



Approval body for construction products and types of construction

Bautechnisches Prüfamt

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European Technical Assessment

ETA-21/1044 of 29 July 2022

English translation prepared by DIBt - Original version in German language

General Part

Technical Assessment Body issuing the European Technical Assessment:

Trade name of the construction product

Product family to which the construction product belongs

Manufacturer

Manufacturing plant

This European Technical Assessment contains

This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of

Deutsches Institut für Bautechnik

BEHA Composite mortar HQC300 for rebar connection

Systems for post-installed rebar connections with mortar

BEHA GmbH Feldstraße 2a 06458 Selke Aue OT Hausneindorf DEUTSCHLAND

BEHA Werk 1

22 pages including 3 annexes which form an integral part of this assessment

EAD 330087-01-0601, Edition 06/2021

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Specific Part

1 Technical description of the product

The subject of this European Technical Assessment is the post-installed connection, by anchoring or overlap connection joint, of reinforcing bars (rebars) in existing structures made of normal weight concrete, using the "BEHA Composite mortar HQC300 for rebar connection" in accordance with the regulations for reinforced concrete construction.

Reinforcing bars made of steel with a diameter ϕ from 8 to 32 mm or the tension anchor ZA from sizes M12 to M24 according to Annex A and BEHA Composite mortar HQC300 are used for rebar connections. The rebar is placed into a drilled hole filled with injection mortar and is anchored via the bond between rebar, injection mortar and concrete.

The product description is given in Annex A.

2 Specification of the intended use in accordance with the applicable European assessment Document

The performances given in Section 3 are only valid if the rebar connection is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the rebar connection of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance under static and quasi-static loading	See Annex C 1
Characteristic resistance under seismic loading	No performance assessed

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Class A1
Resistance to fire	See Annex C 2 and C 3

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with European Assessment Document EAD No. 330087-01-0601, the applicable European legal act is: [96/582/EC].

The system(s) to be applied is (are): 1



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5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable European Assessment Document

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

Issued in Berlin on 29 July 2022 by Deutsches Institut für Bautechnik

Dipl.-Ing. Beatrix Wittstock Head of Section *beglaubigt:* Baderschneider

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Installation post installed rebar

Figure A1: Overlapping joint for rebar connections of slabs and beams

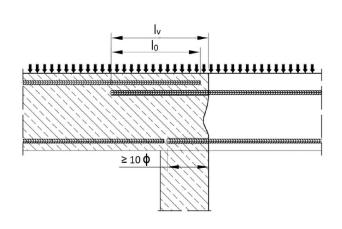


Figure A3: End anchoring of slabs or beams (e.g. designed as simply supported)

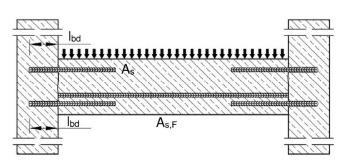


Figure A5: Anchoring of reinforcement to cover the line of acting tensile force

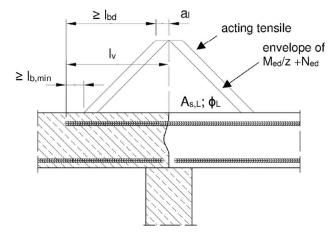


Figure A2: Overlapping joint at a foundation of a wall or column where the rebars are stressed in tension

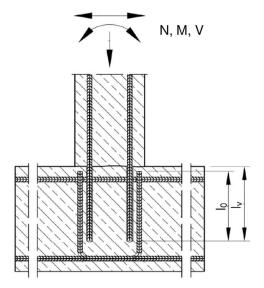
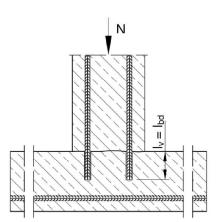


Figure A4: Rebar connection for components stressed primarily in compression.



Note to Figure A1 to A5:

In the Figures no transverse reinforcement is plotted, the transverse reinforcement shall comply with EN 1992-1-1:2004+AC:2010.

Preparing of joints according to Annex B 2

BEHA Composite mortar HQC300 for rebar connection

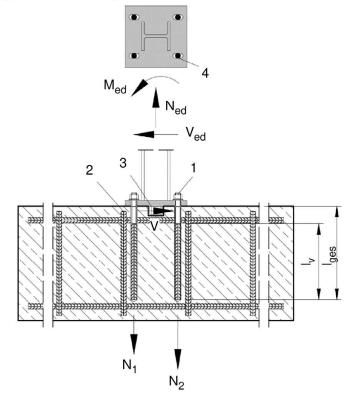
Product description Installed condition and examples of use for rebars

Annex A 1



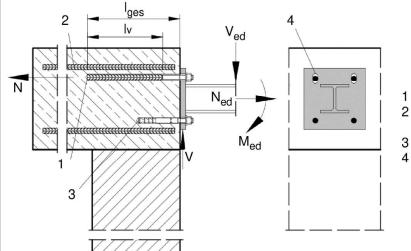
Installation tension anchor ZA

Figure A6: Anchorage of column to foundation with tension anchor ZA.



