4.0	5.0	5,0	5,0	, -	, ,	Assessed	,
Characteristic bond resi	stance in cracked concre			.,	_ , _	_,_	- / -				
		$\tau_{\rm Rk,cr}$	[N/mm ²]	4,0	5,0	5,5	5,5	5,5	5,5	6,5	6,5
Temperature range I:	dry and wet concrete	$\tau_{\rm Rk,eq}$	[N/mm ²]	2,5	3,1	3,7	3,7	3,7	3,8	4,5	4,5
40°C/24°C	flooded here hele	τ _{Rk,cr}	[N/mm ²]	4,0	4,0	5,5	5,5	No Pe	rformance	Assessed	I (NPA
	flooded bore hole	$\tau_{Rk,eq}$	[N/mm ²]	2,5	2,5	3,7	3,7	No Pe	rformance	Assessed	I (NPA
	dry and wet concrete	$ au_{Rk,cr}$	[N/mm ²]	2,5	3,5	4,0	4,0	4,0	4,0	4,5	4,
Temperature range II:	dry and wet concrete	$\tau_{\text{Rk,eq}}$	[N/mm ²]	1,6	2,2	2,7	2,7	2,7	2,8	3,1	3,
80°C/50°C	flooded bore hole	$\tau_{\rm Rk,cr}$	[N/mm ²]	2,5	3,0	4,0	4,0			Assessed	
		$\tau_{\text{Rk,eq}}$	[N/mm ²]	1,6	1,9	2,7	2,7			Assessed	<u>,</u>
	dry and wet concrete	$\tau_{\rm Rk,cr}$	[N/mm ²]	2,0	2,5	3,0	3,0	3,0	3,0	3,5	3,
Temperature range III: 120°C/72°C	-	$\tau_{\rm Rk,eq}$ $\tau_{\rm Rk,cr}$	[N/mm ²]	1,3	1,6	2,0	2,0	2,0	2,1	2,4	2,4
12010/1210	flooded bore hole		[N/mm ²]	2,0	2,5	3,0	3,0 2.0			Assessed	`
		τ _{Rk,eq} C25/30	[N/mm²]	1,3	1,6	2,0	2,0		normance	Assessed	
		C30/3	-				1,0				
Increasing factors for co		C35/4				1,0					
(only static or quasi-stat	tic actions)	C40/50					1,0				
ψ_{c}		C45/5	5	1,09							
							1,1	0			
Concrete cone failure											
Non-cracked concrete		k _{ucr,N}	[-]				11	,0			
Cracked concrete		k _{cr,N}	[-]				7,	7			
Edge distance		C _{cr,N}	[mm]	1,5 h _{ef}							
Axial distance		S _{cr,N}	[mm]	2 c _{or.N}							
Splitting		Scr,N	[[]				2.0	cr,N			
opinting	h/h _{ef} ≥ 2,0			1,0 h _{ef}							
Edge distance	2,0 > h/h _{ef} > 1,3	C _{cr,sp}	[mm]	$2 \cdot h_{ef}\left(2,5-\frac{h}{L}\right)$							
	h/h _{ef} ≤ 1,3	- 0.100				h_{ef})					
Axial distance	1	S _{cr,sp}	[mm]				2,4 2 c	h _{ef}			
Installation factor		Jor,sp)				2 00	,sp			
for dry and wet concrete	9	γinst	[-]	1,0				1,2			
for flooded bore hole		γinst	[-]		1	,4		No Pe	rformance	Assessed	I (NPA
Injection Syster	n ESSVE ONE or	ESSVE ON	E ICE fo	r conc	rete				A	ex C 2	



