



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-17/0128 of 7 June 2019

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

This version replaces

Deutsches Institut für Bautechnik

Mungo Injection system MIT-Hybrid Plus for concrete

Bonded anchor for use in concrete

Mungo Befestigungstechnik AG Bornfeldstrasse 2 4603 OLTEN SCHWEIZ

Werk 13 / Plant 13

31 pages including 3 annexes which form an integral part of this assessment

EAD 330499-01-0601

ETA-17/0128 issued on 20 February 2017



European Technical Assessment ETA-17/0128

Page 2 of 31 | 7 June 2019

English translation prepared by DIBt

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European Technical Assessment ETA-17/0128

Page 3 of 31 | 7 June 2019

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Specific Part

1 Technical description of the product

The "Mungo Injection system MIT-Hybrid Plus for concrete" is a bonded anchor consisting of a cartridge with injection mortar MIT-Hybrid, MIT-Hybrid Plus and a steel element. The steel element consist of a commercial threaded rod with washer and hexagon nut in the range of M8 to M30, reinforcing bar in the range of diameter $\emptyset 8$ to $\emptyset 32$ mm or internal threaded rod IG-M6 to IG-M20.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between metal part, injection mortar and concrete.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

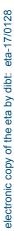
3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance			
Characteristic resistance to tension load	See Annex			
(static and quasi-static loading)	C 1, C 2, C 4, C 6			
Characteristic resistance to shear load	See Annex			
(static and quasi-static loading)	C 1, C 3, C 5, C 7			
Displacements	See Annex			
(static and quasi-static loading)	C 8 to C 10			
Characteristic resistance for seismic performance	See Annex			
category C1	C 11 to C 14			
Characteristic resistance and displacements for seismic	See Annex			
performance category C2	C 11, C 12, C 15			

3.2 Hygiene, health and the environment (BWR 3)

Essential characteristic	Performance
Content, emission and/or release of dangerous substances	No performance assessed





European Technical Assessment ETA-17/0128

Page 4 of 31 | 7 June 2019

English translation prepared by DIBt

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with the European Assessment Document EAD 330499-01-0601 the applicable European legal act is: [96/582/EC].

The system to be applied is: 1

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at Deutsches Institut für Bautechnik.

Issued in Berlin on 7 June 2019 by Deutsches Institut für Bautechnik

BD Dipl.-Ing. Andreas Kummerow Head of Department

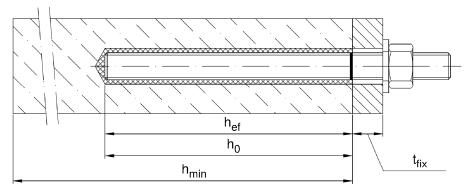
beglaubigt: Baderschneider



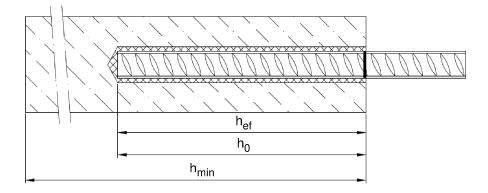
Installation threaded rod M8 up to M30

prepositioned installation or

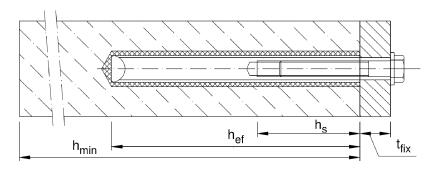
push through installation (annular gap filled with mortar)



Installation reinforcing bar Ø8 up to Ø32



Installation internal threaded anchor rod IG-M6 up to IG-M20



 t_{fix} = thickness of fixture

h_{ef} = effective anchorage depth

 $h_0 = depth of drill hole$

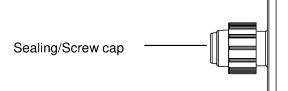
 h_{min} = minimum thickness of member

Mungo Injection system MIT-Hybrid Plus for concrete	
Product description Installed condition	Annex A 1



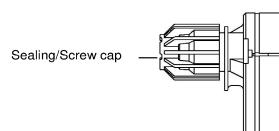
Cartridge: MIT-Hybrid, MIT-Hybrid Plus

150 ml, 280 ml, 300 ml up to 333 ml and 380 ml up to 420 ml cartridge (Type: coaxial)



Imprint: MIT-Hybrid, MIT-Hybrid Plus, processing notes, charge-code, shelf life, storage temperature, hazard-code, curing- and processing time (depending on the temperature), with as well as without travel scale

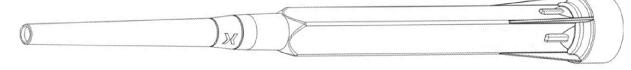
235 ml, 345 ml up to 360 ml and 825 ml cartridge (Type: "side-by-side")



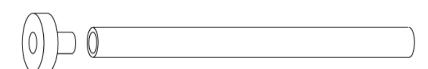
Imprint: MIT-Hybrid, MIT-Hybrid Plus, processing notes, charge-code, shelf life, storage temperature, hazard-code, curing- and processing time (depending on the temperature), with as well as without travel scale

Static Mixer

electronic copy of the eta by dibt: eta-17/0128



Piston plug and mixer extension



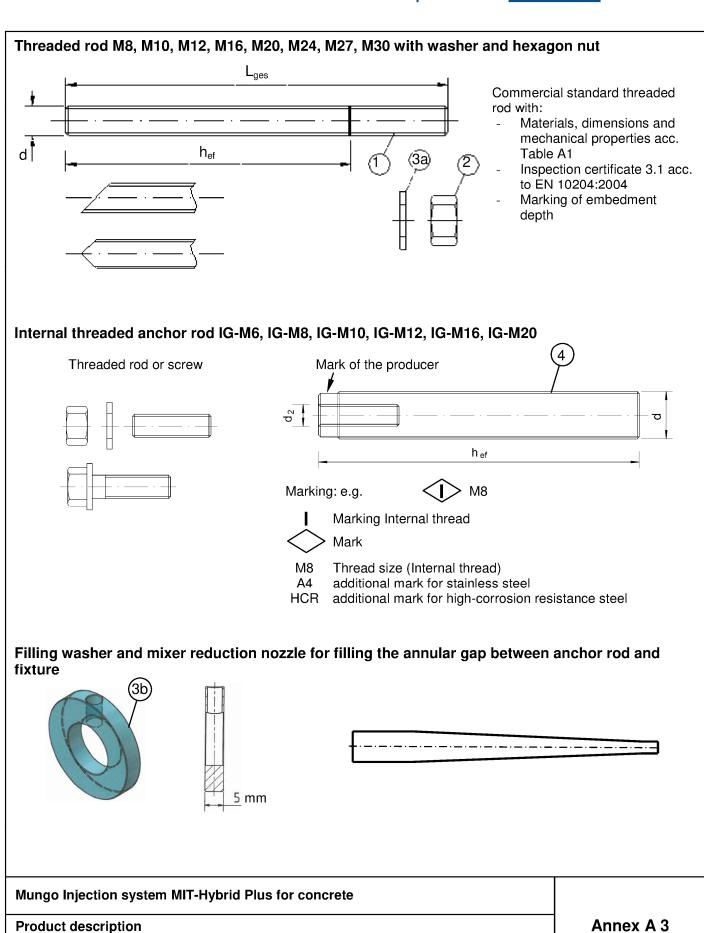
Mungo Injection system MIT-Hybrid Plus for concrete

