



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-19/0209 of 14 May 2019

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

Deutsches Institut für Bautechnik

TILCA® TIM U-H Injection system for rebar connection

Systems for post-installed rebar connections with mortar

EFCO Befestigungstechnik AG Grabenstraße 1 8606 NÄNIKON SCHWEIZ

EFCO AG, Plant 2

21 pages including 3 annexes which form an integral part of this assessment

EAD 330087-00-0601



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Z29432.19 8.06.01-98/19



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Specific Part

1 Technical description of the product

The subject of this European Technical Assessment is the post-installed connection, by anchoring or overlap connection joint, of reinforcing bars (rebars) in existing structures made of normal weight concrete, using the "TILCA® TIM U-H Injection System for rebar connection" in accordance with the regulations for reinforced concrete construction.

Reinforcing bars made of steel with a diameter ϕ from 8 to 32 mm or the tension anchor ZA from sizes M12 to M24 according to Annex A and injection mortar TILCA® TIM U-H are used for rebar connections. The rebar is placed into a drilled hole filled with injection mortar and is anchored via the bond between rebar, injection mortar and concrete.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the rebar connection of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance under static and quasi-static loading	See Annex C 1

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Class A1
Resistance to fire	See Annex C 2 and C 3

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with European Assessment Document EAD No. 330087-00-0601, the applicable European legal act is: [96/582/EC].

The system(s) to be applied is (are): 1

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5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

Issued in Berlin on 14 May 2019 by Deutsches Institut für Bautechnik

BD Dipl.-Ing. Andreas Kummerow Head of Department

beglaubigt: Baderschneider

Z29432.19 8.06.01-98/19



Installation post installed rebar

Figure A1: Overlapping joint for rebar connections of slabs and beams

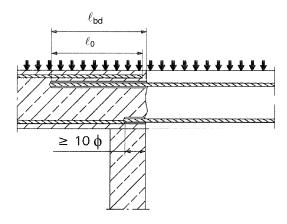


Figure A3: End anchoring of slabs or beams

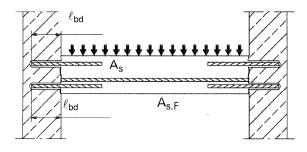


Figure A5: Anchoring of reinforcement to cover the line of acting tensile force

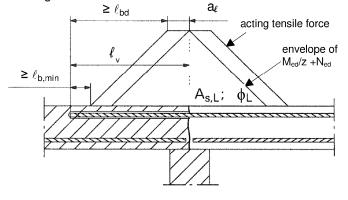


Figure A2: Overlapping joint at a foundation of a wall or column where the rebars are stressed in tension

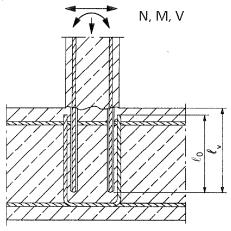
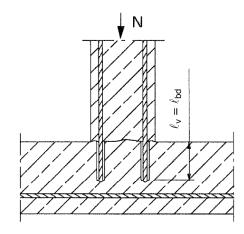


Figure A4: Rebar connection for components stressed primarily in compression. The rebars sre stressed in compression



Note to Figure A1 to A5:

In the Figures no transverse reinforcement is plotted, the transverse reinforcement shall comply with EN 1992-1-1:2004+AC:2010.

Preparing of joints according to Annex B 2

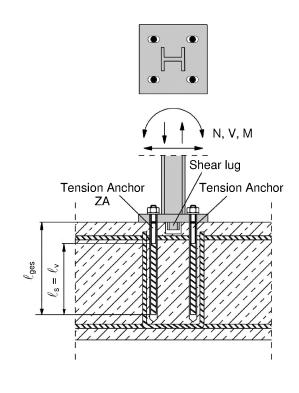
TILCA® TIM U-H Injection System for rebar connection Product description Installed condition and examples of use for rebars Annex A 1



Installation tension anchor ZA

Figure A6: Overlapping joint of a column stressed in bending to a foundation

Figure A7: Overlap joint for the anchorage of barrier posts



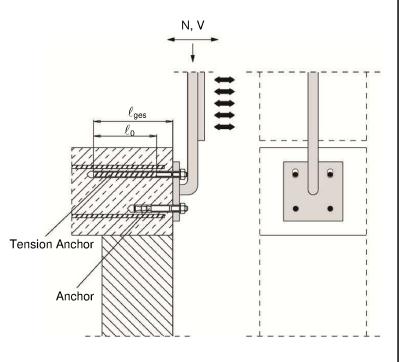
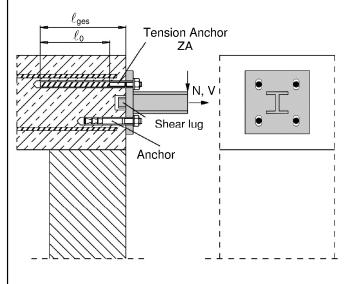