Table C3: Characteristi seismic actio						tic, qu	asi-sta	atic ac	tion and	k
Anchor size threaded rod			M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Steel failure without lever arm									•	
Characteristic shear resistance Steel, strength class 4.6 and 4.8	V ⁰ _{Rk,s}	[kN]			0,	6 ∙ A _s ∙ f _{uk}	(or see T	able C1)		
Characteristic shear resistance Steel, strength class 5.6, 5.8 and 8.8 Stainless Steel A2, A4 and HCR, all classes	V ⁰ _{Rk,s}	[kN]			0,	5・A _s ・f _{uk}	(or see T	able C1)		
Characteristic shear resistance	$V_{Rk,s,eq}$	[kN]				0,7	70 • V ⁰ _{Rk,s}			
Partial factor	γms,v	[-]				see	Table C1			
Ductility factor	k ₇	[-]					1,0			
Steel failure with lever arm										
Obeve eterietie ben ding memorat	M ⁰ _{Rk,s}	[Nm]			1,2	2 ∙ W _{el} ∙ f _{ul}	(or see T	able C1)		
Characteristic bending moment	M ⁰ _{Rk,s,eq}	[Nm]			No F	Performan	ice Asses	sed (NPA	.)	
Partial factor	γMs,∨	[-]	see Table C1							
Concrete pry-out failure										
Factor	k ₈	[-]	2,0							
Installation factor	γinst	[-]	1,0							
Concrete edge failure										
Effective length of fastener	lf	[mm]			min(h _{ef} ;	12 · d _{nom})			max(8 · d _{no}	, 300 mm)
Outside diameter of fastener	d _{nom}	[mm]	8	10	12	16	20	24	27	30
Installation factor	γinst	[-]					1,0			
Factor for annular gap	α_{gap}	[-]				0,	5 (1,0) ¹⁾			

¹⁾ Value in brackets valid for filled annular gab between anchor and clearance hole in the fixture. Use of special filling washer Annex A 3 is required

Injection System ESSVE ONE or ESSVE ONE ICE for concrete

Performances

Characteristic values of shear loads under static, quasi-static action and seismic action (performance category C1)

Annex C 3



Anchor size internal th	readed anchor rods			IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20	
Steel failure ¹⁾										
Characteristic tension re		N _{Bk.s}	[kN]	10	17	29	42	76	123	
Steel, strength class 5.8 Partial factor		,.					.5			
Characteristic tension re	eistance	γMs,N	[-]				,_			
Steel, strength class 8.8		N _{Rk,s}	[kN]	16	27	46	67	121	196	
Partial factor		γMs,N	[-]			1	,5			
Characteristic tension re		N _{Rk.s}	[kN]	14	26	41	59	110	124	
Stainless Steel A4 and I	HCR, Strength class 70				20		00	110		
Partial factor	1	γMs,N	[-]			1,87			2,86	
	d concrete cone failure	ta 000/05								
	stance in non-cracked concre		[N]//21	10	10	10	10	44	0	
Temperature range I: 40°C/24°C	dry and wet concrete	$\tau_{\rm Rk,ucr}$	[N/mm ²]	12	12	12	12	11	9	
	flooded bore hole	$ au_{Rk,ucr}$	[N/mm ²]	8,5	8,5	8,5		mance Asses	, ,	
Temperature range II: 80°C/50°C	dry and wet concrete	$\tau_{\rm Rk,ucr}$	[N/mm ²]	9	9	9	9	8,5	6,5	
	flooded bore hole	τ _{Rk,ucr}	[N/mm ²]	6,5	6,5	6,5		mance Asses	, ,	
Temperature range III: 120°C/72°C	dry and wet concrete	$\tau_{\rm Rk,ucr}$	[N/mm ²]	6,5	6,5	6,5	6,5	6,5	5,0	
	stance in cracked concrete C	τ _{Rk,ucr}	[N/mm²]	5,0	5,0	5,0	No Perfor	mance Asses	sea (NPA)	
			[N]/mm2]	5.0	5,5	5 5	5,5	5,5	6,5	
Temperature range I: 40°C/24°C	dry and wet concrete flooded bore hole	$\tau_{\rm Rk,cr}$	[N/mm ²] [N/mm ²]	5,0 4,0	5,5	5,5 5,5	,	5,5 mance Asses	,	
		$\tau_{\rm Rk,cr}$,						
Temperature range II: 80°C/50°C	dry and wet concrete	$\tau_{\rm Rk,cr}$	[N/mm ²]	3,5	4,0	4,0	4,0	4,0 mance Asses	4,5	
	flooded bore hole	$\tau_{\rm Rk,cr}$	[N/mm ²] [N/mm ²]	3,0	3,0	4,0 3,0	3.0	3.0	3.5	
Temperature range III: 120°C/72°C	dry and wet concrete flooded bore hole	$\tau_{\rm Rk,cr}$	[N/mm ²]	2,5 2,5	3,0	3,0	- / -	3,0 mance Asses	- / -	
		τ _{Rk,cr}	25/30	2,5	3,0	- / -	02	mance Asses	sea (NFA)	
			30/37			,	02			
la craccia e factora for co	navata		35/45			,	07			
Increasing factors for co Ψ_c	ncrete		40/50			-	07			
Ψc			45/55			,	09			
			+5/55 50/60			-	10			
Concrete cone failure		0.	50/00			١,				
Non-cracked concrete		k _{ucr,N}	[-]			11	1,0			
Cracked concrete		k _{cr,N}	[-]				,7			
Edge distance			[mm]							
Axial distance		C _{cr,N} S _{cr,N}	[mm]				i h _{ef} D _{cr.N}			
Splitting failure		UCI,N	[]				CI,N			
	h/h _{ef} ≥ 2,0					1.0) h _{ef}			
	17/11ef = 2,0					1,0	/ Het			
Edge distance	2,0 > h/h _{ef} > 1,3	C _{cr,sp}	[mm]			$2 \cdot h_{ef} \Big 2$	$5-\frac{h}{2}$			
Logo alotalloo	_, , , , , , , , , , , , , , , , , , ,	Ci,sp	[]			- · · ef (-	h_{ef}			
	h/h _{ef} ≤ 1,3			2,4 h _{ef}						
Axial distance		6	[mm]			,	cr.sp			
		S _{cr,sp}	fund			20	cr,sp			
Installation factor										
for dry and wet concrete)	γinst	[-]			1	,2			
for flooded bore hole		γinst	[-]	1,4 -						
threaded rod and the faste	rews or threaded rods (incl. r . The characteristic tension re ening element. strength class 50 is valid									
Injection System	SVE ON	E ICE fo	r concre	ete			Annex (` 4		



