



Approval body for construction products and types of construction

Bautechnisches Prüfamt

An institution established by the Federal and Laender Governments



European Technical Assessment

ETA-20/0227 of 13 November 2020

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of Deutsches Institut für Bautechnik

Primero KEIL undercut anchor KH

Anchor for the rear fixing of facade panels made of selected natural stones according to EN 1469:2015

Primero-Schiefer GmbH Neeskotten 5 51688 Wipperfürth DEUTSCHLAND

Plant 1

29 pages including 4 annexes which form an integral part of this assessment

EAD 330030-00-0601 Edition 10/2018

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Specific Part

1 Technical description of the product

The Primero KEIL undercut anchor KH is a special anchor made of stainless steel consisting of a crosswise slotted anchor sleeve with an M6 internal thread, at the upper edge of which a hexagon is formed to it and a respective hexagon bolt with an integrated tooth lock washer as well as distance washer for levelling of thickness tolerances $\Delta d_p = +6 \text{ mm} / \pm 0 \text{ mm}$. Alternatively, instead of the hexagon bolt with an integrated tooth lock washer, a threaded pin or threaded rod is used. The anchor is put into an undercut drill hole and by driving-in the screw it is placed form-fitted and deformation-controlled.

The product description is given in Annex A. The material values, dimensions and tolerances of the components of the fastener not indicated in the annexes shall correspond to the values laid down in the technical documentation¹.

2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the fastener is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the fasteners of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance to breakout or pull-out failure under tension load	See Annex C 1 and C 2
Characteristic resistance to breakout or pull-out failure under shear load	See Annex C 1 and C 2
Characteristic resistance to breakout or pull-out failure under combined tension and shear load	See Annex C 1 and C 2
Edge distance and spacing	See Annex C 1
Durability	Corrosion Resistance Class (CRC) III in accordance with EN 1993-1-4:2015
Characteristic resistance to steel failure under tension and shear loads	See Annex C 2

The technical documentation comprises all information of the holder of this ETA necessary for the production, installation and maintenance of the fastener; these are in particular design drawings. The part to be treated confidentially is deposited with Deutsches Institut für Bautechnik and, as far as this is relevant to the tasks of the approved bodies involved in the procedure of attestation of conformity, shall be handed over to the approved body.

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3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance		
Reaction to fire	Class A1		

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with EAD No. 330030-00-0601 the applicable European legal act is: [97/161/EG]. The system to be applied is: 2+

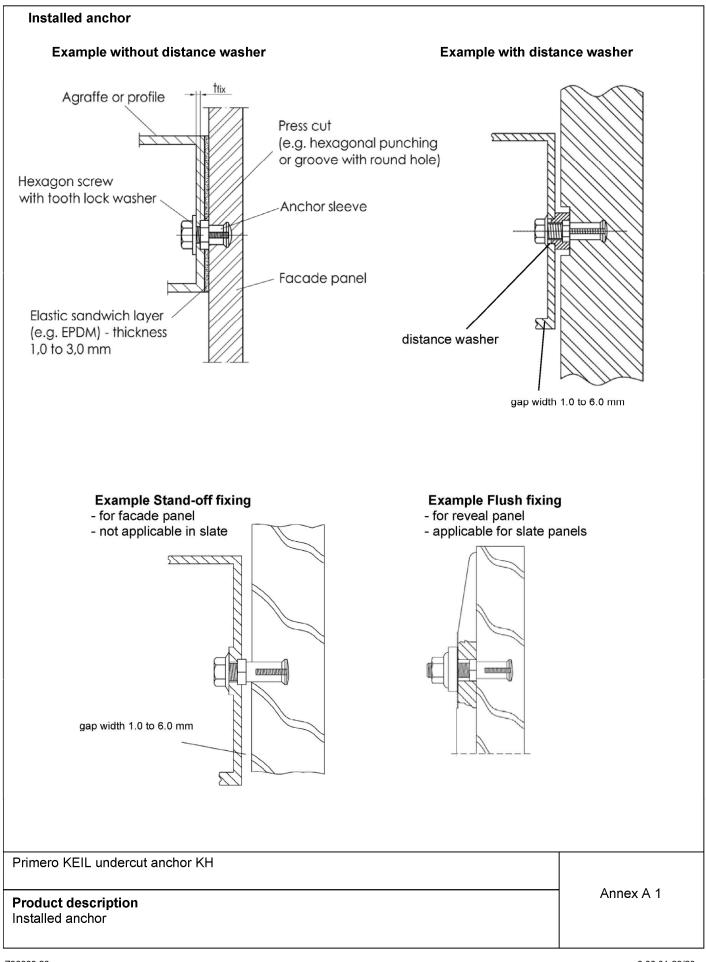
5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

Issued in Berlin on 13 November 2020 by Deutsches Institut für Bautechnik

Dipl.-Ing. Beatrix Wittstock Head of Section *beglaubigt:* Aksünger

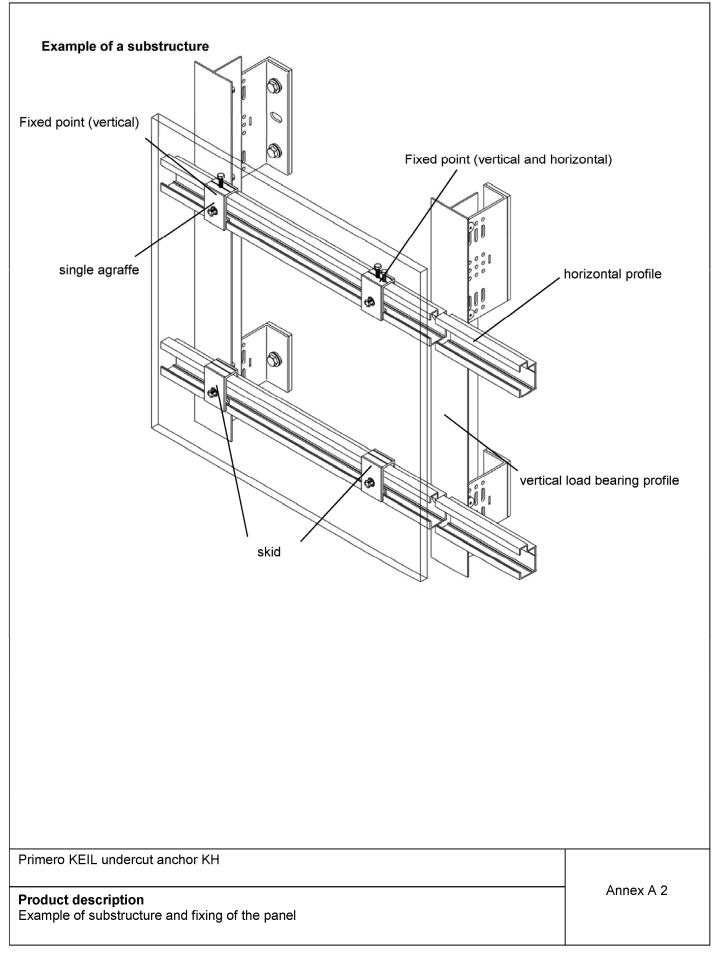




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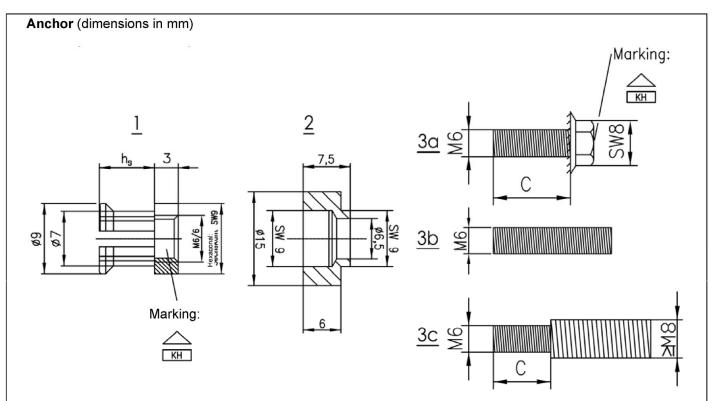