Product description

Injection system

Annex A 2





Threaded rod, internal threaded rod and filling washer

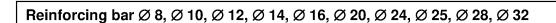


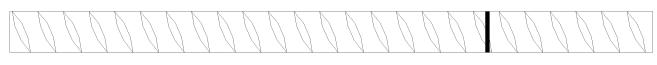
Та	ble A1: Materials								
Part	Designation	Material							
Steel, zinc plated (Steel acc. to EN 10087:1998 or EN 10263:2001) - zinc plated ≥ 5 μm acc. to EN ISO 4042:1999 or - hot-dip galvanised ≥ 40 μm acc. to EN ISO 1461:2009 and EN ISO 10684:2004+AC:2009 or - sherardized ≥ 45 μm acc. to EN ISO 17668:2016									
		Property class	Property class Characteristic Characteristic Elocation Elocation Characteristic Elocation fractions of the control of the cont						
			4.6	f _{uk} = 400 N/mm ²	f _{yk} = 240 N/mm ²	A ₅ > 8%			
1	Threaded rod		4.8	f _{uk} = 400 N/mm ²	f _{yk} = 320 N/mm ²	A ₅ > 8%			
·		acc. to EN ISO 898-1:2013	5.6	f _{uk} = 500 N/mm ²	f _{yk} = 300 N/mm ²	A ₅ > 8%			
		LIV 100 090-1.2010	5.8	f _{uk} = 500 N/mm ²	f _{yk} = 400 N/mm ²	A ₅ > 8%			
			8.8	f _{uk} = 800 N/mm ²	f _{yk} = 640 N/mm ²	$A_5 \ge 12\%^{3}$			
		acc. to	4	for threaded rod c					
2	Hexagon nut	EN ISO 898-2:2012	5	for threaded rod c					
		Ctool sine plated but di	8	for threaded rod c					
3a	Washer	Steel, zinc plated, hot-di (e.g.: EN ISO 887:2006,	EN IS	O 7089:2000, EN IS	SO 7093:2000 or E	N ISO 7094:2000)			
3b	Filling washer	Steel, zinc plated, hot-di	p galva			le			
	Internal threaded	Property class		Characteristic tensile strength	Characteristic yield strength	Elongation at fracture			
4	anchor rod	acc. to		$f_{uk} = 500 \text{ N/mm}^2$	$f_{yk} = 400 \text{ N/mm}^2$	A ₅ > 8%			
		EN ISO 898-1:2013	8.8	f _{uk} = 800 N/mm ²	$f_{yk} = 640 \text{ N/mm}^2$	A ₅ > 8%			
Stai	nless steel A2 (Material 1.4 nless steel A4 (Material 1.4 n corrosion resistance stee	401 / 1.4404 / 1.4571 / 1.43	62 or 1	.4578, acc. to EN	10088-1:2014)				
		Property class		Characteristic tensile strength	Characteristic yield strength	Elongation at fracture			
1	Threaded rod ¹⁾⁴⁾		50	f _{uk} = 500 N/mm ²	f _{yk} = 210 N/mm ²	A ₅ ≥ 12% ³⁾			
		acc. to EN ISO 3506-1:2009	70	f _{uk} = 700 N/mm ²	f _{yk} = 450 N/mm ²	$A_5 \ge 12\%^{3)}$			
		EN 130 3300-1.2009	80	f _{uk} = 800 N/mm ²	f _{vk} = 600 N/mm ²	A ₅ ≥ 12% ³⁾			
			50	for threaded rod c	lass 50	1			
2	Hexagon nut 1)4)	acc. to EN ISO 3506-1:2009	70						
			80	for threaded rod c					
A2: Material 1.4301 / 1.4303 / 1.4307 / 1.4567 or 1.4541, acc. to EN 10088-1:2014 A4: Material 1.4401 / 1.4404 / 1.4571 / 1.4362 or 1.4578, acc. to EN 10088-1:2014 HCR: Material 1.4529 or 1.4565, acc. to EN 10088-1: 2014 (e.g.: EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000)									
3b	Filling washer	Stainless steel A4, High	corrosi						
		Property class		Characteristic tensile strength	Characteristic yield strength	Elongation at fracture			
	Internal threaded	acc. to	50	f _{uk} = 500 N/mm ²	f _{yk} = 210 N/mm ²	A ₅ > 8%			
4	anchor rod ¹⁾²⁾	EN ISO 3506-1:2009	70	$f_{uk} = 700 \text{ N/mm}^2$	$f_{yk} = 450 \text{ N/mm}^2$	A ₅ > 8%			
1) Property class 70 for threaded rods up to M24 and Internal threaded anchor rods up to IG-M16,									

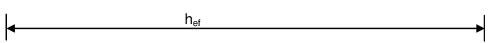
¹⁾ Property class 70 for threaded rods up to M24 and Internal threaded anchor rods up to IG-M16,

Mungo Injection system MIT-Hybrid Plus for concrete	
Product description Materials threaded rod and internal threaded rod	Annex A 4

²⁾ for IG-M20 only property class 50 $^{3)}$ A₅ > 8% fracture elongation if <u>no</u> requirement for performance category C2 exists $^{4)}$ Property class 80 only for stainless steel A4







- Minimum value of related rip area f_{R.min} according to EN 1992-1-1:2004+AC:2010
- Rib height of the bar shall be in the range 0,05d ≤ h ≤ 0,07d (d: Nominal diameter of the bar; h: Rip height of the bar)

Table A2: Materials

Part	Designation	Material
Reinf	orcing bars	
1	FN 1447-1-1"2007-140" 2010 ANNOV (:	Bars and de-coiled rods class B or C f_{yk} and k according to NDP or NCL of EN 1992-1-1/NA $f_{uk} = f_{tk} = k \cdot f_{yk}$

Mungo Injection system MIT-Hybrid Plus for concrete	
Product description Materials reinforcing bar	Annex A 5



Specifications of intended use

Anchorages subject to:

- Static and quasi-static loads: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.
- Seismic action for Performance Category C1: M8 to M30 (except hot-dip galvanised rods), Rebar Ø8 to Ø32.
- Seismic action for Performance Category C2: M12 to M24 (except hot-dip galvanised rods).

Base materials:

- Compacted reinforced or unreinforced normal weight concrete without fibres according to EN 206:2013.
- Strength classes C20/25 to C50/60 according to EN 206:2013.
- Non-cracked concrete: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.
- Cracked concrete: M8 to M30, Rebar Ø8 to Ø32, IG-M6 to IG-M20.

Temperature Range:

- I: 40 °C to +80 °C (max long term temperature +50 °C and max short term temperature +80 °C)
- II: 40 °C to +120 °C (max long term temperature +72 °C and max short term temperature +120 °C)
- III: 40 °C to +160 °C (max long term temperature +100 °C and max short term temperature +160 °C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions
 (zinc coated steel, stainless steel A2 resp. A4 or high corrosion resistant steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel A4 or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist (high corrosion resistant steel).
 - Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to supports, etc.).
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- The anchorages are designed in accordance to EN 1992-4:2018 and Technical Report TR 055

Installation:

- Dry, wet concrete or flooded bore holes (not sea-water).
- Hole drilling by hammer (HD), hollow (HDB) or compressed air drill mode (CD).
- Overhead installation allowed.
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.

Mungo Injection system MIT-Hybrid Plus for concrete	
Intended Use Specifications	Annex B 1



Table B1: Installation parameters for threaded rod											
Anchor size		M 8	M 10	M 12	M 16	M 20	M 24	M 27	M 30		
Diameter of element		$d = d_{nom}$	[mm]	8	10	12	16	20	24	27	30
Nominal drill hole dia	ameter	d ₀	[mm]	10	12	14	18	22	28	30	35
		h _{ef,min}	[mm]	60	60	70	80	90	96	108	120
Effective embedmer	h _{ef,max}	[mm]	160	200	240	320	400	480	540	600	
Diameter of	Prepositioned i	nstallation d _f	[mm]	9	12	14	18	22	26	30	33
clearance hole in the fixture ¹⁾	Push through installation d _f		[mm]	12	14	16	20	24	30	33	40
Maximum torque mo	ment	T _{inst} ≤	[Nm]	10	20	40 ²⁾	60	100	170	250	300
Minimum thickness of member		h _{min}	[mm]	h _{ef} + 30 mm ≥ 100 mm			$h_{ef} + 2d_0$				
Minimum spacing S _{min}			[mm]	40	50	60	75	95	115	125	140
Minimum edge dista	nce	c _{min}	[mm]	35	40	45	50	60	65	75	80

Tor application under seismic loading the diameter of clearance hole in the fixture shall be at maximum d₁ + 1mm or alternatively the annular gap between fixture and threaded rod shall be filled force-fit with mortar.
An aximum Torque moment for M12 with steel Grade 4.6 is 35 Nm

Installation parameters for rebar Table B2:

Rebar size				Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
Diameter of element	d = d _{nom}	[mm]	8	10	12	14	16	20	24	25	28	32
Nominal drill hole diameter	d ₀	[mm]	12	14	16	18	20	25	32	32	35	40
[[[]]] [] [] [] [] [] [] []	h _{ef,min}	[mm]	60	60	70	75	80	90	96	100	112	128
Effective embedment depth	h _{ef,max}		160	200	240	280	320	400	480	500	560	640
Minimum thickness of member	h _{min}	[mm]		30 mm 0 mm				h _{ef} +	- 2d ₀			
Minimum spacing	s _{min}	[mm]	40	50	60	70	75	95	120	120	130	150
Minimum edge distance	c _{min}	[mm]	35	40	45	50	50	60	70	70	75	85