Anchor size for internal threaded	l anchor ro	ds	IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20				
Steel failure without lever arm ¹⁾								1				
Characteristic shear resistance, Steel, strength class 5.8	$V^0_{\ Rk,s}$	[kN]	5	9	15	21	38	61				
Partial factor	γ _{Ms,V}	[-]				1,25		1				
Characteristic shear resistance, Steel, strength class 8.8	V ⁰ _{Rk,s}	[kN]	8	14	23	34	60	98				
Partial factor	γ _{Ms,V}	[-]				1,25						
Characteristic shear resistance, Stainless Steel A4 and HCR, Strength class 70 ²⁾	V ⁰ _{Rk,s}	[kN]	7	13	20	30	55	40				
Partial factor	γMs,∨	[-]		1,56 2,38								
Ductility factor	k ₇	[-]				1,0						
Steel failure with lever arm ¹⁾												
Characteristic bending moment, Steel, strength class 5.8	M ⁰ _{Rk,s}	[Nm]	8	19	37	66	167	325				
Partial factor	γMs,V	[-]		1,25								
Characteristic bending moment, Steel, strength class 8.8	M ⁰ _{Rk,s}	[Nm]	12	30	60	105	267	519				
Partial factor	γ̃Ms,∨	[-]										
Characteristic bending moment, Stainless Steel A4 and HCR, Strength class 70 ²⁾	M ⁰ _{Rk,s}	[Nm]	11	26	52	92	233	456				
Partial factor	ŶMs,V	[-]			1,56			2,38				
Concrete pry-out failure												
actor	k ₈	[-]				2,0						
nstallation factor	γinst	[-]				1,0						
Concrete edge failure												
Effective length of fastener	l _f	[mm]		m	in(h _{ef} ; 12 • d _n	om)		max(8•d _{nom} ; 300 mm				
Dutside diameter of fastener	d _{nom}	[mm]	10	12	16	20	24	30				
nstallation factor	γinst	[-]			1	1,0						
 Fastening screws or thr threaded rod. The chara and the fastening eleme For IG-M20 strength cla 	acteristic tei ent.	nsion resis										
Injection System ESSV Performances		or ESS\	/E ONE I	CE for co	oncrete			Annex C 5				