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c: The screw length must be adjusted to the respective design

Anchor type				KH 7	KH 10	KH 15	
anchora	age depth	h _s =	[mm]	7,0	10	15	
screw le	ength	c =	[mm]		h _s + 3mm + t _{fix}		
installat	ion torque moment	T _{inst}	[Nm]		$2,5 \leq T_{inst} \leq 4,0$)	
Materials							
1	anchor sleeve stainle			tainless steel 1.4404 according to EN 10 088:2014			
2	2 washer			aluminum 3.1645/ EN AW-2007 according to EN 573-3:2013-12			
			stainless steel 1.4401, 1.4404 or 1.4578 according to EN 10 088:2014				
3b	threaded pin			s steel 1.4401, 1 988:2014	.4404 or 1.4578	according to	
3c	threaded bold			s steel 1.4401, 1 88:2014	.4404 or 1.4578	according to	

Table A1: Dimensions and Materials

Primero KEIL undercut anchor KH

Product description Example of substructure and fixing of the panel

Annex A 3



Specifications of intended use

Anchorages subject to:

Static and quasi-static loads.

Base materials:

- Facade panels made of natural stone according EN 1469:2015
- · Natural stone free of fractures, mechanically effective cracks and aging.
- · Natural stone according to Table B1
- · Parameters of facade panel according to Table B2 to B4

Table B1: Natural stone group

Group of stone		Natural stone	Conditions	
I High-quality intrusive rocks die		granite, granitite, syenite, tonalite, diorite, monzonite, gabbro other magmatic plutonic rocks	none	
II	Metamorphic rocks with "hard stone character"	quarzite, granulite, gneiss, migmatite, slate ¹	slate type: only slates according to Table B2 and Table B3	
111	High-quality extrusive rocks (volcanic rocks)	basalt and basaltlava damaging ingredients (see Sonnenbrennerbasalt) may not existent	Density: basalt: ρ ≥ 2,7 kg/dm³ basaltlava: ρ ≥ 2,2 kg/dm³	
IV	Sedimentary rocks with "hard stone character" ^{3,4}	sandstone and limestone	sandstone: ρ ≥ 2,1 kg/dm³	

For façade panels made of natural stones with planes of anisotropy, the difference between the bending strengths determined parallel to the planes of anisotropy and perpendicular to the edges of the planes of anisotropy shall not be more than 50 %.

1

Primero KEIL undercut anchor KH

Intended use Specifications



Table B2: Characteristic values of anchors, façade, reveal panel and soffit made of natural stone							
Natural stone exceed slate							
Panel thickness		h _{nom} [mm]	20 (30) $^{1)} \le h_{nom} \le 70$				
Maximum size of	panel	$A \leq [m^2]$	3,0				
Maximum side ler	ngth	H or $L \leq [m]$	3,0				
Number of ancho	rs (rectangular arrangement)	[-]	4				
Anchorage depth		h₅ = [mm]	10 or 15				
Nominal diameter of drill hole		\varnothing d ₀ = [mm]	7				
Edge distance of	anchor	a _r = [mm]	50 mm ≤ a _r ≤ 0,25L or 0,25H				
Spacing of anchor	r for reveal panel	b _r = [mm]	40 mm \leq b _r = 0,2H or 0,2L				
Spacing		a ≥ [mm]	8 hs				
Sorow longth	without distance washer	o _ [mm] _	h _s + 3 mm + t _{fix}				
Screw length	with distance washer	c = [mm]	h _s + 7,5 mm + t _{fix}				
Remaining wall th	ickness ²⁾	$R \ge [mm]$	0,4 h _{norn}				
	Epprechtstein yellow	σ _{5%} ≥ [N/mm²]	15,6				
Bending stress	Padang light	σ _{5%} ≥ [N/mm²]	10,3				
	Sto-Kilzinger sandstone	$\sigma_{5\%} \ge [N/mm^2]$	4,3				

¹⁾ for sandstone, limestone and basaltlava: panel thickness $h_{nom} \ge 30$ mm, if from the panel manufacturer warrented lowest expect-value (5% fractile) of the bending tensile strength is < 8 N/mm².

2) only stand-off fixing

 Table B3: Characteristic values of anchors, façade, reveal panel and soffit made of slate CS 50, SIN 120 or SIN 150

Slate		CS 50	SIN 120	SIN 150
Panel thickness	h _{nom} ≥ [mm]		10	
Maximum size of panel	$A \leq [m^2]$		1,0	
Maximum side length	H oder L \leq [m]		1,2	
Number of anchors (rectangular arrangement)	[-]	4 or 6	4	4
Density	$\gamma = [kN/m^3]$		28,0	
E-Modulus	$E = [N/mm^2]$	130000	120000	90000
Bending stress	σ _{5%} ≥ [N/mm²]	40	25	30

Primero KEIL undercut anchor KH

Intended use Specifications



Table B4: Characteristic values of anchors, façade, reveal panel and soffit made of slate Primero Vulcano Schie						
Slate						
Panel thickness	h _{nom} ≥ [mm]	20				
Number of anchors (rectangular arrangement)	[-]	4				
Density	γ = [kN/m³]	28				
E-Modulus	E,mean = [N/mm²]	65 000				
Bending stress	σ _{5%} ≥ [N/mm²]	35,0				

Use conditions (Environmental conditions):

• In accordance with EN 1993-1-4:2015 dependent on Corrosion Resistancy Class (see ETA section 3.1).

Design:

- The facade anchorage must be dimensioned for the respective application under the responsibility of an engineer experienced in the field of facade construction.
- · Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored.
- Anchorages under static or quasi-static loading are designed in accordance with: EOTA Technical Report TR062 "Design of fasteners for facade panels made of natural stone".

Installation:

- The drillings are done at the factory or on site under workshop conditions; when making the drillings on site the execution is supervised by the responsible project supervisor or a skilled representative of the project supervisor.
- Making of the undercut drilling is done with the drill bit according to Annex B 5 and a special drilling device in accordance with the information deposited with Deutsches Institut für Bautechnik.
- The drill dust must be removed from the borehole.
- In case of aborted hole: new drilling at a minimum distance away of twice the depth of the aborted hole.
- the geometry of the drill hole is checked on 1 % of all drillings. The following dimensions shall be checked and documented according to manufacturer's information and testing instructions by means of a measuring device according to Annex B 5:
 - Volume of the undercut drill hole
 - Depth position of the undercut; the distance between the lower edge of the measuring device and the façade panel is between 0,0 and 0,3 mm (see Annex B 5).