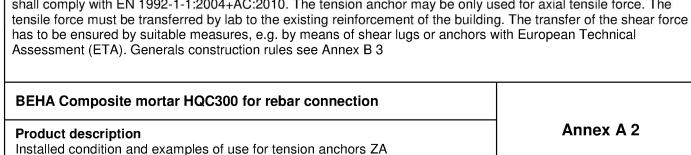
- 1 Tension anchor ZA (tension only)
- 2 Existing stirrup / reinforcement for overlap (lap splice)
- Shear lug (or fastener loaded in shear) 3
- Slotted hole with axial direction to the shear 4 force

Figure A7: Anchorage of guardrail posts or cantilevered building components with tension anchor ZA and fastner.

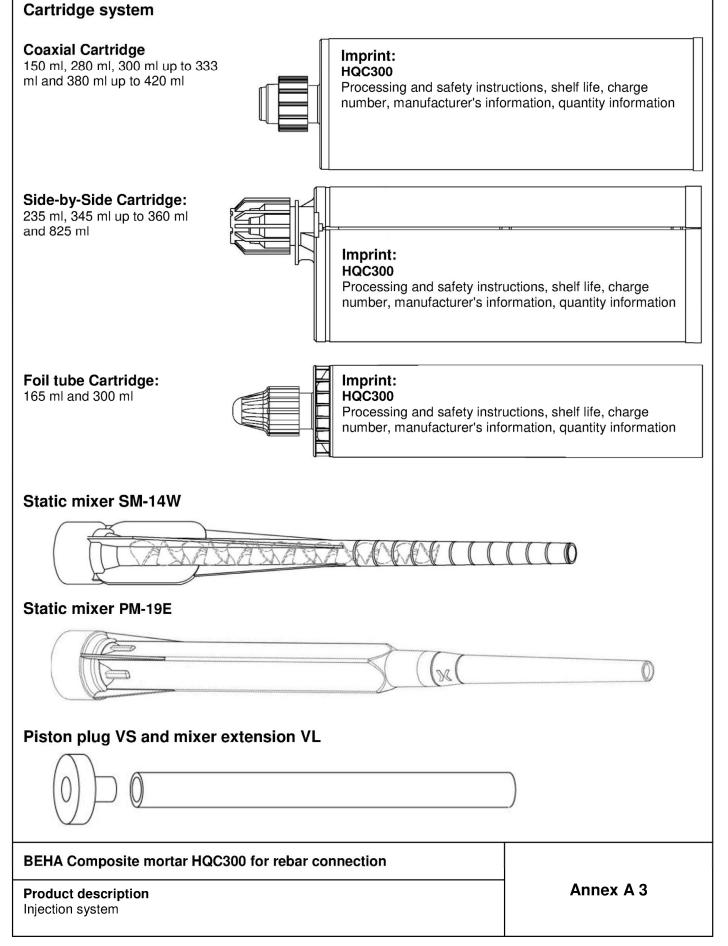


- Tension anchor ZA (tension only)
- Existing stirrup / reinforcement for overlap (lap splice)
- Fastener (or shear lug loaded in shear)
- Slotted hole with axial direction to the shear force

Note to Figure A6 and A7: In the Figures no transverse reinforcement is plotted, the transverse reinforcement shall comply with EN 1992-1-1:2004+AC:2010. The tension anchor may be only used for axial tensile force. The tensile force must be transferred by lab to the existing reinforcement of the building. The transfer of the shear force has to be ensured by suitable measures, e.g. by means of shear lugs or anchors with European Technical Assessment (ETA). Generals construction rules see Annex B 3