Figure A8: Overlap joint for the anchorage to centilever members



Note to Figure A6 to A8:

In the Figures no transverse reinforcement is plotted, the transverse reinforcement shall comply with EN 1992-1-1:2002+AC:2010

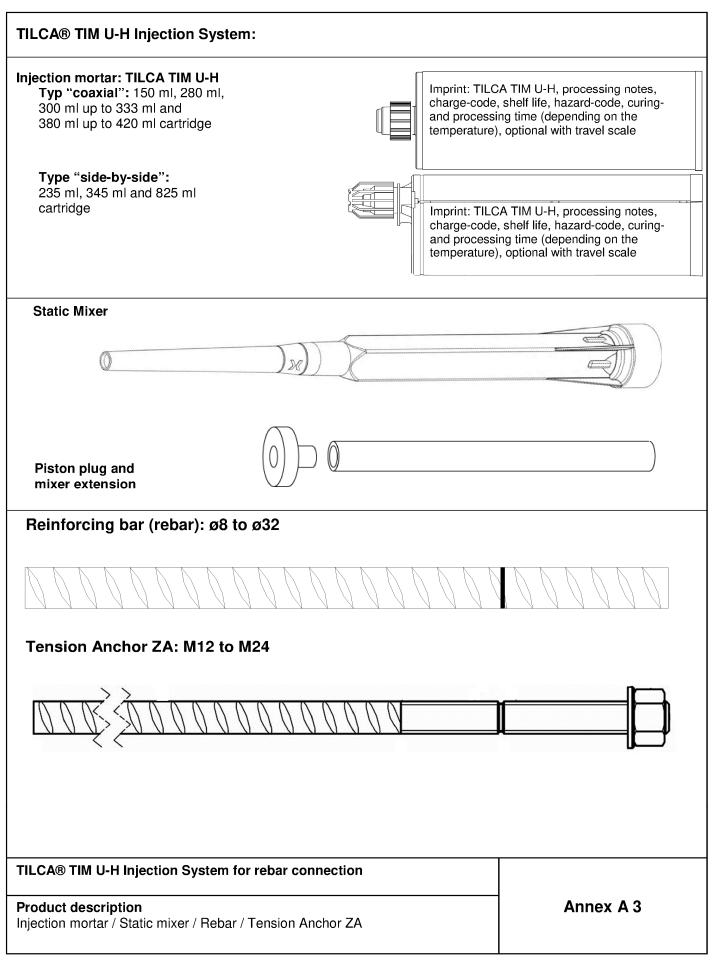
TILCA® TIM U-H Injection System for rebar connection

Product description

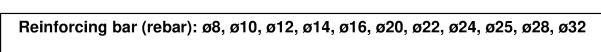
Installed condition and examples of use for tension anchors ZA

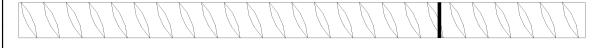
Annex A 2











- Minimum value of related rip area f_{R,min} according to EN 1992-1-1:2004+AC:2010
- Rib height of the bar shall be in the range 0,05φ ≤ h ≤ 0,07φ
 (φ: Nominal diameter of the bar; h: Rip height of the bar)

Table A1: Materials

electronic copy of the eta by dibt: eta-19/0209

Designation	Material		
Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C f_{yk} and k according to NDP or NCL of EN 1992-1-1/NA $f_{uk} = f_{tk} = k \cdot f_{yk}$		