If the tolerances are exceeded, the geometry of the drill hole shall be checked on 25% of the drillings performed. No further drill hole may exceed the tolerances otherwise all the drill holes shall be controlled. Drilling holes falling below or exceeding the tolerances shall be rejected.

Note: Checking the geometry of the drill hole on 1 % of all drillings means that on one of the 25 panels (this corresponds to 100 drillings in façade panels with four anchors) one drilling shall be checked. If the tolerances given in Annex B 22 are exceeded the extent of the control shall be increase to 25 % of the drillings, i.e. one drilling each shall be checked on all the 25 panels.

- During transport and storage on site the façade panels are protected from damages; the façade panels are not be hung up jerkily (if need be lifters shall be used for hanging up the façade panels); façade panels and reveal panels respectively with incipient cracks are not be installed.
- · Between agraffe and façade panel an elastic sandwich layer may be placed.
- Overhead installation is allowed

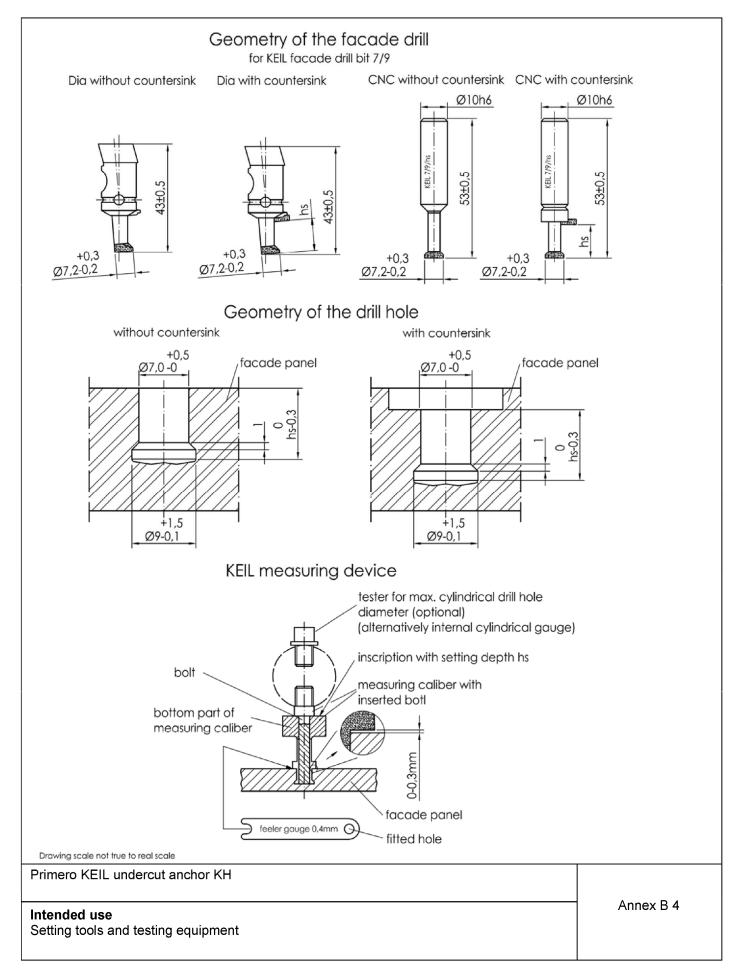
Primero KEIL undercut anchor KH

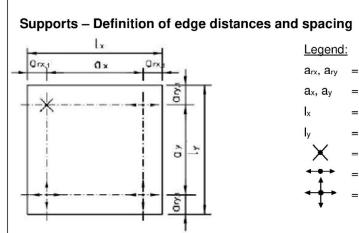
Intended use Specifications

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edge distance – anchor distance to the panel edge spacing – distance between the anchors length of the panel in horizontal direction length of the panel in vertical direction fixed bearing (fixed support) horizontal slide bearing (slide support)

horizontal and vertical slide bearing (slide support)

Figure 2: Façade panel with 4 agraffes – support condition 1

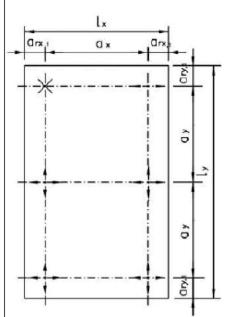


Figure 3: Façade panel with 6 agraffes - support condition 2

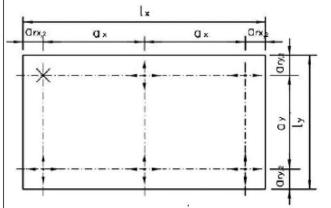


Figure 4: Façade panel with 6 agraffes - support condition 3

Primero KEIL undercut anchor KH

Intended use

Supports - definition of edge distance and spacing



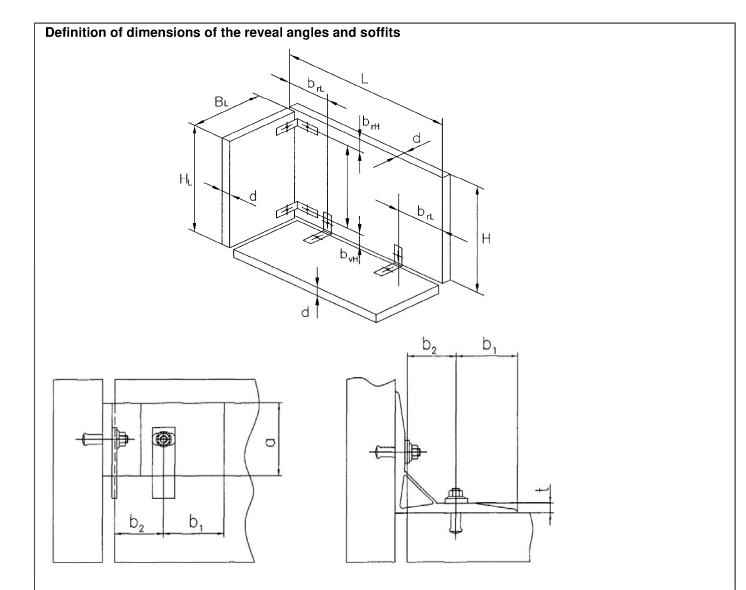


Table B5:	Characteristic values of the reveal angles
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		stainless steel 1.4401. 1.4404 bzw. 1.4571 EN 10088-3:2014	aluminium EN 755:2016
angle thickness	t [mm]	t ≥ 4	t ≥ 5
angle width	a [mm]	$40 \le a \le 100$	$40 \le a \le 100$
Distance between the centre of anchor to outer edge of reveal angle	b₁ [mm]	$25 \le b_1 \le 10 \ t$	$25 \le b_1 \le 8 \ t$
Distance between the centre of anchor to inner edge of reveal angle	b ₂ [mm]	$40 \le b_2 \le 10 \ t$	$40 \le b_2 \le 8 \ t$
cross tension stiffness	c _q [MN/m]	$c_q \leq 2,5$	