Table B3: Installation parameters for Internal threaded rod

Anchor size			IG-M 6	IG-M 16	IG-M 20			
Internal diameter of sleeve	d ₂	[mm]	6	8	10	12	16	20
Outer diameter of sleeve ¹⁾	$d = d_{nom}$	[mm]	10	12	16	20	24	30
Nominal drill hole diameter	d_0	[mm]	12	14	18	22	28	35
Effective embedment depth	h _{ef,min}	[mm]	60	70	80	90	96	120
Enective embedment depth	h _{ef,max}	[mm]	200	240	320	400	480	600
Diameter of clearance hole in the fixture	d _f	[mm]	7	9	12	14	18	22
Maximum torque moment	T _{inst} ≤	[Nm]	10	10	20	40	60	100
Thread engagement length min/max	l _{IG}	[mm]	8/20	8/20	10/25	12/30	16/32	20/40
Minimum thickness of member	h _{min}	[mm]	•	30 mm O mm		h _{ef} +	- 2d ₀	
Minimum spacing	s _{min}	[mm]	50	60	75	95	115	140
Minimum edge distance	c _{min}	[mm]	40	45	50	60	65	80

¹⁾ With metric threads according to EN 1993-1-8:2005+AC:2009

Mungo Injection system MIT-Hybrid Plus for concrete	
Intended Use Installation parameters	Annex B 2



Table B4	: Paran	neter clea	ning and s	etting	g tool	S					
	SCHOOL SCHOOL SERVER		8								
Threaded Rod	Rebar	Internal threaded rod	d ₀ Drill bit - Ø HD, HDB, CA		ь h - Ø	d _{b,min} min. Brush - Ø	Piston plug	Installatio of	n directio piston plu		
[mm]	[mm]	[mm]	[mm]	МІТ-	[mm]	[mm]	МІТ-	1		1	
M8			10	BS10	11,5	10,5					
M10	8	IG-M6	12	BS12	13,5	12,5		No plua	required		
M12	10	IG-M8	14	BS14	15,5	14,5		ino piug	required		
	12		16	BS16	17,5	16,5					
M16	14	IG-M10	18	BS18	20,0	18,5	VS18				
	16		20	BS20	22,0	20,5	VS20				
M20		IG-M12	22	BS22	24,0	22,5	VS22				
	20		25	BS25	27,0	25,5	VS25	h _{ef} >	h _{ef} >		
M24		IG-M16	28	BS28	30,0	28,5	VS28	250 mm	250 mm	all	
M27			30	BS30	31,8	30,5	VS30	230 IIIM	250 mm		
	24 / 25		32	BS32	34,0	32,5	VS32				
M30	28	IG-M20	35	BS35	37,0	35,5	VS35				
	32		40	BS40	43,5	40,5	VS40				





Drill bit diameter (d_0): 10 mm to 20 mm Drill hole depth (h_0): < 10 d

Drill hole depth (h_0) : < 10 d_s Only in non-cracked concrete



CAC - Rec. compressed air tool (min 6 bar)

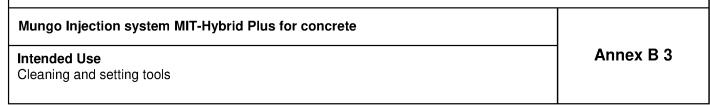
Drill bit diameter (d₀): all diameters



HDB - Hollow drill bit system

Drill bit diameter (d₀): all diameters

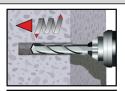
The hollow drill bit system contains the Mungo MHP-Clean / MHX-Clean hollow drill bit and a class M vacuum with minimum negative pressure of 230 hPa and flow rate of minimum 61 l/s.





Installation instructions

Drilling of the bore hole



1a. Hammer (HD) or compressed air drilling (CD) Drill a hole into the base material to the size and embedment depth required by the

In case of aborted drill hole, the drill hole shall be filled with mortar.

selected anchor (Table B1, B2, or B3). Proceed with Step 2.



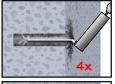
1b. Hollow drill bit system (HDB) (see Annex B 3)

Drill a hole into the base material to the size and embedment depth required by the selected anchor (Table B1, B2, or B3). This drilling system removes the dust and cleans the bore hole during drilling (all conditions). Proceed with Step 3.

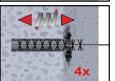
In case of aborted drill hole, the drill hole shall be filled with mortar.

Attention! Standing water in the bore hole must be removed before cleaning.

MAC: Cleaning for dry and wet bore holes with diameter $d_0 \le 20$ mm and bore hole depth $h_0 \le 10d_{nom}$ (uncracked concrete only!)

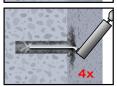


2a. Starting from the bottom or back of the bore hole, blow the hole clean by a hand pump (Annex B 3) a minimum of four times.



2b. Check brush diameter (Table B4). Brush the hole with an appropriate sized wire brush > d_{b,min} (Table B4) a minimum of four times in a twisting motion.

If the bore hole ground is not reached with the brush, a brush extension must be used.

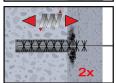


2c. Finally blow the hole clean again with a hand pump (Annex B 3) a minimum of four times.

CAC: Cleaning for dry, wet and water-filled bore holes with all diameter in uncracked and cracked concrete



2a. Starting from the bottom or back of the bore hole, blow the hole clean with compressed air (min. 6 bar) (Annex B 3) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension must be used.



Check brush diameter (Table B4). Brush the hole with an appropriate sized wire brush > d_{b,min} (Table B4) a minimum of two times in a twisting motion.
 If the bore hole ground is not reached with the brush, a brush extension must be used.



2c. Finally blow the hole clean again with compressed air (min. 6 bar) (Annex B 3) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension must be used.

After cleaning, the bore hole has to be protected against re-contamination in an appropriate way, until dispensing the mortar in the bore hole. If necessary, the cleaning has to be repeated directly before dispensing the mortar. In-flowing water must not contaminate the bore hole again.

Mungo Injection system MIT-Hybrid Plus for concrete

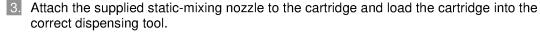
Intended Use
Installation instructions

Annex B 4

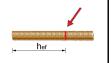


Installation instructions (continuation)

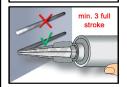




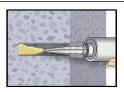
For every working interruption longer than the recommended working time (Table B5) as well as for new cartridges, a new static-mixer shall be used.



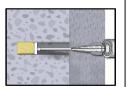
4. Prior to inserting the anchor rod into the filled bore hole, the position of the embedment depth shall be marked on the anchor rods.



5. Prior to dispensing into the anchor hole, squeeze out separately a minimum of three full strokes and discard non-uniformly mixed adhesive components until the mortar shows a consistent grey colour.

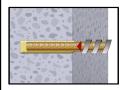


6. Starting from the bottom or back of the cleaned anchor hole, fill the hole up to approximately two-thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid creating air pockets. If the bottom or back of the anchor hole is not reached, an appropriate extension nozzle must be used. Observe the gel-/ working times given in Table B5.

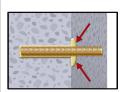


Piston plugs and mixer nozzle extensions shall be used according to Table B4 for the following applications:

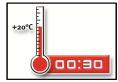
- Horizontal assembly (horizontal direction) and ground erection (vertical downwards direction): Drill bit-Ø d₀ ≥ 18 mm and embedment depth h_{ef} > 250mm
- Overhead assembly (vertical upwards direction): Drill bit-Ø d₀ ≥ 18 mm



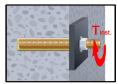
8. Push the threaded rod or reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached. The anchor shall be free of dirt, grease, oil or other foreign material.



9. After inserting the anchor, the annular gab between anchor rod and concrete, in case of a push through installation additionally also the fixture, must be complety filled with mortar. If excess mortar is not visible at the top of the hole, the requirement is not fulfilled and the application has to be renewed. For overhead application the anchor rod shall be fixed (e.g. wedges).



10. Allow the adhesive to cure to the specified time prior to applying any load or torque. Do not move or load the anchor until it is fully cured (attend Table B5).



11. After full curing, the add-on part can be installed with up to the max. torque (Table B1 or B3) by using a calibrated torque wrench. In case of prepositioned installation the annular gab between anchor and fixture can be optioned filled with mortar. Therefor substitute the washer by the filling washer and connect the mixer reduction nozzle to the tip of the mixer. The annular gap is filled with mortar, when mortar oozes out of the washer.