TILCA® TIM U-H Injection System for rebar connection

Product description
Specifications Rebar

Annex A 4



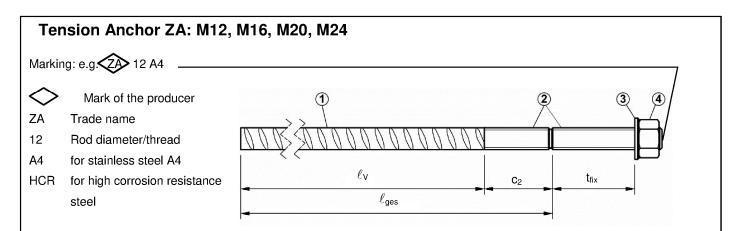


Table A2: Materials

		Material											
Part	Designation	ZA vz			ZA A4			ZA HCR					
		M12	M16	M20	M24	M12	M16	M20	M24	M12	M16	M20	M24
1	Reinforcement bar		Class B according to NDP or NCL of EN 1992-1-1/NA										
2	Threaded rod	to EN	zinc pla 10087:1 263:200	998 or	ording	1.4401		l, 1.4362 4, 1.457 014		steel,	orrosior 1.4529, 088-1:2	1.4565,	nt
	f _{yk} [N/mm²]		64	40			640		560		640		560
3	Washer		Steel, zinc plated according						High corrosion resistant				
4	Nut	1	to EN 10087:1998 or EN 10263:2001			1.4401, 1.4404, 1.4571, EN 10088-1:2014		1,	steel, 1.4529, 1.4565, EN 10088-1:2014				

Table A3: Dimensions and installation parameter

Size				ZA-M12	ZA-M16	ZA-M20	ZA-M24
Diameter of threaded rod			[mm]	12	16	20	24
Diameter of reinfo	rcement bar		[mm]	12	16	20	25
Drill hole diameter			[mm]	16	20	25	32
Diameter of cleara	nce hole in fixture		[mm]	14	18	22	26
With across nut fla	With across nut flats SV			19	24	30	36
Stress area		As	[mm²]] 84 157 245 353			353
Effective embedm	ent depth	$\ell_{ m v}$	[mm]		according to st	atic calculation	
Length of bonded	plated		c ₂ [mm] -	≥ 20	≥ 20	≥ 20	≥ 20
thread	A4/HCR	C ₂		≥ 100	≥ 100	≥ 100	≥ 100
Minimum thickness of fixture		t _{fix}	[mm]	5	5	5	5
Maximum thickness of fixture			[mm]	3000	3000	3000	3000
Maximum installation torque			[Nm]	50	100	150	150

TILCA® TIM U-H Injection System for rebar connection	
Product description Specifications Tension Anchor ZA	Annex A 5



Specifications of intended use

Anchorages subject to:

- · Static and quasi-static loads.
- Fire exposure

Base materials:

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000.
- Strength classes C12/15 to C50/60 according to EN 206-1:2000.
- Maximum chloride concrete of 0,40% (CL 0.40) related to the cement content according to EN 206-1:2000.
- Non-carbonated concrete.

Note: In case of a carbonated surface of the existing concrete structure the carbonated layer shall be removed in the area of the post-installed rebar connection with a diameter of ϕ + 60 mm prior to the installation of the new rebar.

The depth of concrete to be removed shall correspond to at least the minimum concrete cover in accordance with EN 1992-1-1:2004+AC:2010.

The foregoing may be neglected if building components are new and not carbonated and if building components are in dry conditions.

Temperature Range:

• - 40°C to +80°C (max. short term temperature +80°C and max long term temperature +50°C).

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions or subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist
 - (stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist (high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- · Verifiable calculation notes and drawings are prepared taking account of the forces to be transmitted.
- Design according to EN 1992-1-1:2004+AC:2010, EN 1992-1-2:2004+AC:2008 and Annex B 2 and B 3.
- The actual position of the reinforcement in the existing structure shall be determined on the basis of the construction documentation and taken into account when designing.

Installation:

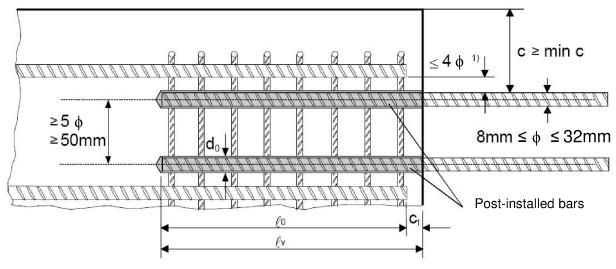
- · Dry or wet concrete.
- It must not be installed in flooded holes.
- Overhead installation allowed.
- · Hole drilling by hammer drill (HD) hollow drill (HDB) or compressed air drill mode (CD).
- The installation of post-installed rebar resp. tension anchors shall be done only by suitable trained installer and under supervision on site; the conditions under which an installer may be considered as suitable trained and the conditions for supervision on site are up to the Member States in which the installation is done.
- Check the position of the existing rebars (if the position of existing rebars is not known, it shall be determined using a rebar detector suitable for this purpose as well as on the basis of the construction documentation and then marked on the building component for the overlap joint).

TILCA® TIM U-H Injection System for rebar connection	
Intended use Specifications	Annex B 1



Figure B1: General construction rules for post-installed rebars

- · Only tension forces in the axis of the rebar may be transmitted
- The transfer of shear forces between new concrete and existing structure shall be designed additionally according to EN 1992-1-1:2004+AC:2010.
- The joints for concreting must be roughened to at least such an extent that aggregate protrude.