Primero KEIL undercut anchor KH

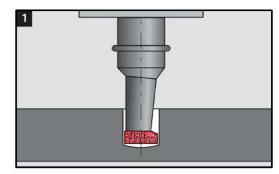
Intended use

Reveal angle and soffit of the panel made of natural stone (except slate)

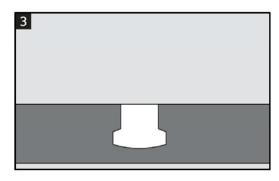


Installation instructions

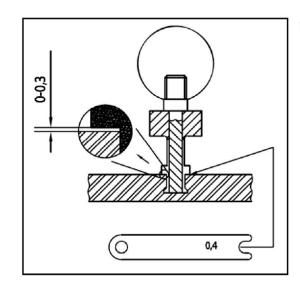
- 1. Drilling the undercut hole
- a) cylindrical drilling



c) finished undercut hole



2. Checking the undercut hole

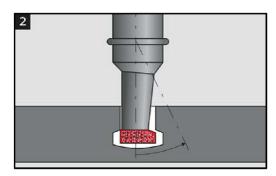


Primero KEIL undercut anchor KH

Intended use

Installation instructions

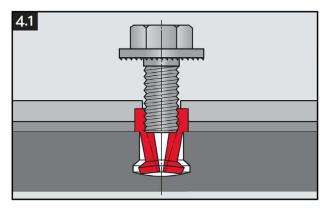
b) undercutting and cleaning



with KEIL depth control guide

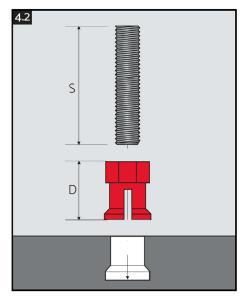


3. Installation of anchor (sleeve and screw)

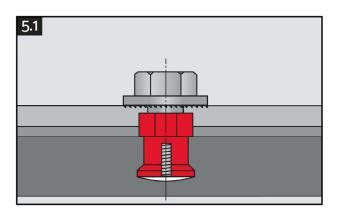


a) Insert the sleeve in the undercut hole and screw in the screw in the sleeve

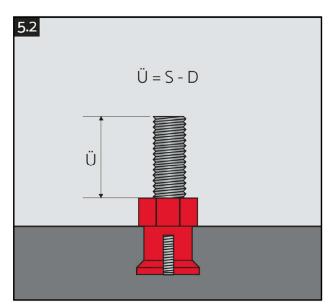
4. Installation of anchor (sleeve and grub screw)



a) Insert the sleeve in the undercut hole and screw in the grub screw in the sleeve



b) Installed anchor



b) Installed anchor





Characteristic load-bearing capacity of the anchor

Table C1: Characteristic values for facade panels and reveal panels

Natural stone			Epprechstein yellow	Padang light	Sto-Kilzinger Sandstein
Country of origin			Germany	China	Poland
Petrographic desription			granite	granite	sandstone
Embedment depth	hs =	[mm]	15	10	15
Edge distance	ar ≥	[mm]	100	50	100
Spacing	a≥	[mm]	120	80	120
Characteristic resistance					
under tension load	N _{Rk} =	[kN]	4,6	3,6	2,9
under shear load	V _{Rk} =	[kN]	5,1	4,2	3,0
Combined tension and shear lo	ad				
Value for tri-linear function	Х	[-]	1,2	1,2	1,2

Table C2: Characteristic values for facade panels made of slate CS 50, SIN 120 und SIN 150

			CS 50 ¹⁾		SIN 1201)	SIN 150 ¹⁾	
Country of origin			Brasilia		Spain	Spain	
Petrographic desription			sl	ate	slate	slate	
Embedment depth	hs =	[mm]			7		
Edge distance	a₁ ≥	[mm]	50	100	50		
Spacing	a≥	[mm]	100	200	100		
Characteristic resistance							
Characteristic resistance to tension load	N _{Rk} =	[kN]	1,1	1,5	1,3	1,2	
Characteristic resistance to shear load	V _{Rk} =	[kN]	1,6	1,9	2,7 3,0		
Combined tension and shear load							
Value for tri-linear function	Х	[-]	1,0	1,0	1,0	1,0	

¹⁾ According to Table B3

Primero KEIL undercut anchor KH

Performance Characteristic resistance Annex C 1

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Table C3: Characteristic values for facade panels made of slate Primero Vulcano Schiefer

Country of origin			Spain
Petrographic desription			slate
Embedment depth	hs =	[mm]	15
Edge distance	a₁ ≥	[mm]	50
Spacing	a≥	[mm]	100
Characteristic resistance			
Characteristic resistance to tension load	N _{Rk} =	[kN]	4,2
Characteristic resistance to shear load	V _{Rk} =	[kN]	6,0
Combined tension and shear load			
Value for tri-linear function	Х	[-]	1,2

¹⁾ According to Table B4

Table C4: Characteristic resistance for steel failure

Characteristic resistance			
Characteristic resistance under tension load	$N_{Rk,s}$	[kN]	14,1
Partial safety factor for tension load	$\gamma Ms^{1)}$	[-]	1,87
Characteristic resistance under shear load	$V_{Rk,s}$	[kN]	7,0
Partial safety factor	$\gamma Ms^{1)}$	[-]	1,56

¹⁾ In absence of national regulations.