Anchor size reinforcin	g bar				Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
Steel failure														
Characteristic tension re	esistance		N _{Rk,s}	[kN]					$A_s \cdot f_{uk}^{(1)}$					
	seismic action (reinforcing bar area area <t< td=""><td></td><td>N_{Rk,s, eq}</td><td>[kN]</td><td></td><td></td><td></td><td></td><td>0 ∙ A_s ∙ f</td><td>1</td><td></td><td></td><td></td></t<>		N _{Rk,s, eq}	[kN]					0 ∙ A _s ∙ f	1				
Cross section area	ure ristic tension resistance ction area ctor d pull-out and concrete failure ristic bond resistance in non-cracked cure range I: dry and wet concrete C flooded bore hole ure range III: dry and wet concrete °C flooded bore hole ure range III: dry and wet concrete °C flooded bore hole ristic bond resistance in cracked cor flooded bore hole rure range III: dry and wet concrete flooded bore hole flooded bore hole dry and wet concrete flooded bore hole dry and wet concrete flooded bore hole flooded bore hole dry and wet concrete flooded bore hole flooded bore hole g factors for concrete concrete ance 2,0 > h/h _{ef} > 1,3 ance h/h _{ef}		As	[mm ²]	50	79	113	154	201	314	491	616	804	
Partial factor			γms,N	[-]					1,4 ²⁾					
			porete OOO	25										
				²⁵ [N/mm ²]	10	12	12	12	12	12	11	10	8,5	
Temperature range I: 40°C/24°C			$\tau_{Rk,ucr}$ $\tau_{Rk,ucr}$	[N/mm ²]	7,5	8,5	8,5	8,5	8,5				,	
Temperature range II:			τ _{Rk,ucr}	[N/mm ²]	7,5	9	9	9	9	9	8,0	7,0	6,0	
80°C/50°C			τ _{Rk,ucr}	[N/mm ²]	5,5	6,5	6,5	6,5	6,5	No Per	,	,	,	
Temperature range III:	dry and wet	concrete	$\tau_{\rm Rk,ucr}$	[N/mm ²]	5,5	6,5	6,5	6,5	6,5	6,5	6,0	5,0	4,5	
120°C/72°C			$\tau_{\text{Rk,ucr}}$	[N/mm²]	4,0	5,0	5,0	5,0	5,0	No Per	formance	Assessed	d (NPA	
Characteristic bond resi	stance in crac	cked concre	ete C20/25											
	dry and wet	concrete	$\tau_{Rk,cr}$	[N/mm ²]	4,0	5,0	5,5	5,5	5,5	5,5	5,5	6,5	6,5	
Temperature range I: 40°C/24°C			$\tau_{Rk,eq}$	[N/mm ²]	2,5	3,1	3,7	3,7	3,7	,	- / -	4,5	4,5	
40 0/24 0	flooded bore	e hole	τ _{Rk,cr}	[N/mm ²]	4,0 2,5	4,0 2,5	5,5 3,7	5,5 3,7	5,5 3,7				(
			$\tau_{\rm Rk,eq}$	[N/mm ²] [N/mm ²]	2,5	2,5	4,0	4,0	3,7 4,0		314 491 12 11 No Performance 9 9 8,0 No Performance 6,5 6,5 6,0 No Performance 9 5,5 5,5 3,7 3,8 No Performance No Performance No Performance 10 4,0 4,0 2,7 2,8 No Performance 3,0 3,0 3,0 2,0 2,1 No Performance 3,0	Assessed	d (NPA	
Temperature range II:	dry and wet	concrete	$\tau_{\text{Rk,cr}}$ $\tau_{\text{Rk,eq}}$	[N/mm ²]	2,5	2,2	2,7	2,7	2,7	,		4,5	4,5	
80°C/50°C			τ _{Rk,cr}	[N/mm ²]	2,5	3.0	4,0	4,0	4.0	,	,	- /	,	
	flooded bore	e hole	τ _{Rk,eq}	[N/mm ²]	1,6	1,9	2,7	2,7	2,7	No Performance Ass No Performance Ass 3,0 3,0 2,0 2,1 No Performance Ass				
			$\tau_{\rm Rk,cr}$	[N/mm ²]	2,0	2,5	3,0	3,0	3,0	3,0	3,0	3,5	3,5	
Temperature range III:	dry and wet	concrete	$\tau_{\rm Rk,eq}$	[N/mm²]	1,3	1,6	2,0	2,0	2,0	2,0	-	2,4	2,4	
120°C/72°C	flooded bore	hole	$\tau_{\text{Rk,cr}}$	[N/mm²]	2,0	2,5	3,0	3,0	3,0	No Per	formance	Assessed	d (NPA	
			$\tau_{\rm Rk,eq}$	[N/mm ²]	1,3	1,6	2,0	2,0	2,0	No Per	No Performance Assessed (N		d (NPA	
			C25						1,02					
Increasing factors for co	oncrete		C30/37 1,04 C35/45 1,07											
(only static or quasi-stat			C36		1,07									
ψ_{c}			C40						1,08					
			C50						1,10					
Concrete cone failure														
Non-cracked concrete			k _{ucr,N}	[-]					11,0					
Cracked concrete			k _{cr,N}	[-]					7,7					
Edge distance			C _{cr,N}	[mm]					1,5 h _{ef}					
Axial distance			S _{cr,N}	[mm]					$2 c_{cr,N}$					
Splitting	h/h > 0.0								1.0.5					
	n/n _{ef} 2 2,0		-						1,0 h _{ef}	``				
Edge distance	2,0 > h/h _{ef} >	1,3	C _{cr,sp}	[mm]				$2 \cdot h_{a}$	_{ef} 2,5 -	$\left(\frac{h}{h_{ef}}\right)$				
	h/h _{ef} ≤ 1,3								2,4 h _{ef}	~ /				
Axial distance			S _{cr,sp}	[mm]					$2 c_{\text{cr,sp}}$					
Installation factor														
for dry and wet concrete	9		γ_{inst}	[-]	1,0				1	,2	-			
for flooded bore hole ¹⁾ f _{uk} shall be take ²⁾ in absence of n	n from the spe ational regula	ecifications tion	γ _{inst} of reinforcin	[-] g bars			1,4			No Per	formance	Assessed	d (NPA	
Injection System Performances Characteristic values							ete				Anne	ex C (6	



