



Approval body for construction products and types of construction

#### **Bautechnisches Prüfamt**

An institution established by the Federal and Laender Governments



### **European Technical** Assessment

### ETA-18/1109 of 14 January 2019

English translation prepared by DIBt - Original version in German language

#### **General Part**

Technical Assessment Body issuing the Deutsches Institut für Bautechnik European Technical Assessment: Panaria KEIL undercut anchor KH Trade name of the construction product Product family Fastener for the rear fixing of facade panels made of to which the construction product belongs ceramic plates (stoneware) in accordance with EN 14411:2012 Manufacturer Panariagroup Industrie Ceramiche S.p.A. via Panaria bassa n. 22/A 41034 FINALE EMILIA (MO) ITALIEN Manufacturing plant Plant 1 This European Technical Assessment 17 pages including 4 annexes which form an integral part contains of this assessment EAD 330030-00-0601 This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

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#### Specific Part

#### 1 Technical description of the product

The Panaria KEIL undercut anchor KH is a fastener consisting of a crosswise slotted anchor sleeve with an M6 internal thread, at the upper edge of which a hexagon is formed to it and a respective hexagon screw with a tooth lock washer formed to it. The anchor sleeve and the hexagon screw with a tooth lock washer formed to it are made of stainless steel. Instead of the hexagon screw a grub screw or threated rod made of stainless steel may also be used. The anchor is put into an undercut drill hole and by driving-in the screw it is placed form-fitted and deformation-controlled.

The product description is given in Annex A. The material values, dimensions and tolerances of the components of the fastener not indicated in the annexes shall correspond to the values laid down in the technical documentation<sup>1</sup>.

# 2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the fastener is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the fasteners of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

#### 3 Performance of the product and references to the methods used for its assessment

#### 3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance for tension and shear loads	See Annex C 1
Fastener distances and spacing	See Annex C 1
Durability	Corrosion Resistance Class (CRC) III in accordance with EN 1993-1-4:2015

#### 3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance		
Reaction to fire	Class A1		
Resistance to fire	No performance assessed		

1

The technical documentation comprises all information of the holder of this ETA necessary for the production, installation and maintenance of the fastener; these are in particular design drawings. The part to be treated confidentially is deposited with Deutsches Institut für Bautechnik and, as far as this is relevant to the tasks of the approved bodies involved in the procedure of attestation of conformity, shall be handed over to the approved body.



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4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with EAD No. 330030-00-0601 the applicable European legal act is: [97/161/EG]. The system to be applied is: 2+

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

Issued in Berlin on 14 January 2019 by Deutsches Institut für Bautechnik

BD Dipl.-Ing. Andreas Kummerow Head of Department *beglaubigt:* Aksünger

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<sup>1)</sup> elastic sandwich layer (e.g. EPDM) - thickness 1.0 to 3.0 mm

#### Table A1: Dimensions and Materials

Anchor type			KH 5,5	KH 7,0	KH 8,5		
anchorage depth	h <sub>s</sub> =	[mm]	5,5 7,0 8,5				
panel thickness	h≥	[mm]	8,0 9,5 11,0				
diameter of drill hole	d <sub>o</sub> =	[mm]	7,0				
Diameter of undercut	d <sub>1</sub> =	[mm]	9,0				
screw length	с =	[mm]	h <sub>s</sub> + 3mm + t <sub>fix</sub>				
installation torque moment	T <sub>inst</sub>	[Nm]	$2,5 \le T_{inst} \le 4,0$				
Materials			KH 5,5	KH 7,0	KH 8,5		
anchor sleeve			Stainless steel 1.4404 according to EN 10 088:2014				
hexagon screw with tooth lock washer			Stainless steel 1.4401, 1.4404 or 1.4578 according to EN 10 088:2014				

Panaria KEIL undercut anchor KH	ł
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Product description						
Dimensions and materials						

Annex A 2



#### Specifications of intended use

#### Anchorages subject to:

Static and quasi-static loads.

#### **Base materials:**

- The stoneware façade panels shall correspond to the group Al<sub>a</sub>, Al<sub>b</sub>, Bl<sub>a</sub> or Bl<sub>b</sub> according to EN 14411:2012
- Characteristic values of the panel correspond Table B 1
- The façade panels and the constancy of the performance of the fastener shall be checked in follow

#### Classification test (Initial type test)

The following values shall be checked on at least 10 samples:

- bending strength determined according to EN ISO 10545-4:2014-11 with the "visible face" on top; deviating from EN ISO 10 545-4:2014-11 the dimension of the test specimen is l/b = 400/200 mm and the support span is  $l_s = 300$  mm
- axial tension load determined on test specimens with dimensions of l/b = 200/200 mm, an edge distance of 100 mm and a support diameter of Ø = 70 mm; N<sub>U5%</sub> ≥ N<sub>Rk</sub> :Characteristic resistance to tension load according to Table C1
- shear load determined on test specimens with dimensions of I/b = 400/200 mm and an edge distance of 100 mm; V<sub>U5%</sub> ≥ V<sub>Rk</sub> :Characteristic resistance to shear load according to Table C1