Table C5: reduction factor α_{TR}

Reduction factor	αtr	[-]	1,0	

Primero KEIL undercut anchor KH

Performance Characteristic resistance and α_{TR}

Annex C2

Γ



 1 Admissible wind loads for selective panel sizes and bearing conditions for slate panels CS 50, SIN 120 and SiN 150 1.1 General In Annex D 3 to D 12 several panel systems are listed as a function of the panel thickness, anchorage depth, edge distance, panel size, number of agraffes and the kind of support. The substructure has to be symmetrical. For flush-fixed profiles following has to be considered: Cu = height of the agraffe Classification of the profiles in the following ratios: e' = distance between of the facade panel and shear centre of the horizontal profile (s. figure 1) fugure 1: Open profile with shear centre The proof of structural stability is fulfilled if Eq. (1) is satisfied. w_{Ex} ≤ w_{Tab} classified and the stability is fulfilled if Eq. (1) is satisfied. w_{Ex} ≤ w_{Tab} classified according to EN 1991-1-4 w_{Tab} = value of admissible wind load according to EN 1991-1-4 w_{Tab} = value of admissible wind loads, see Annex D Following partial sately factors are fundamental for evaluation: y₀ = 1,35; y₀ = 1,5; y₁ = 1,8. The constructional requirements of Annex D 3 for the support with 6 fixing points must be fulfilled. For subframes supporting three fixing points of a one panel or unsymmetrical supported panels with 4 fixing points, the moment of inertia of profiles must be at minimum: ly [cm²] = 65,2 + L [m] - 58,5 (gilt für: 0,9 m ≤ L ≤ 1,4 m) (2) with: L = equivalent support width (Annex D 3) k = moment of inertia of profiles has to be E ≥ 70.000 N/mm². Primero KEIL undercut anchor KH	Design of slate according to table B 3	
In Annex D 3 to D 12 several panel systems are listed as a function of the panel thickness, anchorage depth, edge distance, panel size, number of agraffes and the kind of support. The substructure has to be symmetrical. For flush-fixed profiles following has to be considered: $C_{H} = height of the agraffe Classification of the profiles in the tollowing ratios: e'C_{H} = 0.54e'C_{H} = 0.52e'C_{H} = 0.52e'$	•	ng conditions for slate panels CS 50, SIN 120 and
distance, panel size, number of agraffes and the kind of support. The substructure has to be symmetrical. For flush-fixed profiles following has to be considered: $c_{H} = height of the agraffe Classification of the profiles in the following ratios: e(C_{H} \le 0.54)e(C_{H} \le 0.54)e(C_$	1.1 General	
Gri = height of the agraffeClassification of the profiles in the following ratios: $e/CH = 50,53$ $e/CH = 50,33$ Image: Colspan="2">Image: Colspan="2" Image: C		
Classification of the profiles in the following ratios: $e'C_{H} \le 0.75$ $e'C_{H} \le 0.33$ $e'C_{H} = 0.32$ $e'C_{H} = 0.32$ $e'C_{H}$	For flush-fixed profiles following has to be considered:	
	$c_{H} =$ height of the agraffe	e t
shear centre of the horizontal profile (s. figure 1) figure 1: Open profile with shear centre The proof of structural stability is fulfilled if Eq. (1) is satisfied. (1) wEk ≤ wTmb (1) mit: wEk = characteristic wind load according to EN 1991-1-4 wTab = value of admissible wind loads, see Annex D Following partial safety factors are fundamental for evaluation: yG = 1,35; YG = 1,5; YM = 1,8. The constructional requirements of Annex D 3 for the support with 6 fixing points must be fulfilled. For subframes supporting three fixing points of a one panel or unsymmetrical supported panels with 4 fixing points, the moment of inertia of profiles must be at minimum: Iv [cm ^d] = 65,2 • Li [m] - 58,5 (gilt für: 0,9 m ≤ Li ≤ 1,4 m) (2) with: Li = equivalent support width (Annex D 3) Iv = moment of inertia of profiles has to be E ≥ 70.000 N/mm². Primero KEIL undercut anchor KH Primero KEIL undercut anchor KH	e/сн ≤ 0,75 e/сн ≤ 0,54	
(s. figure 1) figure 1: Open profile with shear centre The proof of structural stability is fulfilled if Eq. (1) is satisfied. w _{Ek} ≤ w _{Tab} (1) mit: w _{Ek} = characteristic wind load according to EN 1991-1-4 w _{Tab} = value of admissible wind loads, see Annex D Following partial safety factors are fundamental for evaluation: yo = 1,35; yo = 1,5; ym = 1,8. The constructional requirements of Annex D 3 for the support with 6 fixing points must be fulfilled. For subframes supporting three fixing points of a one panel or unsymmetrical supported panels with 4 fixing points, the moment of inertia of profiles must be at minimum: ly [cm4] = 65,2 • Li [m] - 58,5 (gilt für: 0,9 m ≤ Li ≤ 1,4 m) (2) with: Li = equivalent support width (Annex D 3) ly = moment of inertia of profiles has to be E ≥ 70.000 N/mm². Primero KEIL undercut anchor KH		
$ \begin{split} & w_{Ek} \leq w_{Tab} & (1) \\ & mit: w_{Ek} = characteristic wind load according to \mathsf{EN 1991-1-4} \\ & w_{Tab} = value \text{ of admissible wind loads, see Annex D} \\ & Following partial safety factors are fundamental for evaluation: \\ & \forall g = 1,3S; \forall q = 1,S; \forall m = 1,R. \end{split} $ The constructional requirements of Annex D 3 for the support with 6 fixing points must be fulfilled. For subframes supporting three fixing points of a one panel or unsymmetrical supported panels with 4 fixing points, the moment of inertia of profiles must be at minimum: \\ & lv [cm^4] = 65, 2 \cdot L_i [m] \cdot 58, 5 (gilt \ fur: 0, 9 \ m \leq L_i \leq 1, 4 \ m) \qquad (2) \\ & with: L_i = equivalent \ support \ with \ (Annex D 3) \\ & lv = moment \ of \ inertia \ of \ profiles (y\operatorname{-axis} \ of \ the profile: \ parallel \ to \ the \ fac \ panel \ layer) \\ & The module \ of \ elasticity \ of \ the profiles \ has \ to \ be \ E \geq 70.000 \ N/mm^2. \end{split}		figure 1: Open profile with shear centre
mit:wEk=characteristic wind load according to EN 1991-1-4wTab=value of admissible wind loads, see Annex DFollowing partial safety factors are fundamental for evaluation: $\gamma_{G} = 1,35; \gamma_{G} = 1,5; \gamma_{M} = 1,8.$ The constructional requirements of Annex D 3 for the support with 6 fixing points must be fulfilled.For subframes supporting three fixing points of a one panel or unsymmetrical supported panels with 4 fixing points, the moment of inertia of profiles must be at minimum:Iv [cm4] =65,2 \cdot Li [m] - 58,5 (gilt für: 0,9 m ≤ Li ≤ 1,4 m)(2)with:Li=equivalent support width (Annex D 3)Iv=moment of inertia of profiles has to be E ≥ 70.000 N/mm².	The proof of structural stability is fulfilled if Eq. (1) is satisfied.	
$\begin{split} w_{Tab} &= value of admissible wind loads, see Annex D Following partial safety factors are fundamental for evaluation: \\ &\gamma \alpha = 1,35; \gamma \alpha = 1,5; \gamma M = 1,8. \end{split}$ The constructional requirements of Annex D 3 for the support with 6 fixing points must be fulfilled. For subframes supporting three fixing points of a one panel or unsymmetrical supported panels with 4 fixing points, the moment of inertia of profiles must be at minimum: \\ &I_{Y} [cm^{d}] = 65,2 \cdot L_{i} [m] - 58,5 (gilt für: 0,9 m \leq L_{i} \leq 1,4 m) (2) \\ &with: L_{i} &= equivalent support width (Annex D 3) \\ &I_{Y} &= moment of inertia of profiles has to be E \geq 70.000 N/mm^{2}. \end{split}	$W_{Ek} \leq W_{Tab}$	(1)
Following partial safety factors are fundamental for evaluation: $\gamma_{G} = 1,35; \gamma_{O} = 1,5; \gamma_{M} = 1,8.$ The constructional requirements of Annex D 3 for the support with 6 fixing points must be fulfilled. For subframes supporting three fixing points of a one panel or unsymmetrical supported panels with 4 fixing points, the moment of inertia of profiles must be at minimum: $I_{Y} [cm^{4}] = 65, 2 \cdot L_{i} [m] - 58,5$ (gilt für: 0,9 m ≤ $L_{i} \le 1,4$ m) (2) with: $L_{i} =$ equivalent support width (Annex D 3) $I_{Y} =$ moment of inertia of profiles (y-axis of the profile: parallel to the façade panel layer) The module of elasticity of the profiles has to be E ≥ 70.000 N/mm ² .	mit: w_{Ek} = characteristic wind load according to) EN 1991-1-4
$\label{eq:gamma} \begin{array}{l} \gamma_{G} = 1,35; \ \gamma_{O} = 1,5; \ \gamma_{M} = 1,8. \end{array}$ The constructional requirements of Annex D 3 for the support with 6 fixing points must be fulfilled. For subframes supporting three fixing points of a one panel or unsymmetrical supported panels with 4 fixing points, the moment of inertia of profiles must be at minimum: $I_{Y} [cm^{4}] = \ 65,2 \cdot L_{i} [m] - 58,5 \ (gilt für: 0,9 \ m \leq L_{i} \leq 1,4 \ m) \qquad (2)$ with: $L_{i} = equivalent \ support \ width \ (Annex D 3) \\ I_{Y} = moment \ of \ inertia \ of \ profiles \ (y-axis \ of \ the \ profile: \ parallel \ to \ the \ façade \ panel \ layer) \ The module \ of \ elasticity \ of \ the \ profiles \ has \ to \ be \ E \geq 70.000 \ N/mm^{2}. \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \$	w _{Tab} = value of admissible wind loads, see	Annex D
For subframes supporting three fixing points of a one panel or unsymmetrical supported panels with 4 fixing points, the moment of inertia of profiles must be at minimum: $I_{Y} [cm^{4}] = 65,2 \cdot L_{i} [m] - 58,5$ (gilt für: 0,9 m ≤ $L_{i} \le 1,4$ m) (2) with: $L_{i} = equivalent$ support width (Annex D 3) $I_{Y} = moment of inertia of profiles (y-axis of the profile: parallel to the façade panel layer)$ The module of elasticity of the profiles has to be E ≥ 70.000 N/mm ² . Primero KEIL undercut anchor KH		undamental for evaluation:
the moment of inertia of profiles must be at minimum: $I_{Y} [cm^{4}] = 65,2 \cdot L_{i} [m] - 58,5 (gilt für: 0,9 m \le L_{i} \le 1,4 m) $ (2) with: $L_{i} = equivalent support width (Annex D 3)$ $I_{Y} = moment of inertia of profiles (y-axis of the profile: parallel to the façade panel layer)$ The module of elasticity of the profiles has to be $E \ge 70.000 \text{ N/mm}^{2}$.	The constructional requirements of Annex D 3 for the support w	rith 6 fixing points must be fulfilled.
with: L_i = equivalent support width (Annex D 3) I_Y = moment of inertia of profiles (y-axis of the profile: parallel to the façade panel layer) The module of elasticity of the profiles has to be E \geq 70.000 N/mm ² . Primero KEIL undercut anchor KH		nsymmetrical supported panels with 4 fixing points,
I_Y = moment of inertia of profiles (y-axis of the profile: parallel to the façade panel layer) The module of elasticity of the profiles has to be E \ge 70.000 N/mm ² . Primero KEIL undercut anchor KH	l _Y [cm⁴] = 65,2 • L _i [m] - 58,5 (gilt für: 0,9 m ≤ L _i ≤ 1,4 m)	(2)
The module of elasticity of the profiles has to be E ≥ 70.000 N/mm². Primero KEIL undercut anchor KH	with: L_i = equivalent support width (Annex D 3)	
Primero KEIL undercut anchor KH	I _Y = moment of inertia of profiles (y-axis of t	the profile: parallel to the façade panel layer)
	The module of elasticity of the profiles has to be $E \geq 70.000 \; \text{N/n}$	nm².
Appey D 1	Primero KEIL undercut anchor KH	
	Informative	Annex D 1