Table C7: Characteristic value seismic action (perfection)					atic,	quas	i-stat	ic act	tion a	nd		
Anchor size reinforcing bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
Steel failure without lever arm												
Characteristic shear resistance	$V^0{}_{Rk,s}$	[kN]				0,5	0 ∙ A _s ∙ 1	t 1) uk				
Characteristic shear resistance	$V_{Rk,s,eq}$	[kN]				0,3	5 • A _s • 1	; 1) uk				
Cross section area	A _s	[mm²]	50	79	113	154	201	314	491	616	804	
Partial factor	γms,v	[-]					1,5 ²⁾					
Ductility factor	k ₇	[-]					1,0					
Steel failure with lever arm												
Characteristic handing moment	M ⁰ _{Rk,s}	[Nm]	1.2 • W _{el} • f _{uk} ¹⁾									
Characteristic bending moment	M ⁰ _{Rk,s, eq}	[Nm]			No P	erforma	nce Ass	sessed (NPA)				
Elastic section modulus	W _{el}	[mm³]	50	98	170	269	402	785	1534	2155	3217	
Partial factor	γms,v	[-]					1,5 ²⁾	1,5 ²⁾				
Concrete pry-out failure												
Factor	k ₈	[-]					2,0					
Installation factor	γ inst	[-]					1,0					
Concrete edge failure												
Effective length of fastener	lf	[mm]		r	nin(h _{ef} ; 1	2 • d _{nom})		max(8 ·	• d _{nom} , 30	00 mm)	
Outside diameter of fastener	d _{nom}	[mm]	8	10	12	14	16	20	25	28	32	
Installation factor	γinst	[-]					1,0					
Factor for annular gap	$lpha_{gap}$	[-]				C	,5 (1,0) ¹)				
 f_{uk} shall be taken from the specifications of reinfo in absence of national regulation Value in brackets valid for filled annular gab betv required 	rcing bars veen anchor and	d clearan	ice hole	in the fi	xture. U	se of spe	ecial fillir	ng wash	er Anne	x A 3 is		