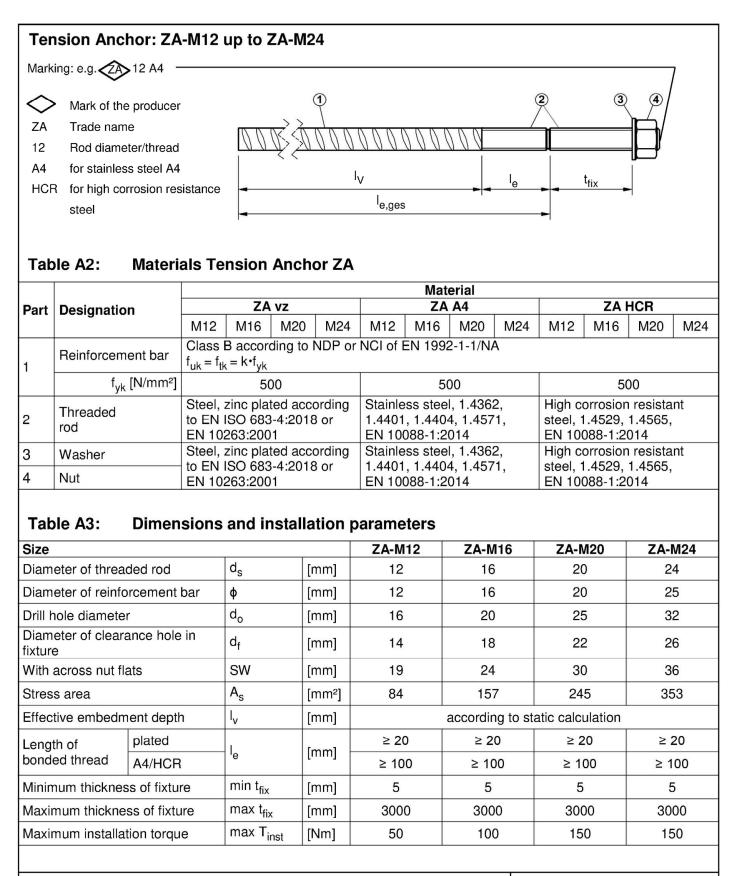
Reinforcing bar (rebar): ø8 up to ø32						
Minimum value of related rip area f _{R,min} accordin	g to EN 1992-1-1:2004+AC:2010					
	 Rib height of the bar shall be in the range 0,05φ ≤ h_{rib} ≤ 0,07φ (φ: Nominal diameter of the bar; h_{rib}: Rib height of the bar) 					
Table A1: Materials Rebar						
Designation	Material					
Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C f_{yk} and k according to NDP or NCI of EN 1992-1-1/NA $f_{uk} = f_{tk} = k \cdot f_{yk}$					

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Product description Specifications Rebar

Annex A 4





BEHA Composite mortar HQC300 for rebar connection

Product description Specifications Tension Anchor ZA

Annex A 5



Specification of the intended use								
Anchorages subject to:		Working life 50 years	Working life 100 years					
HD: Hammer drilling	static and quasi-static loads	Ø8 to Ø32 ZA-M12 to ZA-M24	No performance assessed					
HDB: Hammer drilling with hollow drill bit	seismic action	No performance assessed	No performance assessed					
CD: Compressed air drilling	Fire exposure	Ø8 to Ø32 ZA-M12 to ZA-M24	No performance assessed					
Temperature Range:	- 40°C to +80°C (max long-term temperature +50 °C and max short-term temperature +80 °C)							

Base materials:

- Reinforced or unreinforced normal weight concrete according to EN 206:2013 + A1:2016.
- Strength classes C12/15 to C50/60 according to EN 206:2013 + A1:2016.
- Maximum chloride content of 0,40% (CL 0.40) related to the cement content according to EN 206:2013 + A1:2016.
- Non-carbonated concrete.

Note: In case of a carbonated surface of the existing concrete structure the carbonated layer shall be removed in the area of the post-installed rebar connection with a diameter of ϕ + 60 mm prior to the installation of the new rebar.

The depth of concrete to be removed shall correspond to at least the minimum concrete cover in accordance with EN 1992-1-1:2004+AC:2010. The foregoing may be neglected if building components are new and not carbonated and if building components are in dry conditions.

Use conditions (Environmental conditions) with tension anchor ZA:

- Structures subject to dry internal conditions (all materials).
- For all other conditions according to EN 1993-1-4:2006+A1:2015 corresponding to corrosion resistance class:
 - Stainless steel Stahl A4 according to Annex A 4, Table A1: CRC III
 - High corrosion resistance steel HCR according to Annex A 4, Table A1: CRC V

Design:

- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Verifiable calculation notes and drawings are prepared taking account of the forces to be transmitted.
- Design according to EN 1992-1-1:2004+AC:2010, EN 1992-1-2:2004+AC:2008 and Annex B 2 and B 3.
- The actual position of the reinforcement in the existing structure shall be determined on the basis of the construction documentation and taken into account when designing.