If the clear distance between lapped bars exceeds 4φ, then the lap length shall be increased by the difference between the clear bar distance and 4φ.

The following applies to Figure B1:

c concrete cover of post-installed rebar
concrete cover at end-face of existing rebar
min c minimum concrete cover according to Table R1 and to

min c minimum concrete cover according to Table B1 and to EN 1992-1-1:2004+AC:2010, Section 4.4.1.2

diameter of post-installed rebar

 ℓ_0 lap length, according to EN 1992-1-1:2004+AC:2010, Section 8.7.3

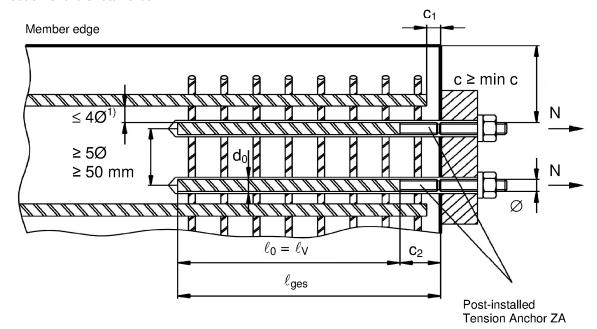
 $\begin{array}{ll} \ell_v & \text{effective embedment depth,} \geq \ell_0 + c_1 \\ d_0 & \text{nominal drill bit diameter, see Annex B 6} \end{array}$

TILCA® TIM U-H Injection System for rebar connection	
Intended use General construction rules for post-installed rebars	Annex B 2



Figure B2: General construction rules for tension anchors ZA

- · The length of the bonded-in thread may be not be accounted as anchorage
- Only tension forces in the direction of the bar axis may be transmitted by the tension anchor ZA
- · The tension force must be transferred via an overlap joint to the reinforcement in the building part.
- The transfer of shear forces shall be ensured by appropriate additional measures, e.g shear lugs or by anchors with an European technical assessment.
- In the anchor plate, the holes for the tension anchors shall be executed as elongated holes with axis in the direction of the shear force.



If the clear distance between lapped bars exceeds 4φ, then the lap length shall be increased by the difference between the clear bar distance and 4φ.

The following applies to Figure B2:

c concrete cover of tension anchor ZA

concrete cover at end-face of existing rebar

 c_2 Length of bonded thread

min c minimum concrete cover according to Table B1 and to EN 1992-1-1:2004+AC:2010, Section 4.4.1.2

φ diameter of tension anchor

 ℓ_0 lap length, according to EN 1992-1-1:2004+AC:2010, Section 8.7.3

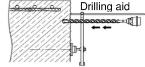
 ℓ_{v} effective embedment depth, $\geq \ell_{0} + c_{1}$ ℓ_{des} overall embedment depth, $\geq \ell_{0} + c_{2}$

d₀ nominal drill bit diameter, see Annex B 6

TILCA® TIM U-H Injection System for rebar connection	
Intended use General construction rules for tension anchors	Annex B 3



Table B1: Minimum concrete cover min c¹⁾ of post-installed rebar and tension anchor ZA depending of drilling method



Drilling method	Rebar diameter	Without drilling aid	With drilling aid
Hammer drilling (HD) Hollow drill bit system (HDB)	< 25 mm	30 mm + 0,06 · ℓ_{v} ≥ 2 ϕ	30 mm + 0,02 · ℓ_{v} ≥ 2 ϕ
	≥ 25 mm	40 mm + 0,06 · ℓ_{v} ≥ 2 ϕ	40 mm + 0,02 · ℓ_{v} ≥ 2 ϕ
Compressed air drilling (CD)	< 25 mm	50 mm + 0,08 · ℓ _v	50 mm + 0,02 · ℓ _v
Compressed air drilling (CD)	≥ 25 mm	60 mm + 0,08 · l _v	60 mm + 0,02 · l _v

see Annex B2, Figures B1 and Annex B3, Figure B2 Comments: The minimum concrete cover acc. EN 1992-1-1:2004+AC:2010 must be observed