Performances

Characteristic values of shear loads under static, quasi-static action and seismic action (performance category C1)

Annex C 7



Table C8: Di	splaceme	nts under tens	ion load ¹⁾	(threa	aded ro	od)					
Anchor size thread	led rod		M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30	
Non-cracked conc	rete C20/25										
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]	0,021	0,023	0,026	0,031	0,036	0,041	0,045	0,049	
40°C/24°C	$\delta_{N_\infty}\text{-factor}$	[mm/(N/mm ²)]	0,030	0,033	0,037	0,045	0,052	0,060	0,065	0,071	
Temperature range II:	δ_{N0} -factor	[mm/(N/mm ²)]	0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119	
80°C/50°C	$\delta_{N\infty}\text{-factor}$	[mm/(N/mm ²)]	0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172	
Temperature range III:	δ_{N0} -factor	[mm/(N/mm ²)]	0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119	
120°C/72°C	$\delta_{N_\infty}\text{-factor}$	[mm/(N/mm ²)]	0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172	
Cracked concrete	C20/25										
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]	0,0	90	0,070						
40°C/24°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm ²)]	0,1	05			0,1	05			
Temperature range II:	δ_{N0} -factor	[mm/(N/mm ²)]	0,2	219			0,1	70			
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm ²)]	0,2	255	0,245						
Temperature range III:	δ_{N0} -factor	[mm/(N/mm ²)]	0,2	219			0,1	70			
120°C/72°Č	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm ²)]	0,2	255			0,2	245			

¹⁾ Calculation of the displacement $\delta_{N0} = \delta_{N0} \text{-factor} \cdot \tau;$

 τ : action bond stress for tension

 $\delta_{N\infty} = \delta_{N\infty} \text{-factor} \cdot \tau;$

Displacements under shear load¹⁾ (threaded rod) Table C9:

Anchor size thre	aded rod		M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
For non-cracked	l concrete C2	0/25								
All temperature	δ_{V0} -factor	[mm/kN]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V\infty}\text{-}factor$	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05
For cracked con	crete C20/25									
All temperature ranges	δ_{V0} -factor	[mm/kN]	0,12	0,12	0,11	0,10	0,09	0,08	0,08	0,07
	$\delta_{V\infty}$ -factor	[mm/kN]	0,18	0,18	0,17	0,15	0,14	0,13	0,12	0,10
$\delta_{V0} = \delta_{V0}$ -facto $\delta_{V\infty} = \delta_{V\infty}$ -facto	or · V;	V: action shear load								
Injection System	em ESSVE C	ONE or ESSVE ONE	ICE for	concr	ete					
Performances								An	nex C	8

Displacements (threaded rods)