Installation:

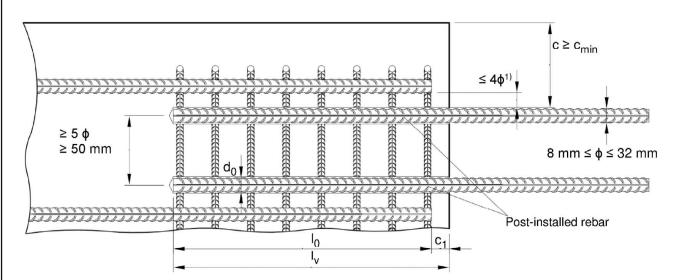
- Dry or wet concrete. It must not be installed in flooded holes.
- Overhead installation allowed.
- Hole drilling by hammer drill (HD), hollow drill (HDB) or compressed air drill mode (CD).
- The installation of post-installed rebar resp. tension anchors shall be done only by suitable trained installer and under supervision on site; the conditions under which an installer may be considered as suitable trained and the conditions for supervision on site are up to the Member States in which the installation is done.
- Check the position of the existing rebars (if the position of existing rebars is not known, it shall be determined using a rebar detector suitable for this purpose as well as on the basis of the construction documentation and then marked on the building component for the overlap joint).

BEHA Composite mortar HQC300 for rebar connection	
Intended use Specifications	Annex B 1



Figure B1: General construction rules for post-installed rebars

- Only tension forces in the axis of the rebar may be transmitted.
- The transfer of shear forces between new concrete and existing structure shall be designed additionally according to EN 1992-1-1:2004+AC:2010.
- The joints for concreting must be roughened to at least such an extent that aggregate protrude.



 If the clear distance between lapped bars exceeds 4φ, then the lap length shall be increased by the difference between the clear bar distance and 4φ.

The following applies to Figure B1:

- c concrete cover of post-installed rebar
- c1 concrete cover at end-face of existing rebar

c_{min} minimum concrete cover according to Table B1 and to EN 1992-1-1:2004+AC:2010, Section 4.4.1.2

- φ diameter of post-installed rebar
- Iap length, according to EN 1992-1-1:2004+AC:2010, Section 8.7.3
- I_v effective embedment depth, $\ge I_0 + c_1$
- d₀ nominal drill bit diameter, see Annex B 5

BEHA Composite mortar HQC300 for rebar connection

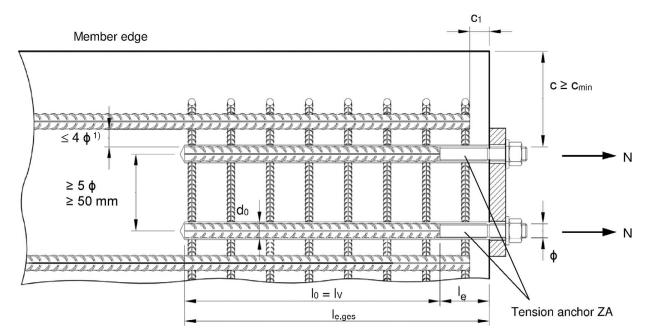
Intended use

General construction rules for post-installed rebars



Figure B2: General construction rules for tension anchors ZA

- The length of the bonded-in thread may be not be accounted as anchorage.
- Only tension forces in the direction of the bar axis may be transmitted by the tension anchor ZA.
- The tension force must be transferred via an overlap joint to the reinforcement in the building part.
- The transfer of shear forces shall be ensured by appropriate additional measures, e.g shear lugs or by anchors with an European technical assessment.
- In the anchor plate, the holes for the tension anchors shall be executed as elongated holes with axis in the direction of the shear force.



 If the clear distance between lapped bars exceeds 4φ, then the lap length shall be increased by the difference between the clear bar distance and 4φ.

The following applies to Figure B2:

concrete cover of tension anchor ZA С concrete cover at end-face of existing rebar C_1 \mathbf{c}_{\min} minimum concrete cover according to Table B1 and to EN 1992-1-1:2004+AC:2010, Section 4.4.1.2 diameter of tension anchor φ lap length, according to EN 1992-1-1:2004+AC:2010, Section 8.7.3 ۱₀ I_v effective embedment depth Length of bonded thread ۱_e overall embedment depth, $\geq I_0 + c_2$ l_{e,ges} nominal drill bit diameter, see Annex B 5 d₀