Mungo Injection system MIT-Hybrid Plus for concrete

Intended Use

Installation instructions (continuation)

Annex B 5



Table B5:	Ма	aximum we	orking time and minim	um curing time					
Concrete	temp	erature	Gelling working time	Minimum curing time in dry concrete	Minimum curing time in wet concrete				
- 5 °C	to	- 1 °C	50 min	5 h	10 h				
0 °C	to	+ 4 °C	25 min	3,5 h	7 h				
+ 5 °C	to	+ 9 °C	15 min	2 h	4 h				
+ 10 °C	to	+ 14 °C	10 min	1 h	2 h				
+ 15 °C	to	+ 19 °C	6 min	40 min	80 min				
+ 20 °C	to	+ 29 °C	3 min	30 min	60 min				
+ 30 °C	to	+ 40 °C	2 min	30 min	60 min				
Cartridge	temp	erature	+5°C to +40°C						

Mungo Injection system MIT-Hybrid Plus for concrete	
Intended Use	Annex B 6
Curing time	



Т	Table C1: Characteristic values for steel tension resistance and steel shear resistance of threaded rods										
Si	ze			М 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Cr	oss section area	A _s	[mm²]	36,6	58	84,3	157	245	353	459	561
Cł	naracteristic tension resistance, Steel failu	re 1)	•	•							
	eel, Property class 4.6 and 4.8	N _{Rk,s}	[kN]	15 (13)	23 (21)	34	63	98	141	184	224
St	eel, Property class 5.6 and 5.8	N _{Rk,s}	[kN]	18 (17)	29 (27)	42	78	122	176	230	280
St	eel, Property class 8.8	N _{Rk,s}	[kN]	29 (27)	46 (43)	67	125	196	282	368	449
St	ainless steel A2, A4 and HCR, class 50	N _{Rk,s}	[kN]	18	29	42	79	123	177	230	281
St	ainless steel A2, A4 and HCR, class 70	N _{Rk,s}	[kN]	26	41	59	110	171	247	-	-
	ainless steel A4 and HCR, class 80	N _{Rk,s}	[kN]	29	46	67	126	196	282	-	-
Cł	naracteristic tension resistance, Partial fac	tor ²⁾									
Steel, Property class 4.6 and 5.6 $\gamma_{Ms,N}$ [-] 2,0											
St	eel, Property class 4.8, 5.8 and 8.8	γMs,N	[-]				1,5	5			
St	ainless steel A2, A4 and HCR, class 50	γMs,N	[-]				2,8	6			
St	ainless steel A2, A4 and HCR, class 70	γ _{Ms,N}	[-]				1,8	7			
Stainless steel A4 and HCR, class 80 YMs,N [-] 1,6											
Cł	naracteristic shear resistance, Steel failure										
_	Steel, Property class 4.6 and 4.8	V ⁰ _{Rk,s}	[kN]	9 (8)	14 (13)	20	38	59	85	110	135
arm	Steel, Property class 5.6 and 5.8	V ⁰ Rk,s	[kN]	9 (8)	15 (13)	21	39	61	88	115	140
lever	Steel, Property class 8.8	V ⁰ Rk,s	[kN]	15 (13)	23 (21)	34	63	98	141	184	224
	Stainless steel A2, A4 and HCR, class 50	V ⁰ Rk,s	[kN]	9	15	21	39	61	88	115	140
Without	Stainless steel A2, A4 and HCR, class 70	V ⁰ Rk,s	[kN]	13	20	30	55	86	124	-	-
>	Stainless steel A4 and HCR, class 80	V ⁰ _{Rk,s}	[kN]	15	23	34	63	98	141	-	-
	Steel, Property class 4.6 and 4.8	M ⁰ Rk,s	[Nm]	15 (13)	30 (27)	52	133	260	449	666	900
arm	Steel, Property class 5.6 and 5.8	M ⁰ Rk,s	[Nm]	19 (16)	37 (33)	65	166	324	560	833	1123
	Steel, Property class 8.8	M ⁰ _{Rk,s}	[Nm]	30 (26)	60 (53)	105	266	519	896	1333	1797
Vith lever	Stainless steel A2, A4 and HCR, class 50	M ⁰ Rk.s	[Nm]	19	37	66	167	325	561	832	1125
×	Stainless steel A2, A4 and HCR, class 70	M ⁰ Rk,s	[Nm]	26	52	92	232	454	784	-	-
	Stainless steel A4 and HCR, class 80	M ⁰ Rk,s	[Nm]	30	59	105	266	519	896	-	-
CI	naracteristic shear resistance, Partial facto	r ²⁾									
_	eel, Property class 4.6 and 5.6	γ _{Ms,V}	[-]				1,6	7			
St	eel, Property class 4.8, 5.8 and 8.8	γ _{Ms,V}	[-]				1,2	5			
St	ainless steel A2, A4 and HCR, class 50	γ _{Ms,V}	[-]				2,3	8			
St	ainless steel A2, A4 and HCR, class 70	γ _{Ms,V}	[-]				1,5	6			
St	ainless steel A4 and HCR, class 80	γ _{Ms,V}	[-]				1,3	3			

¹⁾ Values are only valid for the given stress area A_s. Values in brackets are valid for undersized threaded rods with smaller stress area A_s for hot-dip galvanised threaded rods according to EN ISO 10684:2004+AC:2009.
2) in absence of national regulation

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values for steel tension resistance and steel shear resistance of threaded rods	Annex C 1

English translation prepared by DIBt



Table	e C2: Ch	arac	teristic value	es of tension	on loads เ	ınder	stati	c and	l qua	si-sta	tic ac	ction	
Ancho	r size thread	led ro	d			M 8	M 10	M 12	M 16	M 20	M24	M27	M30
Steel fa	ailure									•			
Charac	teristic tensio	n resi	stance	N _{Rk,s}	[kN]			$A_s \cdot f_l$	ık (or s	ee Tab	le C1)		
Partial f	factor			γ _{Ms,N}	[-]	see Table C1							
Combi	ned pull-out	and o	concrete failure	-,									
Charac	teristic bond	resista	ance in non-cracl	ked concrete C	20/25								
iture	ण्डू I: 80°C/50°C Dry		Dry, wet	τ _{Rk,ucr}	[N/mm²]	17	17	16	15	14	13	13	13
Temperature range	II: 120°C/72	2°C	concrete and flooded bore	^τ Rk,ucr	[N/mm²]	15	14	14	13	12	12	11	11
	III: 160°C/1		hole	^τ Rk,ucr	[N/mm²]	12	11	11	10	9,5	9,0	9,0	9,0
Charac	teristic bond	resista	ance in cracked o	concrete C20/2	25								
ature	I: 80°C/50°0	2	Dry, wet	^τ Rk,cr	[N/mm²]	7,0	7,5	8,0	9,0	8,5	7,0	7,0	7,0
Temperature range	II: 120°C/72	2°C	concrete and flooded bore	τ _{Rk,cr}	[N/mm²]	6,0	6,5	7,0	7,5	7,0	6,0	6,0	6,0
Ten	III: 160°C/1	00°C	hole	τ _{Rk,cr}	[N/mm²]	5,5	5,5	6,0	6,5	6,0	5,5	5,5	5,5
				C25/30						02			
			C30/37						04				
1	Increasing factors for concrete		crete	C35/45	· · · · · · · · · · · · · · · · · · ·								
Ψc				C40/50		1,08							
				C45/55									
Concre	ete cone fail	Iro		C50/60	50/60 1,10								
	acked concre			k	Г1				1.	1,0			
		ie		k _{ucr,N}	[-]								
	d concrete			k _{cr,N}	[-]					,7			
Edge di				c _{cr,N}	[mm]					h _{ef}			
Axial di				s _{cr,N}	[mm]				2 0	cr,N			
Splittin	ng	I							4.0				
		n/n _{ef}	≥ 2,0						1,0	h _{ef}			
Edge di	istance	2,0 >	$h/h_{ef} > 1,3$	C _{cr,sp}	[mm]			2 · I	$n_{ef} \bigg(2$	$5 - \frac{h}{h}$	n ef		
		h/h _{ef}	≤ 1,3						2,4	h _{ef}			
Axial di	Axial distance S _{cr,sp} [mm]									cr,sp			
	ation factor			,- -	,	1				- ,- -			
			MAC					1,2			١	NPA	
for dry a	and wet cond	rete	CAC] _{2/2} .	r 1	1,0							
			HDB	γ_{inst}	[-]				1	,2			
for floor	ded bore hole	9	CAC						1	,4			