Design slate

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1.2 Smaller panel dimensions

In case of smaller panel dimensions for panels supported with 4 anchors Eq. (3) must be satisfied. For positioning of the fixings the relation of edge distance to length of the panel must be kept. The minimum edge distances can be taken from the respective Tables (Annex D 3 to D 12).

$$w_{Ek} \leq 0.9 \times \frac{A_{Tab}}{A_{vorh}} w_{Tab}$$
(3)
mit:
$$w_{Ek} = \text{characteristic wind load according to EN 1991-1-4} \\ w_{Tab} = \text{value of admissible wind loads aus Anhang D} \\ A_{Tab} = \text{panel size given in the Tables (Annex D 3 to D 12), related to admissible wind loads of the Tables} \\ A_{vorh} = \text{existing panel size (area)}$$

1.3 Unsymmetrical substructure

Unsymmetrical substructures can only be applied for panels supported with 4 fixing points. In this case Eq. (4) has to be fulfilled.

WEk	≤ (0,5 w та	b
mit:	WEk	=	characteristic wind load according to EN 1991-1-4
	WTab	=	value of admissible wind loads according to Annex D 3 to D 12

.				
Primero	KEIL	undercut	anchor	KH

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Annex D 2

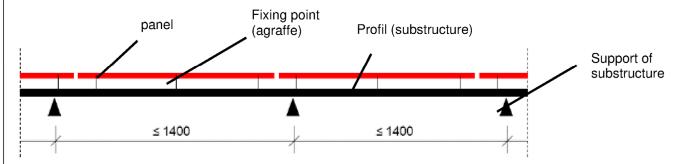
(4)

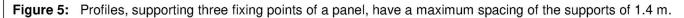


Dimensioning aid

The following bearing conditions are to use for the admissible wind loads in Annex D 4 to D 12. By object-releated calculation other admissible wind load tables may be determined.

Maximum support spacing and location of the substructure fixings





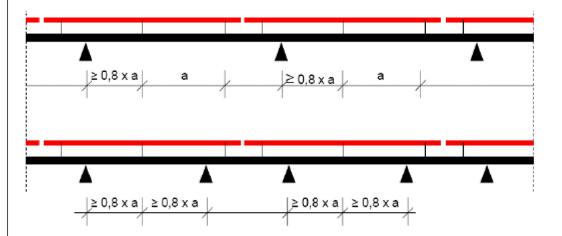
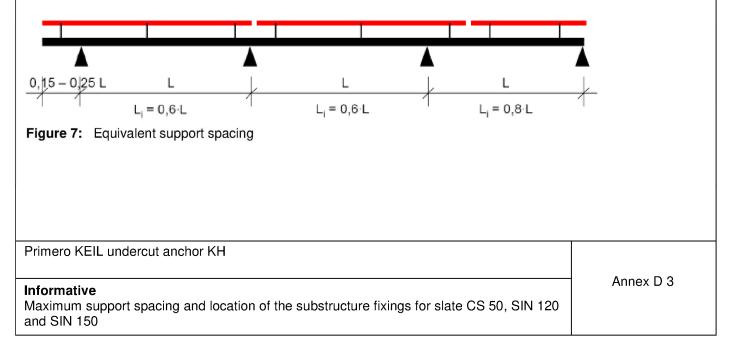


Figure 6: In profiles, supporting three fixing points of a panel, the central fixing points must have a minimum distance of 0,8 x a (a= spacing of the fixing points of the panel) to the supports.