BEHA Composite mortar HQC300 for rebar connection

Intended use

General construction rules for tension anchors ZA



Drilling method	Rebar diameter	Rebar Without drilling aid		With	drilling aid		
Hammer drilling (HD)	< 25 mm				Drilling aid		
Hammer drilling with hollow drill (HDB)	≥ 25 mm	40 mm + 0,06 · l	,≥2φ 4	$0 \text{ mm} + 0,02 \cdot _{v} \ge 2 $	ta radaanaanaan		
Compressed air	< 25 mm	50 mm + 0,08 · l	, 5	0 mm + 0,02 · I _v			
drilling (CD)	≥ 25 mm	60 mm + 0,08 · l	,≥2¢ 6	0 mm + 0,02 · I _v ≥ 2 o	Þ		
Comments: The minimum	oconcrete cover		004+AC:2010) must be observed.			
Cartridge type/size	•	Ha	nd tool		Pneumatic tool		
Coaxial cartridges ar foil tube cartridges 150, 165, 280, 300 up to 333 ml	ıd	e.g. Type	H297 / H24	e.g. Type TS 492 X			
Coaxial cartridges 380 up to 420 ml	e.g. Ty	ype CCM 380/10	e.g. Type	H 285 or H244C	e.g. Type TS 485 LX		
Side-by-side cartridge 235, 345 ml		e.g. Type CBM 330A			e.g. Type TS 477 LX		
Side-by-side cartridg 825 ml			-	e.g. Type TS 498X			
All cartridges could also be e	xtruded by a batte	ery tool.		I			
			ction				



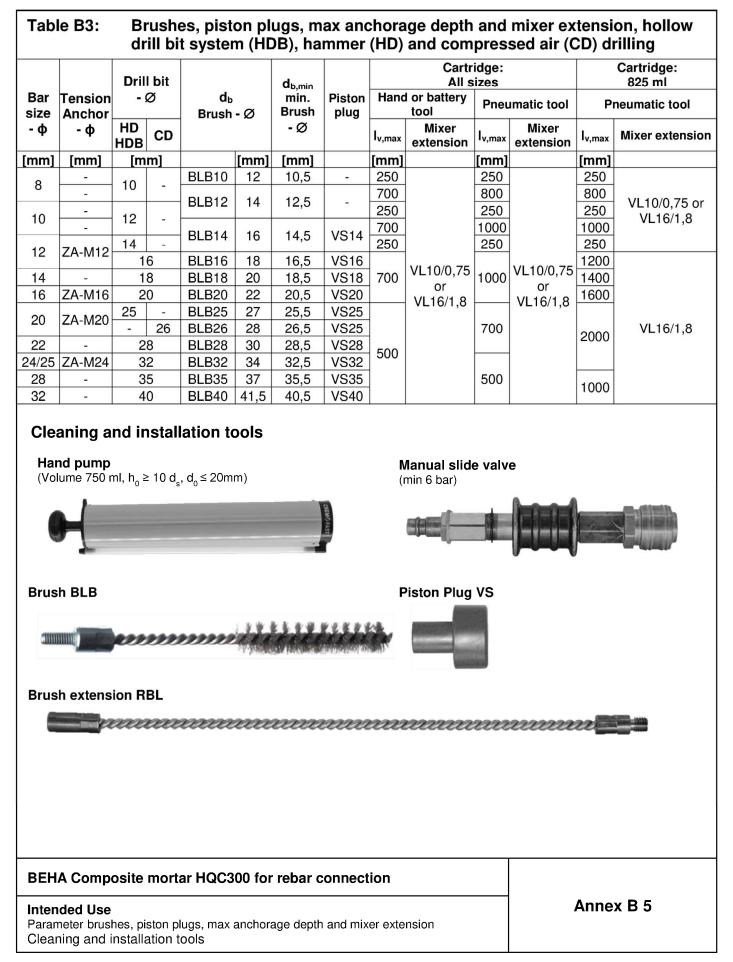




Table B4:	Workiı	ng time and c	uring time	
Temperat	ture in bas	e material	Maximum working time	Minimum curing time ¹⁾
	Т		t _{work}	t _{cure}
- 10°C	up to	- 6 °C	90 min ²⁾	24 h
- 5 °C	up to	- 1 °C	90 min ³⁾	14 h
0°C	up to	+ 4 °C	45 min ³⁾	7 h
+ 5 °C	up to	+ 9 °C	25 min ³⁾	2 h
+ 10 °C	up to	+ 19 °C	15 min ³⁾	80 min
+ 20 °C	up to	+ 24 °C	6 min ³⁾	45 min
+ 25 °C	up to	+ 29 °C	4 min ³⁾	25 min
+ 30 °C	up to	+ 40 °C	2,5 min ⁴⁾	15 min
Cartr	idge tempe	rature	+5°C up	to +40°C
1) The mainting una	au unita ar tima a l	a avalution l'al faut alum	has a matarial	

¹⁾ The minimum curing time is only valid for dry base material.