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values of tension loads under static and quasi-static action	Annex C 2



Anchor size threaded rod			M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30	
Steel failure without lever arm		•		•			•	•			
Characteristic shear resistance Steel, strength class 4.6 and 4.8	V ⁰ Rk,s	[kN]	0,6 • A _s • f _{uk} (or see Table C1)								
Characteristic shear resistance Steel, strength class 5.6, 5.8 and 8.8 Stainless Steel A2, A4 and HCR, all classes	V ⁰ Rk,s	[kN]	0,5 ⋅ A _s ⋅ f _{uk} (or see Table C1)								
Partial factor	γMs,V	[-]	see Table C1								
Ductility factor	k ₇	[-]	1,0								
Steel failure with lever arm											
Characteristic bending moment	M ⁰ Rk,s	[Nm]			1,2 • \	W _{el} • f _{uk}	(or see	Table C	C1)		
Elastic section modulus	W _{el}	[mm³]	31	62	109	277	541	935	1387	1874	
Partial factor	γMs,V	[-]				see	Table C	1		•	
Concrete pry-out failure											
Factor	k ₈	[-]					2,0				
Installation factor	γinst	[-]					1,0				
Concrete edge failure											
Effective length of fastener	I _f	[mm]	min(h _{ef} ; 12 · d _{nom}) min(h _{ef} ; 300n							300mm)	
Outside diameter of fastener	d _{nom}	[mm]	8	10	12	16	20	24	27	30	
Installation factor	γ _{inst}	[-]	1,0								

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values of shear loads under static and quasi-static action	Annex C 3

English translation prepared by DIBt



		istic values	or tell	31011 106		•					
Anchor size internal th	readeo	d anchor rods			IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20	
Steel failure ¹⁾		F 0	N_	FL-N II	10	17	00	40	70	100	
Characteristic tension re Steel, strength class	sistand		N _{Rk,s}	[kN]	10	17	29	42	76	123	
			N _{Rk,s}	[kN]	16	27	46	67	121	196	
Partial factor, strength cl		γMs,N	[-]		<u> </u>	1	,5 I		1		
Characteristic tension resistance, Stainless Steel A4 and HCR, Strength class 70 ²⁾			N _{Rk,s}	[kN]	14	26	41	59	110	124	
Partial factor			γ _{Ms,N}	[-]			1,87			2,86	
Combined pull-out and											
Characteristic bond resis	stance	in non-cracked	concrete	C20/25		ı	1	Γ		ı	
1: 80°C/50°C		Dry, wet	τ _{Rk,ucr}	[N/mm ²]	17	16	15	14	13	13	
III: 120°C/72°C III: 160°C/100°C		concrete and flooded bore	^τ Rk,ucr	[N/mm²]	14	14	13	12	12	11	
		hole	τ _{Rk,ucr}	[N/mm ²]	11	11	10	9,5	9,0	9,0	
Characteristic bond resis	stance	in cracked cond	crete C20)/25	Г	ı		.	T		
1: 80°C/50°C		Dry, wet	τ _{Rk,cr}	[N/mm²]	7,5	8,0	9,0	8,5	7,0	7,0	
III: 120°C/50°C III: 120°C/72°C III: 160°C/100°C		concrete and flooded bore	τ _{Rk,cr}	[N/mm²]	6,5	7,0	7,5	7,0	6,0	6,0	
년 III: 160°C/100°C		hole	τ _{Rk,cr}	[N/mm²]	5,5	6,0	6,5	6,0	5,5	5,5	
				25/30				02			
				30/37				04			
Increasing factors for co	ncrete		C35/45 1,07								
Ψ_{C}			C40/50 1,08 C45/55 1,09								
				50/60				10			
Concrete cone failure							.,				
Non-cracked concrete			k _{ucr,N}	[-]			11	1,0			
Cracked concrete			k _{cr,N}	[-]			7	,7			
Edge distance			c _{cr,N}	[mm]			1,5	h _{ef}			
Axial distance			s _{cr,N}	[mm]			2 c	cr,N			
Splitting failure				'	1.						
	h/h _{ef}	≥ 2,0					1,0	h _{ef}			
Edge distance		h/h _{ef} > 1,3	c _{cr,sp}	[mm]			2 · h _{ef} (2				
h/h _{ef} ≤ 1,3					2,4 h _{ef}						
Axial distance			s _{cr,sp}	[mm]			2 c	cr,sp			
Installation factor		I						Г			
		MAC	4			1,2		NPA			
for dry and wet concrete		CAC	γ _{inst}	[-]				,0			
for flooded bore hole		HDB CAC	1		1,2						
		1		oppropri-t	1,4						

¹⁾ Fastenings (incl. nut and washer) must comply with the appropriate material and property class of the internal threaded rod. The characteristic tension resistance for steel failure is valid for the internal threaded rod and the fastening element.

2) For IG-M20 strength class 50 is valid

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values of tension loads under static and quasi-static action	Annex C 4

English translation prepared by DIBt



Anchor size for internal thread	ed anch	or rods		IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20		
Steel failure without lever arm ¹)										
Characteristic shear resistance,	5.8	V ⁰ _{Rk,s}	[kN]	5	9	15	21	38	61		
Steel, strength class	8.8	V ⁰ _{Rk,s}	[kN]	8	14	23	34	60	98		
Partial factor, strength class 5.8 a	and 8.8	γ _{Ms,V}	[-]	1,25							
Characteristic shear resistance, Stainless Steel A4 and HCR, Strength class 70 ²⁾		V ⁰ Rk,s	[kN]	7	13	20	30	55	40		
Partial factor		γ _{Ms,V}	[-]			1,56			2,38		
Ductility factor		k ₇	[-]	1,0							
Steel failure with lever arm 1)											
Characteristic bending moment, Steel, strength class	5.8	M ⁰ Rk,s	[Nm]	8	19	37	66	167	325		
	8.8	M ⁰ Rk,s	[Nm]	12	30	60	105	267	519		
Partial factor, strength class 5.8 a	and 8.8	γ _{Ms,V}	[-]				1,25				
Characteristic bending moment, Stainless Steel A4 and HCR, Strength class 70 ²⁾		M ⁰ Rk,s	[Nm]	11	26	52	92	233	456		
Partial factor		γ _{Ms,V}	[-]	1,56 2,38					2,38		
Concrete pry-out failure											
Factor		k ₈	[-]				2,0				
Installation factor		γ _{inst}	[-]				1,0				
Concrete edge failure		•	•								
Effective length of fastener			[mm]		min(h _{ef} ; 12 • 0	d _{nom})		min(h _{ef} ; 300mm		
Outside diameter of fastener		d _{nom}	[mm]	10	12	16	20	24	30		
Installation factor		γ _{inst}	[-]	1,0							
			1								

¹⁾ Fastenings (incl. nut and washer) must comply with the appropriate material and property class of the internal threaded rod. The characteristic tension resistance for steel failure is valid for the internal threaded rod and the fastening element.
2) For IG-M20 strength class 50 is valid

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values of shear loads under static and quasi-static action	Annex C 5