#### Acceptance Test (Verification of constancy of performance)

For each construction project the following values shall be checked on at least 10 samples independent of the scope of delivery:

 axial tension load – determined on test specimens with dimensions of I/b = 200/200 mm, an edge distance of 100 mm and a support diameter of Ø = 70 mm; N<sub>U5%</sub> ≥ N<sub>Rk</sub> :Characteristic resistance to tension load according to Table C1

From the test results (Classification and Acceptance tests) the 5%-Fractile (confidence level of 75%, unknown standard deviation and lognormal distribution) shall be determined.

With the determined values of the 5%-Fractile the façade panels are to be classified according to the respective property class corresponding to Table B1.

#### Table B1: Characteristic value of the panel

Strength class of facade panels	Α	В	С		
Bending strength ("visible face" on top)	$\sigma_{u5\%} \geq$	[N/mm²]	35	40	45
Modulus of elasticity	E =	[N/mm²]	30000		
poisson's ratio	ν =	[-]	0,2		
specific weight	γ =	[kN/m <sup>3</sup> ]	25,0		
Panel thickness	h≥	[mm]	8		

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Intended use Specifications

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#### Use conditions (Environmental conditions):

According to EN 1993-1-4:2015 according to the Corrosion Resistance Class of the fastener III

#### Design:

Generals:

- Each façade panel is fixed with at least four anchors in a rectangular arrangement via single agraffes on the substructure (for small panels or small fitted pieces, differential or fill- in pieces the number and position of the anchors shall be chosen constructively).
- Edge distance and spacing shall be observed. For small fitted pieces, differential and fill-in pieces the edge distance and spacing shall be chosen.
- The substructure is constructed such that the façade panels are fixed according to Annex B 7 technically strainfree via skids (loose bearings) and one fixed point (fixed bearing) - the fixed point may be placed at the panel edge or in the panel field - and that there are no additional loads acting on the panels and their fixings due to excentric load application / load transfer (symmetrical bearing of the panels).
- Constraint loads shall be into account for design, If constraint loads exists.
- Two fixing points of the façade panel are designed such that they are able to carry the dead load of the façade panel.
- When using agraffes on horizontal load-bearing profiles the fixing points of a façade panel situated horizontally at the same height are fastened in each case to the same load-bearing profile.
- Verifiable calculation notes and drawings shall be prepared taking account of the loads to be anchored, the nature and strength of the base materials and the dimensions of the anchorage members as well as of the relevant tolerances. The position of the anchor is indicated on the design drawings.
- The calculation shall be carried out in a linear elastic manner. The stiffness of the substructure shall be considered for the respective case of application.
- For hanging panels (overhead installation) or reveals respectively the load direction shall be taken into consideration and the combinations of actions shall be based on EN 1990.

Intended use Specifications

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#### Installation:

- The drillings are done at the factory or on site under workshop conditions; when making the drillings on site the execution is supervised by the responsible project supervisor or a skilled representative of the project supervisor.
  Making of the undercut drilling is done with the drill bit according to Annex B 6 and a special drilling device in
- accordance with the information deposited with Deutsches Institut für Bautechnik.
- In case of aborted hole: new drilling at a minimum distance away of twice the depth of the aborted hole.
- the geometry of the drill hole is checked on 1 % of all drillings. The following dimensions shall be checked and documented according to manufacturer's information and testing instructions by means of a measuring device according to Annex B 6:
  - Volume of the undercut drill hole.
  - Depth position of the undercut; the distance between the lower edge of the measuring device and the façade panel is between 0,0 and 0,3 mm (see Annex B 5).

If the tolerances given in Annex A 2 are exceeded, the geometry of the drill hole shall be checked on 25% of the drillings performed. No further drill hole may exceed the tolerances otherwise all the drill holes shall be controlled. Drilling holes falling below or exceeding the tolerances shall be rejected.

Note: Checking the geometry of the drill hole on 1 % of all drillings means that on one of the 25 panels (this corresponds to 100 drillings in façade panels with four anchors) one drilling shall be checked. If the tolerances given in Annex A 3 are exceeded the extent of the control shall be increase to 25 % of the drillings, i.e. one drilling each shall be checked on all the 25 panels.