In wet base material the curing time must be doubled.

2) Cartridge temperature must be at least +15°C

3) Cartridge temperature must be between +5°C and +25°C

4) Cartridge temperature must be below +20°C

BEHA Composite mortar HQC300 for rebar connection

Intended Use Working and curing time



Installation instructions

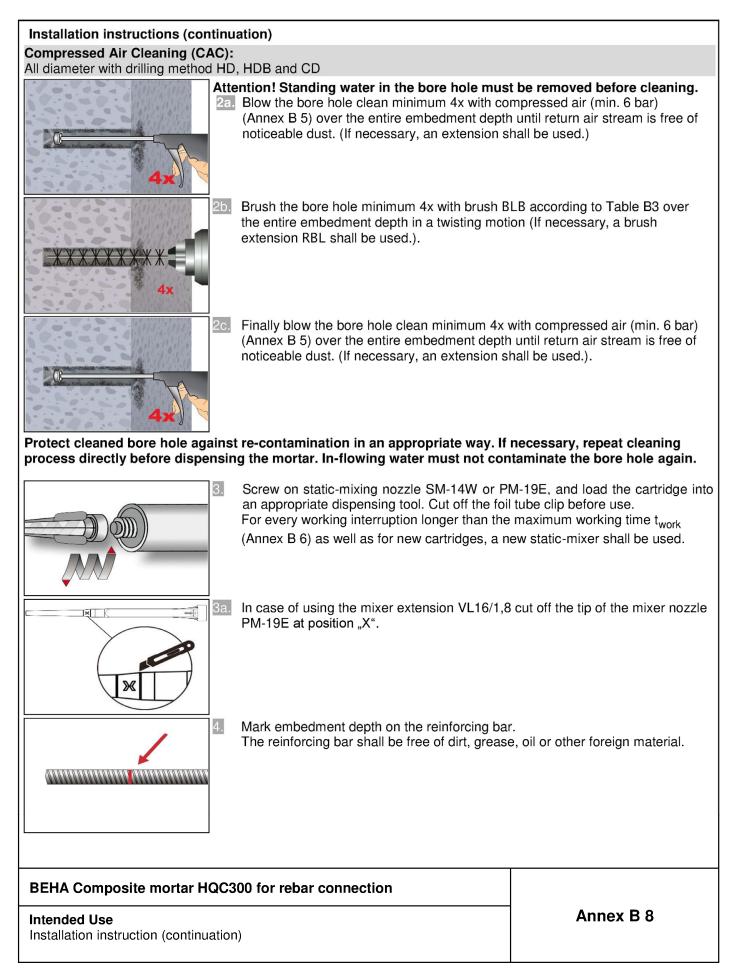
Attention: Before drilling, remove carbonated concrete and clean contact areas (see Annex B1) In case of aborted drill hole: the drill hole shall be filled with mortar.

Drilling of the bore hole Hammer drilling (HD) / Compressed air drilling (CD) 1a. Drill a hole to the required embedment depth. Drill bit diameter according to Table B3. Proceed with Step 2 (MAC or CAC). 1b. Hollow drill bit system (HDB) Drill a hole to the required embedment depth. Drill bit diameter according to Table B3. Proceed with Step 2 (MAC or CAC). Manual Air Cleaning (MAC) for drill hole diameter $d_0 \le 20$ mm and drill hole depth $h_0 \le 10\phi$ with drilling method HD, HDB and CD Attention! Standing water in the bore hole must be removed before cleaning. 2a. Blow the bore hole clean minimum 4x from the bottom or back by hand pump (Annex B 5). 4x Brush the bore hole minimum 4x with brush BLB according to Table B3 over the 2b. entire embedment depth in a twisting motion (if necessary, a brush extension RBL shall be used). Finally blow the bore hole clean minimum 4x from the bottom or back by hand 2c. pump (Annex B 5). BEHA Composite mortar HQC300 for rebar connection