	size reinfo	rcing	bar			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 3	
Steel fa	ilure							•				•	•		•	
Charact	eristic tensi	on resi	stance	N _{Rk,s}	[kN]					A _s •	f _{uk} 1)					
Cross s	ection area			A _s	[mm²]	50	79	113	154	201	314	452	491	616	804	
Partial f	actor			γ _{Ms,N}	[-]					1,	4 ²⁾					
			concrete fail		·											
Charact	eristic bond	l resista	ance in non-d	cracked cor	crete C20/2	25	I	1		I	T	ı				
)	I: 80°C/50°		Dry, wet concrete	^τ Rk,ucr	[N/mm ²]	14	14	14	14	13	13	13	13	13	13	
mperati range	II: 120°C/72		and flooded	^τ Rk,ucr	[N/mm²]	13	12	12	12	12	11	11	11	11	11	
•	III: 160°C/1		bore hole	τ _{Rk,ucr}	[N/mm²]	9,5	9,5	9,5	9,0	9,0	9,0	9,0	9,0	8,5	8,	
			ance in crack	(ed concret)		1	I					Ι		T		
ature e	I: 80°C/50°	C	Dry, wet concrete	^τ Rk,cr	[N/mm²]	5,5	5,5	6,0	6,5	6,5	6,5	6,5	7,0	7,0	7,0	
Temperature range	II: 120°C/72	2°C	and flooded	^τ Rk,cr	[N/mm ²]	4,5	5,0	5,0	5,5	5,5	5,5	5,5	6,0	6,0	6,0	
Tel	III: 160°C/1	00°C	bore hole	^τ Rk,cr	[N/mm ²]							5,0	5,0	5,0		
					25/30	1,02										
					30/37						04					
	ing factors f	or cond	crete		85/45						07					
γ _c					0/50 5/55						80					
					60/60						09 10					
Concre	te cone fail	lure		00	030/00						10					
	cked concr			k _{ucr,N}	[-]					1	1,0					
Cracked	d concrete			k _{cr,N}	[-]					7	7,7					
Edge di	stance			c _{cr,N}	[mm]					1,5	h _{ef}					
Axial dis	stance			s _{cr,N}	[mm]					2 0	cr,N					
Splitting	g															
		h/h _{ef}	≥ 2,0							1,0) h _{ef}					
Edge di	stance	2,0 >	h/h _{ef} > 1,3	c _{cr,sp}	[mm]				2 · h	ef (2	,5 – T	$\left(\frac{h}{r_{ef}}\right)$				
		h/h _{ef} :	≤ 1,3							2,4	h _{ef}					
Axial dis	stance			s _{cr,sp}	[mm]					2 c	cr,sp					
nstalla	tion factor															
			MAC	_				1,2					NPA			
or dry a	and wet con	crete	CAC	γ_{inst}	[-]						<u>,0</u>					
or flood	ded bore ho	ما	HDB CAC	-							,2 ,4					
01 11000			e specificatio							ı	, +					

Performances

Characteristic values of tension loads under static and quasi-static action

Annex C 6

Table C7: Characteristic	values of	shear I	oads	und	er st	atic	and	qua	si-sta	atic ac	tion	
Anchor size reinforcing bar			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
Steel failure without lever arm						•					•	
Characteristic shear resistance	V ⁰ Rk,s	[kN]					0,50	·As	f _{uk} 1)			
Cross section area	A _s	[mm²]	50	79	113	154	201	314	452	491	616	804
Partial factor	γ _{Ms,V}	[-]	1,5 ²⁾									
Ductility factor	k ₇	[-]						1,0				
Steel failure with lever arm		•										
Characteristic bending moment	M ⁰ _{Rk,s}	[Nm]					1.2	w _{el} ·	f _{uk} 1)			
Elastic section modulus	W _{el}	[mm³]	50	98	170	269	402	785	896	1534	2155	3217
Partial factor	γ _{Ms,V}	[-]				•		1,5 ²⁾				
Concrete pry-out failure		•	•									
Factor	k ₈	[-]						2,0				
Installation factor	γ _{inst}	[-]						1,0				
Concrete edge failure	1	'										
Effective length of fastener	I _f	[mm]			min(h _e	_{ef} ; 12 ·	· d _{nom})		min(h _{ef} ; 300	mm)
Outside diameter of fastener	d _{nom}	[mm]	8	10	12	14	16	20	24	25	28	32
Installation factor	γ _{inst}	[-]						1,0			•	

 $[\]stackrel{1)}{\text{s}}$ f_{uk} shall be taken from the specifications of reinforcing bars $\stackrel{2)}{\text{in}}$ in absence of national regulation

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values of shear loads under static and quasi-static action	Annex C 7

Table C8: Displa	acements	under tensior	ı load¹) (threa	aded r	od)				
Anchor size threaded ro	od		M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30
Non-cracked concrete (C20/25 under	static and quasi-	-static ad	ction						
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,031	0,032	0,034	0,037	0,039	0,042	0,044	0,046
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,040	0,042	0,044	0,047	0,051	0,054	0,057	0,060
Temperature range II:	[mm/(N/mm²)]	0,032	0,034	0,035	0,038	0,041	0,044	0,046	0,048	
120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,042	0,044	0,045	0,049	0,053	0,056	0,059	0,062
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,121	0,126	0,131	0,142	0,153	0,163	0,171	0,179
160°C/100°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,124	0,129	0,135	0,146	0,157	0,168	0,176	0,184
Cracked concrete C20/2	25 under stat	ic and quasi-stat	ic action							
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,081	0,083	0,085	0,090	0,095	0,099	0,103	0,106
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,104	0,107	0,110	0,116	0,122	0,128	0,133	0,137
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]	0,084	0,086	0,088	0,093	0,098	0,103	0,107	0,110
120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,108	0,111	0,114	0,121	0,127	0,133	0,138	0,143
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,312	0,321	0,330	0,349	0,367	0,385	0,399	0,412
160°C/100°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,321	0,330	0,340	0,358	0,377	0,396	0,410	0,424

¹⁾ Calculation of the displacement

 $\delta_{N0} = \delta_{N0} \text{-factor } \cdot \tau;$

τ: action bond stress for tension

 $\delta_{N_{\infty}} = \delta_{N_{\infty}} \text{-factor } \cdot \tau;$

Table C9: Displacements under shear load²⁾ (threaded rod)

Anchor size threade	ed rod	М 8	M 10	M 12	M 16	M 20	M24	M 27	M 30		
Non-cracked and cracked concrete C20/25 under static and quasi-static action											
All temperature	δ_{V0} -factor	[mm/kN]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03	
ranges	$\delta_{V_{\infty}}$ -factor	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05	

²⁾ Calculation of the displacement

 $\delta_{V0} = \delta_{V0}\text{-factor} \ \cdot \ V;$

V: action shear load

 $\delta_{V_{\infty}} = \delta_{V_{\infty}}\text{-factor} + V;$

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Displacements under static and quasi-static action (threaded rods)	Annex C 8



Table C10: Displa	cements u	nder tension	load ¹⁾ (Ir	nternal t	hreaded	rod)		
Anchor size Internal thre	eaded rod		IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20
Non-cracked concrete C	20/25 under s	tatic and quasi-s	tatic actio	n	•			
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,032	0,034	0,037	0,039	0,042	0,046
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,042	0,044	0,047	0,051	0,054	0,060
Temperature range II:	δ _{N0} -factor	[mm/(N/mm²)]	0,034	0,035	0,038	0,041	0,044	0,048
120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,044	0,045	0,049	0,053	0,056	0,062
Temperature range III:	δ _{N0} -factor	[mm/(N/mm²)]	0,126	0,131	0,142	0,153	0,163	0,179
160°C/100°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,129	0,135	0,146	0,157	0,168	0,184
Cracked concrete C20/2	5 under static	and quasi-static	action					
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,083	0,085	0,090	0,095	0,099	0,106
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,170	0,110	0,116	0,122	0,128	0,137
Temperature range II:	δ _{N0} -factor	[mm/(N/mm²)]	0,086	0,088	0,093	0,098	0,103	0,110
120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,111	0,114	0,121	0,127	0,133	0,143
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,321	0,330	0,349	0,367	0,385	0,412
160°C/100°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,330	0,340	0,358	0,377	0,396	0,424

¹⁾ Calculation of the displacement

 $\delta_{N0} = \delta_{N0}$ -factor $\cdot \tau$; $\delta_{N_{\infty}} = \delta_{N_{\infty}}$ -factor τ ; τ: action bond stress for tension

Table C11: Displacements under shear load²⁾ (Internal threaded rod)

Anchor size Inter	nal threaded rod		IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20			
Non-cracked and cracked concrete C20/25 under static and quasi-static action											
All temperature	δ _{V0} -factor	[mm/kN]	0,07	0,06	0,06	0,05	0,04	0,04			
ranges	$\delta_{V_{\infty}}$ -factor	[mm/kN]	0,10	0,09	0,08	0,08	0,06	0,06			

²⁾ Calculation of the displacement

$$\begin{split} &\delta_{V0} = \delta_{V0}\text{-factor} & \cdot V; \\ &\delta_{V\infty} = \delta_{V\infty}\text{-factor} & \cdot V; \end{split}$$

V: action shear load

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Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Displacements under static and quasi-static action (Internal threaded anchor rod)	Annex C 9