- During transport and storage on site the façade panels are protected from damages; the façade panels are not be hung up jerkily (if need be lifters shall be used for hanging up the façade panels); façade panels and reveal panels respectively with incipient cracks are not be installed.
- The façade are installed by skilled specialists and the laying instructions of the manufacturer shall be paid attention to.
- Between agraffe and façade panel an elastic sandwich layer may be placed. (see Annex A 1)
- The façade panels are aranged in a "reclined" or "uprigth" position.
- Overhead installation is allowed.

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Intended use Specifications

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#### Installation instructions

#### 1. Drilling the undercut hole





a) Cylindrical drilling

b) Undercutting



c) Finished undercut hole

#### 2. Checking the undercut hole



With KEIL depth control guide

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#### Intended use Installation instructions

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Table C1:         Characteristic values for the design of the anchor									
Embedment depth		h <sub>s</sub> =	[mm]	5,5	7,0	8,5			
Panel thickness		h≥	[mm]	8,0	9,5	11,0			
Characteristic resistance to tension load	Class A	N <sub>Rk</sub> =	[kN]	1,0	1,5	2,7			
	Class B			1,1	1,6	2,8			
	Class C			1,2	1,7	3,0			
characterisitc resistance to shear load	Class A	V <sub>Rk</sub> =	[kN]	2,0	2,2	2,4			
	Class B			2,1	2,3	2,5			
	Class C			2,2	2,4	2,6			
edge distance 1)		a <sub>r</sub> ≥	[mm]	100					
spacing		<b>a</b> ≥	[mm]	200					

The edge distance may be reduced to 50 mm. For edge distances 50 mm  $\le a_r \le 100$  mm the characteristic values of resistance for shear loads shall be reduced by the factor  $a_r/100$  [ $a_r$  in mm]; in case of different edge distances the smaller value is decisive

1)

Panaria KEIL undercut anchor KH

Annex C 1



#### Characteristic resistance to wind loads for selective panel sizes and bearing conditions

For the panel sizes and bearing conditions given in Table D1 depending on the strength class, panel thickness, setting depth and edge distance the verification of structural stability is deemed to be verified, if the following condition is met:

 $w_{Ed} \le w_{Rk} / \gamma_M$ 

With:

- $w_{Ed}$  = design value of the existing wind load
- $w_{Rk}$  = characteristic resistance to wind loads according to Table D1

 $\gamma_M$  = partial safety factor according to Table D1

# Table D1:characteristic resistance $w_{Rk}$ to wind loads for selective panel sizes and bearing conditions<br/>depending on property class, panel thickness, setting depth and edge distance

Class	h	h <sub>s</sub>	a <sub>rx</sub>	a <sub>ry</sub>	panel sizes	panel sizes bearing condition <sup>1)</sup>		γм
[-]	[mm]	[mm]	[mm]	[mm]	[mm]	[-]	[kN/m²]	[-]
В	≥ 11,5	≥7	60-120	100-200	600  imes 1200	4 Agraffen	5,4	
В	≥ 11,5	≥7	60-120	75-150	600  imes 900	4 Agraffen	8,1	
В	≥ 9,5	≥7	60-120	60-120	600  imes 600	4 Agraffen	10,8	
А	≥ 13	≥ 8,5	123	240-350	900 × 1200	4 Agraffen	2,2	1,8
A	≥ 13	≥ 8,5	123	240-350	900 × 1200	6 Agraffen	2,7	
А	≥ 13	≥ 8,5	123	240-350	900 × 1200	8 Agraffen	4,3	
С	≥ 13	≥7	100	100	900 × 900	4 Agraffen	4,3	

<sup>1)</sup> maximum size of agraffe: width = 30 mm, heigth = 60 mm

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#### Informative

Characteristic resistance to wind loads for selective panel sizes and bearing conditions

Annex D 1



#### Guideline for structural calculation by means of FE - method

For structural calculation by means of the Finite-Element-Method the façade panels are to be idealized with their effective dimensions (size and thickness) as panel elements; the system chosen shall have the capacity to sufficiently precise represent the tension and the deformation state as well as the support reactions of the façade panels. The mesh size at fixing range shall not exceed 10 mm.

The modelling of the façade panel is to be calibrated on the basis of the following points:

- modelling a panel section of 580 mm x 300 mm with a panel thickness of 13,3 mm

- support at the short sides with rotable restraint
- loading at centre with a single load of 1,33 kN
- determination of a factor  $f_{cal.FE}$  = 41,8 /  $\sigma_{FE}$
- the determined bending stresses shall be multiplied with factor  $f_{cal.FE}$  ( $\sigma_{Sk} = \sigma_{FE} \cdot f_{cal.FE}$ ); the factor  $f_{cal.FE}$  shall only be considered for stresses due to support moments



 $\sigma_{FE}$  = maximum main tensile stress [N/mm<sup>2</sup>]

Panaria KEIL Hinterschnittanker KH

#### **Informative** Guideline for structural calculation by means of FE - method

Annex D 2