Table B2: maximum embedment depth $\ell_{v,max}$

Rebar	Tension anchor	/ [mm]
ф	ф	$\ell_{ m v,max}$ [mm]
8 mm		1000
10 mm		1000
12 mm	ZA-M12	1000 ¹⁾ / 1200
14 mm		1000 ¹⁾ / 1400
16 mm	ZA-M16	1000 ¹⁾ / 1600
20 mm	ZA-M20	1000 ¹⁾ / 2000
22 mm		1000 ¹⁾ / 2000
24 mm		1000 ¹⁾ / 2000
25 mm	ZA-M24	1000 ¹⁾ / 2000
28 mm		1000 ¹⁾ / 2000
32 mm		1000 ¹⁾ / 2000

¹⁾ maximum embedment depth for use with hollow drill bit system (HDB)

Table B3: Base material temperature, gelling time and curing time

Concrete	Concrete temperature		Concrete temperature		Gelling working time ¹⁾	Minimum curing time in dry concrete	Minimum curing time in wet concrete
- 5 °C	to	- 1 °C	50 min	5 h	10 h		
0 °C	to	+ 4 °C	25 min	3,5 h	7 h		
+ 5 °C	to	+ 9 °C	15 min	2 h	4 h		
+ 10 °C	to	+ 14 °C	10 min	1 h	2 h		
+ 15 °C	to	+ 19 °C	6 min	40 min	60 min		
+ 20 °C	to	+ 29 °C	3 min	30 min	60 min		
+ 30 °C	to	+ 40 °C	2 min	30 min	60 min		
Cartridge	temp	perature	+5°C to +40°C				

¹⁾ t_{ael}: maximum time from starting of mortar injection to completing of rebar setting.

TILCA® TIM U-H Injection System for rebar connection	
Intended use Minimum concrete cover, maximum embedment depth, working time and curing times	Annex B 4



	Hai	Pneumatic tool	
Coaxial cartridges 150, 280, 300 up to 333 ml	e.g. Type F	e.g. Type TS 492 X	
Coaxial cartridges 380 up to 420 ml	e.g. Type CCM 380/10	e.g. Type H 285 or H244C	e.g. Type TS 485 LX
Side-by-side cartridges 235, 345 ml	e.g. Type CBM 330A	e.g. Type H 260	e.g. Type TS 477 LX
Side-by-side cartridge 825 ml	e.g. Type OBW 330A	e.g. Type 11 200	e.g. Type TS 498X
	em contains the Heller Duste	er Expert hollow drill bit and of 230 hPa <u>and</u> flow rate of	DUSTER.
Brush RB:	L	SDS Plus Ad	apter:
	***************************************	d _b	
Brush extension:		d _b	
Brush extension:		d _b	
	(volume 750 ml)	Rec. compresse hand slide valve	



Table B5:	Brushes, piston plugs, max anchorage depth and mixer extension, hammer
	(HD) and compressed air (CD) drilling

1	'	,	and	. 00p	. 0000	a an (c	b) aiii	9									
Bar	Tension bit - Ø			4	Ь	d _{b,min}	Piston			idge: sizes		side	tridge: -by-side 25 ml)				
size	anchor	DIT	- 60		h - Ø	Brush -	1		or battery tool	Pneu	matic tool	Pneur	matic tool				
Ф	ф	HD	CD					$I_{v,max}$	Mixer extension	I _{v,max}	Mixer extension	$I_{v,max}$	Mixer extension				
[mm]	[mm]	[m	m]		[mm]	[mm]		[mm]		[mm]		[mm]					
8		12	-	RB12	13,5	12,5	-			800		800	VI 10/0 75				
10		14	-	RB14	15,5	14,5	IA14 IA16 700					1000	VL10/0,75				
12	ZA-M12	1	6	RB16	17,5	16,5				1000		1200					
14		1	8	RB18	20,0	18,5	IA18			1000		1400					
16	ZA-M16	2	0	RB20	22,0	20,5	IA20							1600			
20	ZA-M20	25	-	RB25	27,0	25,5	IA25		VL10/0,75				VI 10/0 75	VI 10/0 75	VL10/0,75		
	ZA-IVIZU	-	26	RB26	28,0	26,5	IA25	IA25	IA25	IA25	IA25	VL10/0,73	700	VL10/0,73		VL16/1,8	
22		2	8	RB28	30,0	28,5	IA28					2000	VL10/1,0				
24		3	2	RB32	34,0	32,5	IA32	500	00								
25	ZA-M24	3	2	RB32	34,0	32,5	IA32			500							
28		3	5	RB35	37,0	35,5	IA35			500		2000					
32		4	0	RB40	43,5	40,5	IA40					2000					

Table B6: Brushes, piston plugs, max anchorage depth and mixer extension, hammer drilling with hollow drill bit system (HDB)