	imensioning aid for facade panels made of slate CS 50 Table D1: admissible wind loads – flush-fixed, $e/c_H = 0.75$; $a_r \ge 50$ mm												
System	d	h _v = [mm]	a _{rx,1} a _{ry,2} [mm]	a _{ry,1} a _{rx,2} [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{Tab} [kN/m²]					
1	10 12,5 15	7	50-150	50-150	600 x 600	4	1	3,6 3,3 3,0					
2	10 12,5 15	7	50-150	50-200	600 x 900	4	1	2,0 1,7 1,5					
3	10 12,5 15	7	50-150	100-250	600 x 1200	4	1	1,2 1,0 0,7					
4	10 12,5 15	7	50-200	50-200	750 x 750	4	1	1,9 1,6 1,4					
7	10 12,5 15	7	50-100	100-150	600 x 1200	6	2 3	1,3 1,1 0,8					
8	10	7	50-100	150-225	1000 x 1000	6	2 3	0,6					

Table D2: admissible wind loads – flush-fixed, $e/c_H = 0,75$; $a_r \ge 100$ mm

System	d = [mm]	h _∨ = [mm]	a _{rx,1} a _{ry,2} [mm]	a _{ry,1} a _{rx,2} [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{⊺ab} [kN/m²]
1	10 12,5 15	7	100-150	100-150	600 x 600	4	1	4,0 4,7 4,4
2	10 12,5 15	7	100-150	100-200	600 x 900	4	1	2,2 2,7 2,4
3	10 12,5 15	7	100-150	100-250	600 x 1200	4	1	1,3 1,6 1,4
4	10 12,5 15	7	100-200	100-200	750 x 750	4	1	1,9 2,5 2,2
5	10 12,5 15	7	100-200	100-200	900 x 900	4	1	1,4 1,3 1,0
6	10 12,5 15	7	100-200	100-200	1000 x 1000	4	1	1,0 0,8 0,5
7	10 12,5 15	7	100	100-150	600 x 1200	6	2 3	2,0 1,8 1,5
8	10 12,5 15	7	100	150-225	1000 x 1000	6	2 3	0,8 0,8 0,5

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Table D3:	admis	sible w	ind loads	– flush-fix	ed, $e/c_{H} = 0.54$; a _r ≥ 50 mi	m	
System	d = [mm]	h _v = [mm]	a _{rx,1} a _{ry,2} [mm]	a _{ry,1} a _{rx,2} [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{Tab} [kN/m ²]
1	10 12,5 15	7	50-150	50-150	600 x 600	4	1	3,8 3,5 3,3
2	10 12,5 15	7	50-150	50-200	600 x 900	4	1	2,2 2,0 1,8
3	10 12,5 15	7	50-150	100-250	600 x 1200	4	1	1,5 1,2 1,0
4	10 12,5 15	7	50-200	50-200	750 x 750	4	1	2,1 1,9 1,7
7	10 12,5 15	7	50-100	100-150	600 x 1200	6	2 3	1,6 1,3 1,1
8	10	7	50-100	150-225	1000 x 1000	6	2 3	0,9

Table D4: admissible wind loads – flush-fixed, $e/c_H = 0.54$; $a_r \ge 100 \text{ mm}$

System	d = [mm]	h _∨ = [mm]	a _{rx,1} a _{ry,2} [mm]	ary,1 arx,2 [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{Tab} [kN/m ²]
	10							4,2
1	12,5	7	100-150	100-150	150 600 × 600	4	1	5,0
	15							4,8
	10							2,4
2	12,5	7	100-150	100-200	600 x 900	4	1	2,9
	15							2,7
	10							1,5
3	12,5	7	100-150	100-250	600 x 1200	4	1	1,9
	15							1,7
	10							2,1
4	12,5	7	100-200	100-200	750 x 750	4	1	2,8
	15							2,5
	10							1,6
5	12,5	7	100-200	100-200	900 x 900	4	1	1,6
	15							1,3
	10							1,2
6	12,5	7	100-200	100-200	1000 x 1000	4	1	1,0
	15							0,8
	10						2	2,0
7	12,5	7	100	100-150	600 x 1200	6	2 3	2,1
	15						5	1,8
	10						2	1,0
8		7	100	150-225	1000 x 1000	6	2 3	1,1
							,	0,9

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able D5: ad	Imissibl	le wind	loads – flu	ush-fixed,	e/с _н = 0,33; а _r	≥ 50 mm		
System	d = [mm]	h _v = [mm]	a _{rx,1} a _{ry,2} [mm]	a _{ry,1} a _{rx,2} [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{Tab} [kN/m²]
1	10 12,5 15	7	50-150	50-150	600 x 600	4	1	4,0 3,8 3,7
2	10 12,5 15	7	50-150	50-200	600 x 900	4	1	2,4 2,3 2,1
3	10 12,5 15	7	50-150	100-250	600 x 1200	4	1	1,7 1,5 1,3
4	10 12,5 15	7	50-200	50-200	750 x 750	4	1	2,3 2,1 2,0
7	10 12,5 15	7	50-100	100-150	600 x 1200	6	2 3	1,8 1,6 1,4
8	10 12,5 15	7	50-100	150-225	1000 x 1000	6	2 3	1,1 0,9 0,7

Table D6: admissible wind loads – flush-fixed, $e/c_H = 0.33$; $a_r \ge 100 \text{ mm}$

System	d = [mm]	h _v = [mm]	a _{rx,1} a _{ry,2} [mm]	ary,1 arx,2 [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{Tab} [kN/m ²]
1	10 12,5 15	7	100-150	100-150	600 x 600	4	1	4,4 5,2 5,1
2	10 12,5 15	7	100-150	100-200	600 x 900	4	1	2,6 3,2 3,0
3	10 12,5 15	7	100-150	100-250	600 x 1200	4	1	1,7 2,2 2,0
4	10 12,5 15	7	100-200	100-200	750 x 750	4	1	2,3 3,0 2,9
5	10 12,5 15	7	100-200	100-200	900 x 900	4	1	1,8 1,8 1,6
6	10 12,5 15	7	100-200	100-200	1000 x 1000	4	1	1,4 1,3 1,1
7	10 12,5 15	7	100	100-150	600 x 1200	6	2 3	2,0 2,3 2,1
8	10 12,5 15	7	100	150-225	1000 x 1000	6	2 3	1,2 1,4 1,2

Primero KEIL undercut anchor KH

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Dimensioning aid for façade panels made of slate CS 50



					f slate SIN 120)		
able D7:					ed, $e/c_{\rm H} = 0.75$		•	I
System	d =	h _v =	arx,1 ary,2	ary,1 arx,2	Panel length x width	No. of agraffes	Support condition	Admissible windloads wTab [kN/m ²]
	[mm]	[<u>[mm]</u>	[mm]	[mm]	[mm]		(Annex B 5)	
	10,0	-						2,2
	12,5							3,3
1	15,0	7	50-150	50-150	600 x 600	4	1	3,0
	17,5							2,8
	20,0							2,6
	10,0				600 x 900			1,1
	12,5						1	1,8
2	15,0	7	50-150	50-200		4		1,5
	17,5]						1,3
	20,0	1						1,1
	10,0							0,5
	12,5	1						1,0
3	15,0	7	50-150	100-250	600 x 1200	4	1	0,8
	17,5	1						0,6
	20,0	1						0,3
	10,0				750 x 750	4	1	0,9
	12,5	1		50-200				1,7
4	15,0	7	50-200					1,4
	17,5							1,2
	20,0							0,9
	10,0							0,6
	12,5	-						0,8
5	15,0	7	100-200	100-200	900 x 900	4	1	0,5
, J	17,5	'	100 200		000 × 000	т		0,3
	20,0	1						0,0
	10,0							0,1
6	12,5	7	100-200	100-200	1000 x 1000	4	1	0,3
0	15,0	'				4	1	0,4
1	15,0							0,2