Table C12:	Table C12: Displacements under tension load ¹⁾ (rebar)											
Anchor size reinfo	orcing bar		Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
Non-cracked concrete C20/25 under static and quasi-static action												
Temperature δ_{N0} -factor [mm/(N/mm²)] 0,031 0,032 0,034 0,035 0,037 0,039 0,042 0,043 0,045 0,048												
range I: 80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,040	0,042	0,044	0,045	0,047	0,051	0,054	0,055	0,058	0,063
Temperature	δ_{N0} -factor	[mm/(N/mm²)]	0,032	0,034	0,035	0,036	0,038	0,041	0,044	0,045	0,047	0,050
range II: 120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,042	0,044	0,045	0,047	0,049	0,053	0,056	0,057	0,060	0,065
Temperature	δ_{N0} -factor	[mm/(N/mm²)]	0,121	0,126	0,131	0,137	0,142	0,153	0,163	0,164	0,172	0,186
range III: 160°C/100°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,124	0,129	0,135	0,141	0,146	0,157	0,168	0,169	0,177	0,192
Cracked concrete	C20/25 und	er static and qu	asi-stat	ic actic	n							
Temperature	δ_{N0} -factor	[mm/(N/mm²)]	0,081	0,083	0,085	0,087	0,090	0,095	0,099	0,099	0,103	0,108
range I: 80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,104	0,107	0,110	0,113	0,116	0,122	0,128	0,128	0,133	0,141
Temperature	δ_{N0} -factor	[mm/(N/mm²)]	0,084	0,086	0,088	0,090	0,093	0,098	0,103	0,103	0,107	0,113
range II: 120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,108	0,111	0,114	0,118	0,121	0,127	0,133	0,133	0,138	0,148
Temperature	δ_{N0} -factor	[mm/(N/mm²)]	0,312	0,321	0,330	0,340	0,349	0,367	0,385	0,385	0,399	0,425
range III: 160°C/100°C	$\delta_{N\infty}$ -factor	[mm/(N/mm ²)]	0,321	0,330	0,340	0,349	0,358	0,377	0,396	0,396	0,410	0,449

¹⁾ Calculation of the displacement

τ: action bond stress for tension

$$\begin{split} &\delta_{\text{N0}} = \delta_{\text{N0}}\text{-factor} &\cdot \tau; \\ &\delta_{\text{N}_{\infty}} = \delta_{\text{N}_{\infty}}\text{-factor} &\cdot \tau; \end{split}$$

Displacements under shear load²⁾ (rebar) Table C13:

Anchor size rein	Anchor size reinforcing bar				Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
For concrete C20/25 under static and quasi-static action												
All temperature	δ_{V0} -factor	[mm/kN]	0,06	0,05	0,05	0,04	0,04	0,04	0,03	0,03	0,03	0,03
ranges	$\delta_{V\infty}$ -factor	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05	0,04	0,04

²⁾ Calculation of the displacement

$$\begin{split} &\delta_{V0} = \delta_{V0}\text{-factor} & \cdot V; \\ &\delta_{V\infty} = \delta_{V\infty}\text{-factor} & \cdot V; \end{split}$$
V: action shear load

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances	Annex C 10
Displacements under static and quasi-static action (rebar)	



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Tabl			eristic value ance catego			unde	r seis	mic a	ction				
Ancho	r size thread	led rod				М 8	M 10	M 12	M 16	M 20	M24	M27	M30
Steel f				_			•						
Charad (Seism	cteristic tensionic C1)	on resist	ance	N _{Rk,s,eq,C1}	[kN]	1,0 • N _{Rk,s}							
(Seism Steel, s Stainle	cteristic tension nic C2) strength class sss Steel A4 a th class ≥70	s 8.8	·	N _{Rk,s,eq,C2}	[kN]	N	PA		1,0 •	$N_{Rk,s}$		NF	PA
Partial factor				γ _{Ms,N}	[-]				see Ta	ble C1			
Combined pull-out and concrete failure													
Charac	cteristic bond	resistan	ce in cracked a	nd non-cracke	d concrete (C20/25	<u> </u>						
⊕ I: 80°C/50°C				^τ Rk,eq,C1	[N/mm ²]	7,0	7,5	8,0	9,0	8,5	7,0	7,0	7,0
rang	1. 60 0/50 0		Dry, wet concrete and	^τ Rk,eq,C2	[N/mm²]	N	PA	3,6	3,5	3,3	2,3	NF	PA
nre	II. 10000/70	D0C		^τ Rk,eq,C1	[N/mm ²]	6,0	6,5	7,0	7,5	7,0	6,0	6,0	6,0
Temperature range	II: 120°C/72	2.0	flooded bore hole	τ _{Rk,eq,C2}	[N/mm ²]	N	PA	3,1	3,0	2,8	2,0	NF	PA
emp	III: 160°C/100°C		TIOIC	^τ Rk,eq,C1	[N/mm²]	5,5	5,5	6,0	6,5	6,0	5,5	5,5	5,5
<u> </u>	III. 100 O/ I	00 0		τ _{Rk,eq,C2}	[N/mm²]	NPA		2,5 2,7		2,5	1,8	NF	PA
Increas	sing factors fo	or concre	ete ψ _C	C25/30 to	C50/60	1,0							
Concr	ete cone fail	ure											
Non-cr	acked concre	ete		k _{ucr,N}	[-]				11	,0			
Cracke	ed concrete			k _{cr,N}	[-]					,7			
Edge o	distance			c _{cr,N}	[mm]				1,5	h _{ef}			
Axial d	istance			s _{cr,N}	[mm]				2 c	cr,N			
Splittii	ng												
		h/h _{ef} ≥	2,0						1,0	h _{ef}			
Edge o	listance	2,0 > h	/h _{ef} > 1,3	C _{cr,sp}	[mm]			2 · h	ef (2,	$5-\frac{1}{h}$	h ef		
		h/h _{ef} ≤	1,3	1					2,4	h _{ef}			
Axial d	xial distance			s _{cr,sp}	[mm]	_							
Install	nstallation factor			, , ,									
for dry	r dry and wet concrete					1,0							
for dry and wet concrete HDB			γinst	[-]	1,2								

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values of tension loads under seismic action (performance category C1+C2)	Annex C 11

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for flooded bore hole



Anchor size threaded rod			М 8	M 10	M 12	M 16	M 20	M24	M 27	М 30		
Steel failure without lever arm								l				
Characteristic shear resistance (Seismic C1)	V _{Rk,s,eq,C1}	[kN]	0,70 • V ⁰ _{Rk,s}									
Characteristic shear resistance (Seismic C2), Steel, strength class 8.8 Stainless Steel A4 and HCR, Strength class ≥70	[kN]	N	PA		0,70 •	V ⁰ Rk,s		NI	PA			
Partial factor	$\gamma_{Ms,V}$	[-]				see	Table C	C1				
Ductility factor	k ₇	[-]					1,0					
Steel failure with lever arm												
	M ⁰ Rk,s,eq,C1	[Nm]			No Pe	rforman	ce Asse	essed (N	IPA)			
Characteristic bending moment	M ⁰ _{Rk,s,eq,C2}	[Nm]			No Pe	rforman	ce Asse	essed (N	IPA)			
Concrete pry-out failure												
Factor	k ₈	[-]					2,0					
Installation factor	γinst	[-]					1,0					
Concrete edge failure	·											
Effective length of fastener	If	[mm]		n	nin(h _{ef} ;	12 • d _{no}	m)		min(h _{ef} ;	300mm)		
Outside diameter of fastener	d _{nom}	[mm]	8	10	12	16	20	24	27	30		
Installation factor	γinst	[-]					1,0					
Factor for annular gap $\alpha_{\rm gap}$ [-] $0.5 (1.0)^{1)}$												

¹⁾ Value in brackets valid for filled annular gab between anchor and clearance hole in the fixture. Use of special filling washer Annex A 3 is required

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values of shear loads under seismic action (performance category C1+C2)	Annex C 12