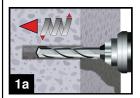
Bar	Tension	Drill	d _b	d _{b,min}	Piston			idge: sizes		side	rtridge: -by-side 25 ml)		
size	anchor	bit - Ø	Brush - Ø	Brush -	Brush -	plug		or battery tool	Pneu	matic tool	Pneui	matic tool	
ф	Ф	HDB		Ø		I _{v,max}	Mixer extension	I _{v,max}	Mixer extension	$I_{v,max}$	Mixer extension		
[mm]	[mm]	[mm]				[mm]		[mm]		[mm]			
8		12			-			800		800	VL10/0,75		
10		14			IA14			1000		1000	VL10/0,73		
12	ZA-M12	16			IA16	700				1000			
14		18	No cleaning IA20 required IA25		1000		1000)					
16	ZA-M16	20					IA20					1000	
20	ZA-M20	25		IA25		VL10/0,75	700	VL10/0,75					
22		28			IA28			700		1000	VL16/1,8		
24		32			IA32	500				1000			
25	ZA-M24	32			IA32	300		500					
28		35		IA35		500	500		10				
32		40			IA40					1000			

TILCA® TIM U-H Injection System for rebar connection	
Intended Use Parameter brushes, piston plugs, max anchorage depth and mixer extension	Annex B 6



A) Bore hole drilling

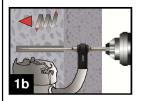
Note: Before drilling, remove carbonated concrete and clean contact areas (see Annex B1) In case of aborted drill hole: the drill hole shall be filled with mortar.



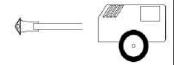
1a. Hammer (HD) or compressed air drilling (CD)
Drill a hole into the base material to the size and
embedment depth required by the selected reinforcing bar
with carbide hammer drill (HD) or a compressed air drill
(CD). Proceed with Step 2.



Hammer drill (HD + HDB)



1b. Hollow drill bit system (HDB) (see Annex B 5) Drill a hole into the base material to the size and embedment depth required by the selected reinforcing bar. This drilling system removes the dust and cleans the bore hole during drilling. Proceed with Step 3.



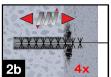
Compressed air drill (CD)

B) Bore hole cleaning

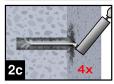
MAC: Cleaning for bore hole diameter $d_0 \le 20$ mm and bore hole depth $h_0 \le 10d_s$



2a. Starting from the bottom or back of the bore hole, blow the hole clean a hand pump (Annex B 7) a minimum of four times.



2b. Check brush diameter (Table B5). Brush the hole with an appropriate sized wire brush > d_{b,min} (Table B5) a minimum of four times in a twisting motion.
If the bore hole ground is not reached with the brush, a brush extension shall be used.

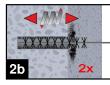


2c. Finally blow the hole clean again with a hand pump (Annex B 7) a minimum of four times.

CAC: Cleaning for all bore hole diameter and bore hole depth



2a. Starting from the bottom or back of the bore hole, blow the hole clean with compressed air (min. 6 bar) (Annex B 7) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension shall be used.



2b. Check brush diameter (Table B5). Brush the hole with an appropriate sized wire brush > d_{b,min} (Table B5) a minimum of two times. If the bore hole ground is not reached with the brush, a brush extension shall be used (Table B5).

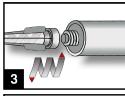


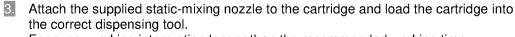
2c. Finally blow the hole clean again with compressed air (min. 6 bar) (Annex B 7) a minimum of two times until return air stream is free of noticeable dust. If the bore hole ground is not reached an extension shall be used.

TH CAR TIM	II-H Injection	System for i	ebar connection
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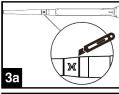
Intended Use Installation instruction Annex B 7

C) Preparation of bar and cartridge

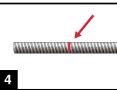




For every working interruption longer than the recommended working time (Table B3) as well as for every new cartridges, a new static-mixer shall be used.

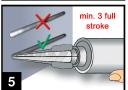


3a. In case of using the mixer extension VL16/1,8, the tip of the mixer nozzle has to be cut off at position "X".



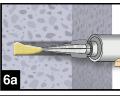
4. Prior to inserting the reinforcing bar into the filled bore hole, the position of the embedment depth shall be marked (e.g. with tape) on the reinforcing bar and insert bar in empty hole to verify hole and depth $\ell_{\rm v}$.

The reinforcing bar should be free of dirt, grease, oil or other foreign material.