Primero KEIL undercut anchor KH

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Dimensioning aid for façade panels made of slate SIN 120

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Table D8:	admiss		nd loads –	flush-fixe	d, e/с _н = 0,54			
System	d = [mm]	h _v = [mm]	a _{rx,1} a _{ry,2} [mm]	a _{ry,1} a _{rx,2} [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{Tab} [kN/m²]
1	10,0 12,5 15,0 17,5 20,0	7	50-150	50-150	600 x 600	4	1	2,4 3,5 3,3 3,1 2,9
2	10,0 12,5 15,0 17,5 20,0	7	50-150	50-200	600 x 900	4	1	1,3 2,0 1,8 1,7 1,5
3	10,0 12,5 15,0 17,5 20,0	7	50-150	100-250	600 x 1200	4	1	0,7 1,3 1,1 0,9 0,7
4	10,0 12,5 15,0 17,5 20,0	7	50-200	50-200	750 x 750	4	1	1,1 1,9 1,7 1,5 1,4
5	10,0 12,5 15,0 17,5 20,0	7	100-200	100-200	900 x 900	4	1	0,8 1,1 0,9 0,7 0,5
6	10,0 12,5 15,0 17,5 20,0	7	100-200	100-200	1000 x 1000	4	1	0,6 0,7 0,5 0,3 0,1

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Table D9:	admis	ssible w	ind loads	– flush-fix	ed, e/c _H = 0,33			
System	d = [mm]	h _v = [mm]	a _{rx,1} a _{ry,2} [mm]	a _{ry,1} a _{rx,2} [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{Tab} [kN/m²]
1	10,0 12,5 15,0 17,5 20,0	7	50-150	50-150	600 x 600	4	1	2,6 3,8 3,7 3,5 3,4
2	10,0 12,5 15,0 17,5 20,0	7	50-150	50-200	600 x 900	4	1	1,5 2,3 2,2 2,0 1,9
3	10,0 12,5 15,0 17,5 20,0	7	50-150	100-250	600 x 1200	4	1	0,9 1,6 1,4 1,3 1,2
4	10,0 12,5 15,0 17,5 20,0	7	50-200	50-200	750 x 750	4	1	1,3 2,2 2,0 1,9 1,8
5	10,0 12,5 15,0 17,5 20,0	7	100-200	100-200	900 x 900	4	1	1,0 1,3 1,2 1,0 0,9
6	10,0 12,5 15,0 17,5 20,0	7	100-200	100-200	1000 x 1000	4	1	0,8 0,9 0,8 0,7 0,5

Primero KEIL undercut anchor KH

Informative

Dimensioning aid for façade panels made of slate SIN 120



					f slate SIN 150 кed, e/сн = 0,75			
System	d = [mm]	h _v = [mm]	a _{rx,1} a _{ry,2} [mm]	a _{ry,1} a _{rx,2} [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{Tab} [kN/m ²]
1	10,0 12,5 15,0 17,5 20,0	7	50-150	50-150	600 x 600	4	1	2,8 3,4 3,1 2,9 2,7
2	10,0 12,5 15,0 17,5 20,0	7	50-150	50-200	600 x 900	4	1	1,5 1,9 1,6 1,4 1,2
3	10,0 12,5 15,0 17,5 20,0	7	50-150	100-250	600 x 1200	4	1	0,8 1,1 0,9 0,6 0,4
4	10,0 12,5 15,0 17,5 20,0	7	50-200	50-200	750 x 750	4	1	1,3 1,7 1,5 1,3 1,0
5	10,0 12,5 15,0 17,5 20,0	7	100-200	100-200	900 x 900	4	1	0,9 0,8 0,6 0,4 0,1
6	10,0 12,5 15,0	7	100-200	100-200	1000 x 1000	4	1	0,6 0,5 0,2

Primero KEIL undercut anchor KH

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System	d = [mm]	h _v = [mm]	a _{rx,1} a _{ry,2} [mm]	a _{ry,1} a _{rx,2} [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads w _{Tab} [kN/m ²]
	10,0	_	50.450					3,0 3,6
1	15,0 17,5 20,0	7	50-150	50-150	600 x 600	4	1	3,4 3,2 3,1
2	10,0 12,5 15,0 17,5 20,0	7	50-150	50-200	600 x 900	4	1	1,7 2,1 1,9 1,7 1,6
3	10,0 12,5 15,0 17,5 20,0	7	50-150	100-250	600 x 1200	4	1	1,0 1,4 1,2 1,0 0,8
4	10,0 12,5 15,0 17,5 20,0	7	50-200	50-200	750 x 750	4	1	1,5 2,0 1,8 1,6 1,4
5	10,0 12,5 15,0 17,5 20,0	7	100-200	100-200	900 x 900	4	1	1,1 1,1 0,9 0,7 0,6
6	10,0 12,5 15,0 17,5 20,0	7	100-200	100-200	1000 x 1000	4	1	0,8 0,7 0,5 0,4 0,2

Primero KEIL undercut anchor KH

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Dimensioning aid for façade panels made of slate SIN 150

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System	d =	h _v =	a _{rx,1} a _{ry,2}	a ry,1 a rx,2 [mm]	Panel length x width [mm]	No. of agraffes	Support condition (Annex B 5)	Admissible windloads wTab [kN/m ²]
	[mm]	[mm]	[mm]	[[[[[[[IIIII]	_	(Annex D 5)	
	10,0		50-150					<u>3,2</u> 3,9
1	12,5	7		50-150	600 x 600	4	4	
I	15,0	/					1	<u>3,8</u> 3,6
	17,5							
	20,0							3,5
	10,0							1,9
0	12,5	-	50 150	50.000	600 × 900	4	1	2,4
2	15,0	7	50-150	50-200				2,3
	17,5							2,1
	20,0							2,0
	10,0			100-250	600 x 1200	4	1	1,2
0	12,5	_	50-150					1,6
3	15,0	7						1,5
	17,5							1,4
	20,0							1,2
	10,0			50-200	750 x 750	4	1	1,7
	12,5	_						2,3
4	15,0	7	50-200					2,1
	17,5							2,0
	20,0							1,9
	10,0			100-200	900 x 900	4	1	1,3
	12,5							1,4
5	15,0	7	100-200					1,2
	17,5							1,1
	20,0							1,0
	10,0							1,0
	12,5							1,0
6	15,0	7	100-200	100-200	1000 x 1000	4	1	0,9
	17,5							0,7
	20,0							0,6

Primero KEIL undercut anchor KH

Informative

Dimensioning aid for façade panels made of slate SIN 150