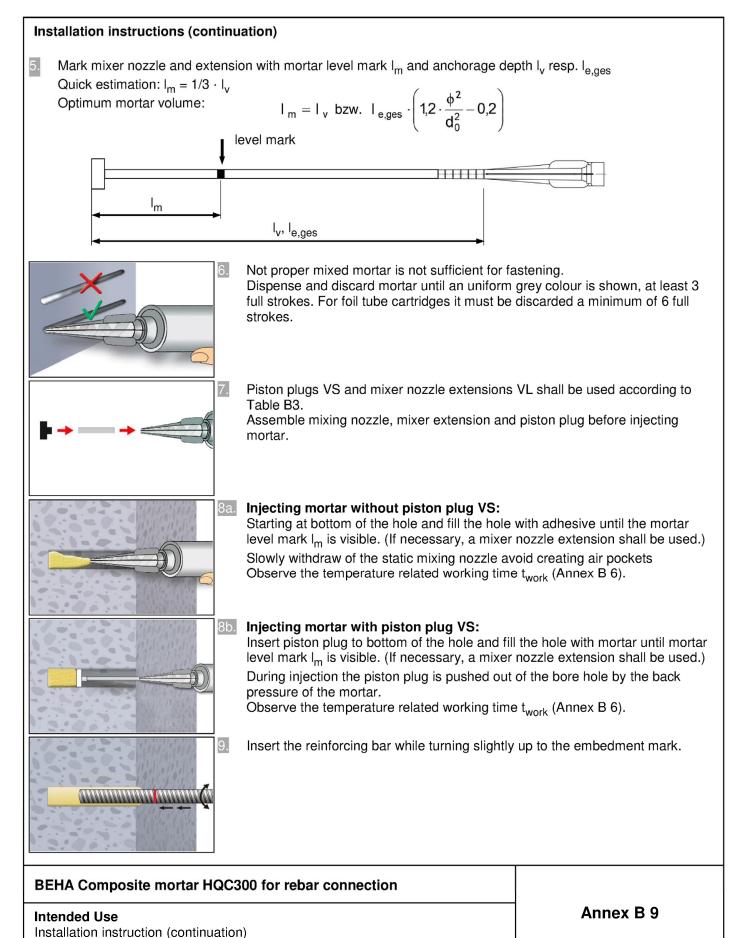
Intended Use

Installation instruction

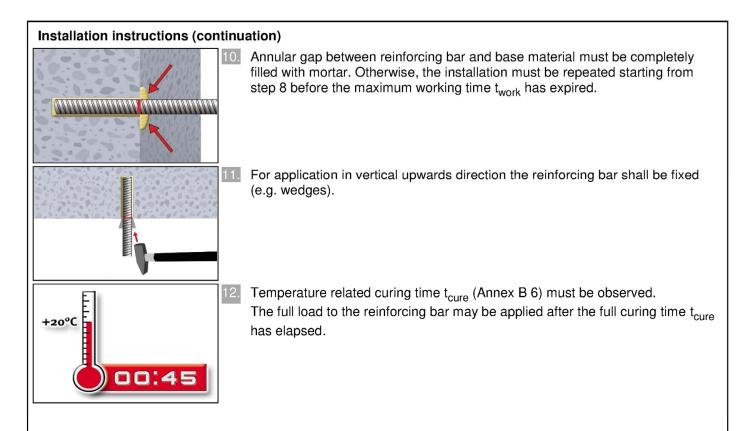












BEHA Composite mortar HQC300 for rebar connection

Intended Use Installation instruction (continuation)



Table C1: Characteristi	c tensior	n resistanc	ce for tension	on anchor Z	Α	
Tension Anchor			M12	M16	M20	M24
Steel, zinc plated (ZA vz)				•		
Characteristic tension resistance	N _{Rk,s}	[kN]	67	125	196	282
Partial factor	γ _{Ms.N} [-] 1,4					
Stainless Steel (ZA A4 or ZA HCR)					
Characteristic tension resistance	N _{Rk,s}	[kN]	67	125	171	247
Partial factor	γ _{Ms,N}	[-]	1	,4	1,3	1,4

Minimum anchorage length and minimum lap length under static or quasi-static loading

The minimum anchorage length I_{b,min} and the minimum Iap length I_{0,min} according to EN 1992-1-1:2004+AC:2010 ($I_{b,min}$ acc. to Eq. 8.6 and Eq. 8.7 and $I_{0,min}$ acc. to Eq. 8.11) shall be multiply by the amplification factor α_{lb} according to Table C2.

Table C2: Amplification factor α_{lb} related to concrete class and drilling method

Concrete class	Drilling method	Drilling method Bar size	
C12/15 to C50/60	all drilling methods	8 mm to 32 mm ZA-M12 to ZA-M24	1,0

Table C3: Reduction factor k_b for all drilling methods

				2.314722					
Rebar		Concrete class							
φ	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
8 to 25 mm ZA-M12 to ZA-M24					1,0				
28 to 32 mm				1,0				0,92	0,86

Table C4: Design values of the ultimate bond stress f_{bd.PIR} in N/mm² for all drilling methods and for good conditions

 $f_{bd,PIR} = k_b \cdot f_{bd}$ with

f_{bd}: Design value of the ultimate bond stress in N/mm² considering the concrete classes, the rebar diameter, the drilling method for good bond condition (for all other bond conditions multiply the values by η_1 =0.7) and recommended partial factor γ_c = 1,5 according to