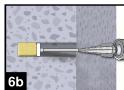


5. Prior to dispensing into the anchor hole, squeeze out separately the mortar until it shows a consistent grey colour, but a minimum of three full strokes, and discard non-uniformly mixed adhesive components.

D) Filling the bore hole

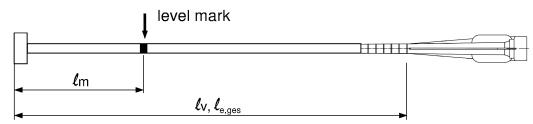


Starting from the bottom or back of the cleaned anchor hole fill the hole up to approximately two-thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid creating air pockets. For embedment larger than 190 mm an extension nozzle shall be used.



For overhead and horizontal installation and bore holes deeper than 240 mm a piston plug and the appropriate mixer extension must be used.

Observe the gel-/ working times given in Table B3.



Injection tool must be marked by mortar level mark ${m \ell}_{\!\!\! m}$ and anchorage depth ${m \ell}_{\!\!\! v}$ resp. ${m \ell}_{\!\!\! e,ges}$ with tape or marker.

Quick estimation: $\ell_{\,\mathrm{m}} = 1/3 \cdot \ell_{\,\mathrm{v}}$ Continue injection until the mortar level mark $\ell_{\,\!\!\mathrm{m}}$ becomes visible.

Optimum mortar volume: $\ell_{\rm m} = \ell_{\rm v} {\rm resp.} \ \ell_{\rm e,ges} \cdot \left(1,2 \cdot \frac{\varphi^2}{d_0^2} - 0,2 \right) \ [mm]$

TILCA® TIM U-H Injection System for rebar connection

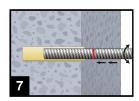
Intended Use

Installation instruction

Annex B 8

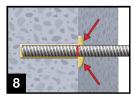
electronic copy of the eta by dibt: eta-19/0209

E) Inserting the rebar

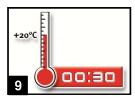


Push the reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached.

The bar should be free of dirt, grease, oil or other foreign material.



Be sure that the bar is inserted in the bore hole until the embedment mark is at the concrete surface and that excess mortar is visible at the top of the hole. If these requirements are not maintained, the application has to be renewed. For overhead installation fix embedded part (e.g. wedges).



Observe gelling time t_{gel}. Attend that the gelling time can vary according to the base material temperature (see Table B3). It is not allowed to move the bar after geling time t_{gel} has elapsed.
Allow the adhesive to sure to the appearing and the adhesive to the sure to the appearing and the adhesive to the sure to the appearing and the adhesive to the sure to the appearing and the adhesive to the appearing and the adhesive to the adhesive to the appearing and the adhesive to the adhesive tof the adhesive to the adhesive to the adhesive to the adhesive to

the adhesive to cure to the specified time prior to applying any load. Do not move or load the bar until it is fully cured (attend Table B3). After full curing time \mathbf{t}_{cure} has elapsed, the add-on part can be installed.

TILCA® TIM U-H Injection System for rebar connection

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Installation instruction

Annex B 9



Minimum anchorage length and minimum lap length

The minimum anchorage length $\ell_{b,min}$ and the minimum lap length $\ell_{0,min}$ according to EN 1992-1-1:2004+AC:2010 ($\ell_{b,min}$ acc. to Eq. 8.6 and Eq. 8.7 and $\ell_{0,min}$ acc. to Eq. 8.11) shall be multiply by the amplification factor α_{lb} according to Table C1.

Table C1: Amplification factor α_{lb} related to concrete class and drilling method

Concrete class	Drilling method	Bar size	Amplification factor α _{lb}	
C12/15 to C50/60	All drilling method	8 mm to 32 mm ZA-M12 to ZA-M24	1,0	

Table C2: Reduction factor k_b for all drilling methods

Rebar - Ø		Concrete class									
ф	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60		
8 to 32 mm ZA-M12 to ZA-M24					1,0						

Table C3: Design values of the ultimate bond stress f_{bd,PIR} in N/mm² for all drilling methods and for good conditions

 $f_{bd,PIR} = k_b \cdot f_{bd}$

with

 f_{bd} : Design value of the ultimate bond stress in N/mm² considering the concrete classes and the rebar diameter according to EN 1992-1-1:2004+AC:2010.