Table			teristic va nance cat			oads	und	er se	ismi	c act	ion				
Ancho	r size reinfo	orcing I	oar			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
Steel fa	ailure				_										
Charac	teristic tensi	on resi	stance	N _{Rk,s,eq}	[kN]	$1.0 \cdot A_s \cdot f_{uk}^{1)}$									
Cross s	section area			A_s	[mm²]	50	79	113	154	201	314	452	491	616	804
Partial factor $\gamma_{Ms,N}$ [-]										1,	4 ²⁾				
Combi	ned pull-ou	t and c	oncrete fail	ure											
Charac	teristic bond	l resista	ince in crack	ed and non-	cracked co	ncrete	C20/2	25							
g I: 80°C/50°C Dry, wet				^τ Rk,eq	[N/mm²]	5,5	5,5	6,0	6,5	6,5	6,5	6,5	7,0	7,0	7,0
Dry, wet concrete and flooded bore hole				^τ Rk,eq	[N/mm²]	4,5	5,0	5,0	5,5	5,5	5,5	5,5	6,0	6,0	6,0
III: 160°C/100°C			τ _{Rk,eq}	[N/mm²]	4,0	4,5	4,5	5,0	5,0	5,0	5,0	5,0	5,0	5,0	
Increas	sing factors f	or conc	rete ψ _C	C25/30 to	C50/60					1	,0			•	
Concre	ete cone fail	lure													
Non-cra	acked concr	ete		k _{ucr,N}	[-]	11,0									
Cracke	d concrete			k _{cr,N}	[-]	7,7									
Edge d	listance			c _{cr,N}	[mm]					1,5	h _{ef}				
Axial di	istance			s _{cr,N}	[mm]					2 c	cr,N				
Splittir	ng			· ·	•						·				
		h/h _{ef} ≥	2,0							1,0	h _{ef}				
Edge d	Edge distance $2.0 > h/h_{ef} > 1.3$ c				[mm]				2 · h	ef (2	,5 – -	h of			
	h/h _{ef} ≤ 1,3									2,4	h _{ef}				
Axial distance S _{cr,sp} [mn						_									
Installa	Installation factor														
for dry	and wet con	γ _{inst}	[-]	1,0 1,2											
for floo	ded bore ho	le	HDB CAC	1,11121	LJ						, <u> </u>				

 $[\]stackrel{1)}{\rm f}_{\rm uk}$ shall be taken from the specifications of reinforcing bars $\stackrel{2)}{\rm in}$ absence of national regulation

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values of tension loads under seismic action (performance category C1)	Annex C 13



Table C17: Characteristic (performance			oads	und	er se	eism	ic ac	tion						
Anchor size reinforcing bar			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32		
Steel failure without lever arm				•	•			•						
Characteristic shear resistance	V _{Rk,s,eq}	[kN]	0,35 • A _s • f _{uk} ¹⁾											
Cross section area	A _s	[mm²]	50	79	113	154	201	314	452	491	616	804		
Partial factor $\gamma_{Ms,V}$ [-] $1,5^{2)}$														
Ductility factor	k ₇	[-]						1,0						
Steel failure with lever arm														
Characteristic bending moment	M ⁰ _{Rk,s,eq}	[Nm]			N	o Perf	ormar	nce As	sesse	d (NPA))			
Concrete pry-out failure	•													
Factor	k ₈	[-]						2,0						
Installation factor	γinst	[-]						1,0						
Concrete edge failure		•												
Effective length of fastener	I _f	[mm]		1	min(h _e	_{ef} ; 12 ·	· d _{nom}	1)		min(h _{ef} ; 300	mm)		
Outside diameter of fastener	d _{nom}	[mm]	8	10	12	14	16	20	24	25	28	32		
Installation factor	γinst	[-]						1,0						
Factor for annular gap	Factor for annular gap $\alpha_{\rm gap}$ [-] 0,5 (1,0) ³⁾													

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Characteristic values of shear loads under seismic action (performance category C1)	Annex C 14

¹⁾ f_{uk} shall be taken from the specifications of reinforcing bars
2) in absence of national regulation
3) Value in brackets valid for filled annular gab between anchor and clearance hole in the fixture. Use of special filling washer Annex A 3 is required

English translation prepared by DIBt



Table C18: Display	Table C18: Displacements under tension load (threaded rod)													
Anchor size threaded ro	od		M 8	M 10	M 12	M 16	M 20	M24	M 27	M 30				
Cracked concrete C20/2	mic C1 action													
Temperature range I:	δ_{N0} -factor	[mm/(N/mm²)]	0,081	0,083	0,085	0,090	0,095	0,099	0,103	0,106				
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,104	0,107	0,110	0,116	0,122	0,128	0,133	0,137				
Temperature range II:	δ_{N0} -factor	[mm/(N/mm²)]	0,084	0,086	0,088	0,093	0,098	0,103	0,107	0,110				
120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,108	0,111	0,114	0,121	0,127	0,133	0,138	0,143				
Temperature range III:	δ_{N0} -factor	[mm/(N/mm²)]	0,312	0,321	0,330	0,349	0,367	0,385	0,399	0,412				
160°C/100°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,321	0,330	0,340	0,358	0,377	0,396	0,410	0,424				

Table C19: Displacements under tension load¹⁾ (rebar)

Anchor size reinfo	Anchor size reinforcing bar			Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
Cracked concrete	Cracked concrete C20/25 under seismic C1 action											
Temperature	δ_{N0} -factor	[mm/(N/mm²)]	0,081	0,083	0,085	0,087	0,090	0,095	0,099	0,099	0,103	0,108
range I: 80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,104	0,107	0,110	0,113	0,116	0,122	0,128	0,128	0,133	0,141
Temperature	δ_{N0} -factor	[mm/(N/mm²)]	0,084	0,086	0,088	0,090	0,093	0,098	0,103	0,103	0,107	0,113
range II: 120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,108	0,111	0,114	0,118	0,121	0,127	0,133	0,133	0,138	0,148
Temperature	δ_{N0} -factor	[mm/(N/mm²)]	0,312	0,321	0,330	0,340	0,349	0,367	0,385	0,385	0,399	0,425
range III: 160°C/100°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,321	0,330	0,340	0,349	0,358	0,377	0,396	0,396	0,410	0,449

¹⁾ Calculation of the displacement

 $\delta_{N0} = \delta_{N0}\text{-factor} \cdot \tau;$

 $\delta_{N_{\infty}} = \delta_{N_{\infty}}$ -factor $\cdot \tau$; (τ : action bond stress for tension)

Table C20: Displacements under shear load²⁾ (threaded rod)

Anchor size thread	М 8	M 10	M 12	M 16	M 20	M24	M 27	M 30		
Non-cracked and c	racked concrete C2	0/25 under seis	mic C1 a	action						
All temperature	δ_{V0} -factor	[mm/kN]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V_{\infty}}$ -factor	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05

Table C21: Displacement under shear load¹⁾ (rebar)

Anchor size reinforcing bar			Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
For concrete C20/25 under seismic C1 action												
All temperature	δ_{V0} -factor	[mm/kN]	0,06	0,05	0,05	0,04	0,04	0,04	0,03	0,03	0,03	0,03
ranges	$\delta_{V\infty}$ -factor	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05	0,04	0,04

²⁾ Calculation of the displacement

$$\begin{split} \delta_{V0} &= \delta_{V0}\text{-factor} \ \cdot \text{V}; \\ \delta_{V\infty} &= \delta_{V\infty}\text{-factor} \ \cdot \text{V}; \ (\text{V: action shear load}) \end{split}$$

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances Displacements under seismic C1 action (threaded rods and rebar)	Annex C 15

English translation prepared by DIBt



Table C22: Displacements under tension load ¹⁾ (threaded rod)										
Anchor size threaded rod				M 10	M 12	M 16	M 20	M24	M 27	M 30
Cracked concrete C20/25 under seismic C2 action										
All temperature $\delta_{N,eq(DLS)}$ -factor [mm/(N/mm ²)]			N	DΛ	0,120	0,100	0,100	0,120	NF	2.4
ranges	$\delta_{N,eq(ULS)}$ -factor	[mm/(N/mm²)]	NPA		0,140	0,150	0,110	0,150	INF	- A

¹⁾ Calculation of the displacement

 $\delta_{\text{N,eq(DLS)}} = \delta_{\text{N,eq(DLS)}} \text{-factor} \cdot \tau;$

 $\delta_{N,eq(ULS)} = \delta_{N,eq(ULS)} \text{-factor} \cdot \tau; \qquad \qquad (\tau: \text{ action bond stress for tension})$

Table C23: Displacements under shear load²⁾ (threaded rod)

Anchor size threaded rod				M 10	M 12	M 16	M 20	M24	M 27	M 30
Cracked concrete C20/25 under seismic C2 action										
All temperature	$\delta_{V,eq(DLS)}$ -factor	[mm/kN]	NIDA	٦,٨	0,27	0,13	0,09 0,06	0,06	NPA	
ranges	$\delta_{V,ep(ULS)}$ -factor	[mm/kN]	- NPA		0,27	0,14	0,10	0,08	INF	-A

²⁾ Calculation of the displacement

 $\delta_{\text{V,eq(DLS)}} = \delta_{\text{V,eq(DLS)}}\text{-factor } \cdot \text{V};$

 $\delta_{V,\text{eq(ULS)}} = \delta_{V,\text{eq(ULS)}}\text{-factor} \quad V; \qquad \text{(V: action shear load)}$

Mungo Injection system MIT-Hybrid Plus for concrete	
Performances	Annex C 16
Displacements under seismic C2 action (threaded rods)	