EN 1992-1-1:2004+AC:2010.

k_b: Reduction factor according to Table C3

Rebar		Concrete class							
φ	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
8 to 25 mm ZA-M12 to ZA-M24	1,6	2,0	2,3	2,7	3,0	3,4	3,7	4,0	4,3
28 to 32 mm	1,6	2,0	2,3	2,7	3,0	3,4	3,7	3,7	3,7

BEHA Composite mortar HQC300 for rebar connection

Performances

Characteristic tension resistance for tension anchor, Minimum anchorage length and minimum lap length, Amplification factor, Reduction factor and Design values of ultimate bond resistance

Electronic copy of the ETA by DIBt: ETA-21/1044

Annex C 1



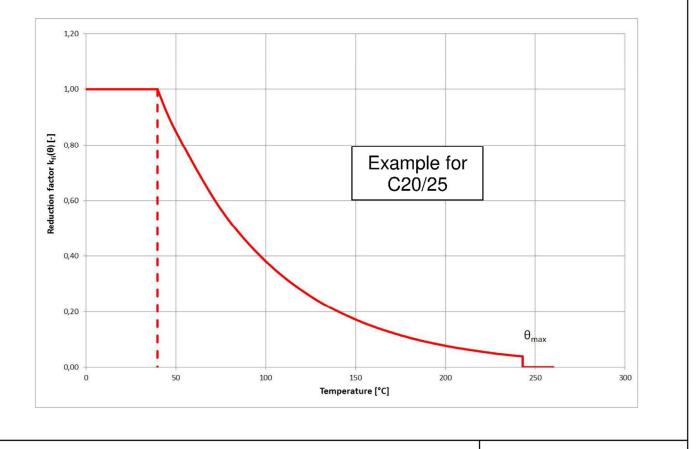
Design value of the ultimate bond stress $f_{bd,fi}$ at increased temperature for concrete classes C12/15 to C50/60, (all drilling methods):

The design value of the bond stress $f_{bd,fi}$ at increased temperature has to be calculated by the following equation:

For wo	orking life 50 years:	$f_{bd,fi} = k_{fi}(\theta) \cdot f_{bd,PIR} \cdot \gamma_c / \gamma_{M,fi}$
mit:	θ ≤ 243°C:	$k_{fi}(\theta) = 18,88 \cdot e^{(\theta \cdot -0,016)} / (f_{bd,PIR} \cdot 4,3) \le 1,0$
	θ > 243°C:	$k_{fi}(\theta) = 0$
f _{bd,fi}	Design value	e of the ultimate bond stress at increased temperature in N/mm ²
θ		e in °C in the mortar layer.
k _{fi} (θ)	Reduction fa	ctor at increased temperature.
f _{bd.PIR}	Design value	e of the bond stress in N/mm ² in cold condition according to Table C4 considering the
		sses, the rebar diameter, the drilling method and the bond conditions according to :2004+AC:2010.
γ _c	= 1,5 recomr	mended partial factor according to EN 1992-1-1:2004+AC:2010
γ _{M,fi}	= 1,0 recomr	mended partial factor according to EN 1992-1-2:2004+AC:2008
For ev	idence at increased tem	perature the anchorage length shall be calculated according to

EN 1992-1-1:2004+AC:2010 Equation 8.3 using the temperature-dependent design value of ultimate bond stress f_{bd,fi}.

Example graph of Reduction factor $k_{fi}(\theta)$ for concrete classes C20/25 for good bond conditions:



BEHA Composite mortar HQC300 for rebar connection

Performances

Design value of ultimate bond stress at increased temperature

Annex C 2



Tension Anchor				M12	M16	M20	M24
Steel, zinc plated				10112	WIO	W20	10124
Characteristic tension resistance	R30	- N _{Rk,s,fi}	[kN]	2,3	4,0	6,3	9,0
	R60			1,7	3,0	4,7	6,8
	R90			1,5	2,6	4,1	5,9
	R120			1,1	2,0	3,1	4,5
Stainless Steel (Z	ZA A4 or Z	A HCR)					
Characteristic tension resistance	R30	- N _{Rk,s,fi}	[kN]	3,4	6,0	9,4	13,6
	R60			2,8	5,0	7,9	11,3
	R90			2,3	4,0	6,3	9,0
	R120			1,8	3,2	5,0	7,2

BEHA Composite mortar HQC300 for rebar connection

Performances

Characteristic tension resistance for tension anchor ZA under fire exposure

Annex C 3