(for all other bond conditions multiply the values by 0.7)

k_b: Reduction factor according to Table C2

Rebar - Ø		Concrete class									
ф	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60		
8 to 32 mm ZA-M12 to ZA-M24	1,6	2,0	2,3	2,7	3,0	3,4	3,7	4,0	4,3		

TILCA® TIM U-H Injection System for rebar connection	
Performances	Annex C 1
Amplification factor α _{lb}	
Design values of ultimate bond resistance f _{bd,PIR}	



Design value of the ultimate bond stress $f_{bd,fi}$ under fire exposure for concrete classes C12/15 to C50/60, (all drilling methods):

The design value of the bond stress f_{bd,fi} under fire exposure has to be calculated by the following equation:

$$f_{bd,fi} = k_{fi}(\theta) \cdot f_{bd,PIR} \cdot \gamma_c / \gamma_{M,fi}$$

with: $\theta \le 364^{\circ}\text{C}$: $k_{\text{fi}}(\theta) = 30,34 \cdot e^{(\theta \cdot -0,011)} / (f_{\text{bd,PIR}} \cdot 4,3) \le 1,0$

 $\theta > 364^{\circ}C$: $k_{fi}(\theta) = 0$

 $f_{bd.fi}$ Design value of the ultimate bond stress in case of fire in N/mm²

 θ Temperature in °C in the mortar layer.

 $k_{fi}(\theta)$ Reduction factor under fire exposure.

f_{bd,PIR} Design value of the ultimate bond stress in N/mm² in cold condition according to Table C3

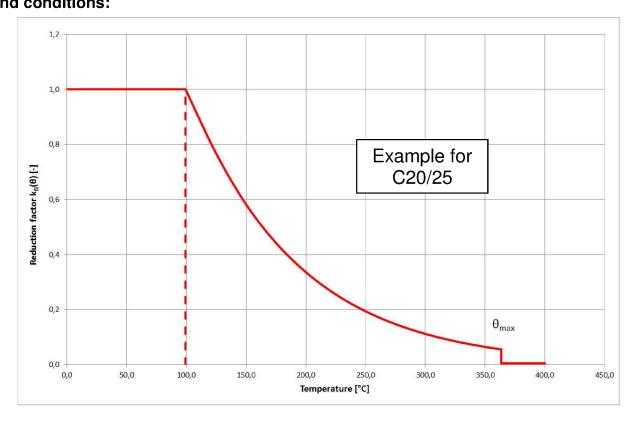
considering the concrete classes, the rebar diameter and the bond conditions according to

EN 1992-1-1:2004+AC:2010.

 γ_c partially safety factor according to EN 1992-1-1:2004+AC:2010 partially safety factor according to EN 1992-1-2:2004+AC:2008

 $\gamma_{M,fi}$ partially safety factor according to EN 1992-1-2:2004+AC:2008 For evidence under fire exposure the anchorage length shall be calculated according to EN 1992-1-1:2004+AC:2010 Equation 8.3 using the temperature-dependent ultimate bond stress $f_{bd,fi}$.

Example graph of Reduction factor $k_{fi}(\theta)$ for concrete classes C20/25 for good bond conditions:



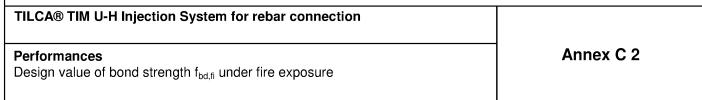




Table C4: Characteristic tension strength for tension anchor ZA under fire exposure,

concrete classes C12/15 to C50/60, according to Technical Report TR 020

Tension Anchor				M12	M16	M20	M24			
Steel, zinc plated	(ZA vz)									
	R30			20						
Characteristic	R60	_	[N/mm²]		15					
steel strength	R90	$oldsymbol{\sigma}_{Rk,s,fi}$	[N/mm²]		1	3				
	R120			10						
Stainless Steel (Z	A A4 or Z	A HCR)								
	R30				3	0				
Characteristic	R60	_	[N/mm²]		2	5				
steel strength	R90	$oldsymbol{\sigma}_{Rk,s,fi}$			2	0				
	R120				1	6				

Design value of the steel strength $\sigma_{\scriptscriptstyle Rd,s,fi}$ under fire exposure

The design value of the steel strength $\sigma_{\text{Rd,s,fi}}$ under fire exposure has to be calculated by the following equation:

$$\sigma_{\text{Rd,s,fi}} = \sigma_{\text{Rk,s,fi}} \, / \, \gamma_{\text{M,fi}}$$

with:

 $\sigma_{Rk,s,fi}$ characteristic steel strength according to Table C4

 $\gamma_{M,fi}$ partially safety factor according to EN 1992-1-2:2004+AC:2008

TILCA® TIM U-H Injection System for rebar connection	
Performances	Annex C 3
Design value of the steel strength $\sigma_{\text{Rd,s,fi}}$ for tension anchor ZA under fire exposure	