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Anchor size reinfo	orcing bar		Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32		
Non-cracked cond	crete C20/2	25											
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]	0,021	0,023	0,026	0,028	0,031	0,036	0,043	0,047	0,052		
40°C/24°C	$\delta_{N\infty}$ -factor	[mm/(N/mm ²)]	0,030	0,033	0,037	0,041	0,045	0,052	0,061	0,071	0,075		
Temperature range II:	δ_{N0} -factor	[mm/(N/mm ²)]	0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,126		
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm ²)]	0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,18		
Temperature range III:	δ_{N0} -factor	[mm/(N/mm ²)]	0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,126		
120°C/72°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm ²)]	0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,18		
Cracked concrete	C20/25	•											
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]	0,0)90	0,070								
40°C/24°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm ²)]	0,1	05				0,105					
Temperature range II:	δ_{N0} -factor	[mm/(N/mm ²)]	0,2	219				0,170					
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm ²)]	0,255					0,245					
Cemperature range III:	δ_{N0} -factor	[mm/(N/mm ²)]	0,219					0,170					
120°C/72°Č	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm ²)]	0,2	255				0,245					
¹⁾ Calculation of th $\delta_{N0} = \delta_{N0}$ -factor $\delta_{N\infty} = \delta_{N\infty}$ -factor Table C11: D	·τ; ·τ;	τ: action bond			ebar)					-			
Anchor size reinfo	orcing bar		Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32		
Non-cracked cond	crete C20/2	25											
All temperature	δ_{V0} -factor	[mm/kN]	0,06	0,05	0,05	0,04	0,04	0,04	0,03	0,03	0,03		
ranges	$\delta_{V_{\infty}}$ -factor	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,04	0,04		
Cracked concrete	C20/25												
All temperature	δ_{V0} -factor	[mm/kN]	0,12	0,12	0,11	0,11	0,10	0,09	0,08	0,07	0,06		
Antemperature													

¹⁾ Calculation of the displacement $\delta_{V0} = \delta_{V0}$ -factor $\cdot V$; $\delta_{V_{\infty}} = \delta_{V_{\infty}}$ -factor $\cdot V$;

V: action shear load

Injection System ESSVE ONE or ESSVE ONE ICE for concrete

Performances Displacements (rebar) Annex C 9



Table C12: Dis	splacements	s under tension	load ¹⁾ (lı	nternal t	hreaded	anchor	rod)	
Anchor size Interna	al threaded and	chor rod	IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20
Non-cracked concret	e C20/25 under	static and quasi-stati	c action					
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]	0,023	0,026	0,031	0,036	0,041	0,049
40°C/24°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,033	0,037	0,045	0,052	0,060	0,071
Temperature range II:	δ_{N0} -factor	[mm/(N/mm ²)]	0,056	0,063	0,075	0,088	0,100	0,119
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm ²)]	0,081	0,090	0,108	0,127	0,145	0,172
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,056	0,063	0,075	0,088	0,100	0,119
120°C/72°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm ²)]	0,081	0,090	0,108	0,127	0,145	0,172
Cracked concrete C2	0/25 under stati	c and quasi-static ac	tion					
Temperature range I:	δ_{N0} -factor	[mm/(N/mm ²)]	0,090			0,070		
40°C/24°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,105			0,105		
Temperature range II:	δ_{N0} -factor	[mm/(N/mm ²)]	0,219			0,170		
80°C/50°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm ²)]	0,255			0,245		
Temperature range III:	δ_{N0} -factor	[mm/(N/mm ²)]	0,219			0,170		
120°C/72°Č	$\delta_{N\infty}$ -factor	[mm/(N/mm ²)]	0,255			0,245		

¹⁾ Calculation of the displacement

 $\delta_{\text{N0}} = \delta_{\text{N0}} \text{-factor} \quad \cdot \ \tau; \qquad \qquad \tau: \text{ action bond stress for tension}$

 $\delta_{N\infty} = \delta_{N\infty} \text{-factor} \quad \tau;$

Table C13: Displacements under shear load¹⁾ (Internal threaded anchor rod)

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Anchor size Int	ternal threaded	l anchor rod	IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20
Non-cracked a	nd cracked cor	ncrete C20/25 u	nder static a	and quasi-s	static action	้า		
All temperature	δ_{V0} -factor	[mm/kN]	0,07	0,06	0,06	0,05	0,04	0,04
ranges	$\delta_{V_{\infty}}$ -factor	[mm/kN]	0,10	0,09	0,08	0,08	0,06	0,06
Injection Sys Performances Displacements (I		ONE or ESSVE	ONE ICE f	or concre	te		Annex	C 10